Contract Provisions and Plans

For Construction of:

2015 RESURFACING OF ACP ROADWAYS
PROJECT #ESAO15-1

SKAGIT COUNTY PUBLIC WORKS

2015 Resurfacing of ACP Roadways
Project #ESAO15-1

This Contract provides for the improvement of approximately 2.2 miles of La Conner Whitney Road. The work to be performed under this contract includes: planing of existing pavement and hauling planings to a County owned pit; application of a 2" HMA Cl ½" PG 64-22 wearing course over the existing roadway; providing erosion control, trimming and cleanup, traffic control, signage; and other work, all in accordance with the attached Contract Plans, these Contract Provisions, and the Standard Specifications.

Schedule: This project is to be completed within 15 working days from Notice to Proceed.

Measurement & Payment: Each item will be per the bid proposal.

Project Locations: La Conner Whitney Road, starting at roundabout heading north 2.2 miles

2015 RESURFACING OF ACP ROADWAYS
PROJECT #ESAO15-1

SKAGIT COUNTY, WASHINGTON

2015
SKAGIT COUNTY
DEPARTMENT OF PUBLIC WORKS
MOUNT VERNON, WASHINGTON 98273-5625

NOTICE TO ALL PLAN HOLDERS

Copies of the Plans and specifications are available at Skagit County Public Works, 1800 Continental Place, Mount Vernon, Washington 98273-5625. Telephone: (360) 416-1400. You may receive the bid information electronically; copies of the plans and specifications are available at: http://www.skagitcounty.net/rfp

APPROVED:

Paul A. Randall-Grutter, P.E.
County Engineer

MAPS, PLANS, AND SPECIFICATIONS APPROVED:

BOARD OF COUNTY COMMISSIONERS
SKAGIT COUNTY, WASHINGTON

Kenneth A. Dahlstedt, Chair

Lisa Janicki, Commissioner

Ron Wesen, Commissioner

2015 RESURFACING OF ACP ROADWAYS
PROJECT #ESAO15-1

CERTIFICATION

We hereby certify that these contract documents were prepared by us or under our direct supervision, and that we are duly registered Professional Engineers under the laws of the State of Washington.

Engineer of Record

Design Engineer

NOTICE OF CALL FOR BIDS

NOTICE IS HEREBY GIVEN by SKAGIT COUNTY that sealed bids will be received and publicly opened in the Commissioners' Hearing Room, 1800 Continental Place, Mount Vernon, WA 98273 on **Monday, June 8, 2015, at the hour of 2:30 p.m.,** or as soon thereafter as possible, for the following construction work:

PROJECT DESCRIPTION: **2015 Resurfacing of ACP Roadways Project #ESAO15-1**

This Contract provides for the improvement of approximately 2.2 miles of La Conner Whitney Road. The work to be performed under this contract includes: planing of existing pavement and hauling planings to a County owned pit; application of a 2" HMA Cl ½" PG 64-22 wearing course over the existing roadway; providing erosion control, trimming and cleanup, traffic control, signage; and other work, all in accordance with the attached Contract Plans, these Contract Provisions, and the Standard Specifications.

The time limit for physical completion of work is a total of 15 WORKING DAYS. The Engineer's Estimate Range is $550,900.00 to $642,800.00

Contractor and all subcontractors shall have a contractor's license to work in the State of Washington.

Information, copies of maps, plans, specifications, and addenda for this project will be available on-line beginning **May 21, 2015**, at http://www.skagitcounty.net/rfp or obtained at Skagit County Public Works Department, 1800 Continental Place, Mount Vernon, Washington; (360) 416-1400. Contractors who download plans and specifications are advised to e-mail pw@co.skagit.wa.us to be added to plan holders list to receive any addenda that may be issued.

An optional, non-mandatory pre-bid meeting will be held **Wednesday May 27, 2015 at 10:00 a.m**. at Skagit County Public Works, 1800 Continental Place, Mount Vernon, WA 98273. All technical questions regarding this project are to be submitted **no later than 2:00 p.m., Thursday, May 28, 2015** in writing to Jennifer Swanson, Project Manager, or by e-mail to jennifers@co.skagit.wa.us with the subject line reading, **"2015 Resurfacing of ACP Roadways Project #ESAO15-1"**. All project specific questions and response to answers for this project will be available on-line as received. **All Addenda will be posted on-line for this project by 5:00 p.m. Friday, May 29, 2015**. If further Addenda are required to be issued, the bid opening will be postponed.

All bid envelopes must be plainly marked on the outside, "**Sealed Bid for 2015 Resurfacing of ACP Roadways Project #ESAO15-1".** Sealed bids shall be received by one of the following delivery methods before **Monday, June 8, 2015, at the hour of 2:30 p.m**. Proposals are to be submitted on the forms provided in the Bid Proposal Packet. Incomplete proposals and proposals received after the time fixed for the opening cannot be considered. Oral, telephonic, telegraphic, electronic or faxed proposals will not be accepted. All bidding shall be based upon compliance with the Contract Provisions and Plans.

1. **Hand delivered**: Bids delivered in person shall be received only at the office of the SKAGIT COUNTY COMMISSIONERS, Reception Desk, 1800 Continental Place, Suite 100, Mount Vernon, WA 98273-5625.
2. **Via mail**: Bids shall be mailed to the SKAGIT COUNTY COMMISSIONERS, 1800 Continental Place, Suite 100, Mount Vernon, WA 98273-5625.

BID GUARANTY: No bid will be considered unless accompanied by a surety company bid bond, or a certified or cashier's check payable to the order of Skagit County for a sum not less than five percent (5%) of the total amount of the bid. A Contract Bond covering performance and payment will be required with the contract. Washington State Prevailing Wage Rates apply to this contract and bidders are advised to consider this charge when tabulating bids.

Skagit County reserves the right to reject any or all bids, and the right to waive any informalities or irregularities in any bid or in any bidding and to further award the Project to the lowest, responsive, responsible bidder whose bid complies with all of the prescribed formalities, as it best serves the interest of Skagit County. After the date and hour set for the opening of bids, no bidder may withdraw its bid unless the award of the contract is delayed for a period exceeding sixty (60) calendar days following bid opening. All bidders agree to be bound by their bids until the expiration of this stated time period.

Skagit County in accordance with Title VI of the Civil Rights Act of 1964, 78 Stat. 252, 42 U.S.C. 2000d to 2000d-4 and Title 49, Code of Federal Regulations, Department of Transportation, subtitle A, Office of the Secretary, Part 21, Nondiscrimination in Federally-Assisted Programs of the Department of Transportation issued pursuant to such Act, hereby notifies all bidders that it will affirmatively ensure that in any contract entered into pursuant to this advertisement, disadvantaged business enterprises as defined at 49 CFR Part 26 will be afforded full opportunity to submit bids in response to this invitation and will not be discriminated against on the grounds of race, color, national origin, or sex in consideration for an award.

For questions regarding Skagit County's Title VI Program, you may contact the Public Works Department's Title VI Liaison, Bill Dowe, at (360) 336-9400

The Board of Skagit County Commissioners reserves the right to reject any or all bids.

NOTICE GIVEN BY ORDER OF THE BOARD OF SKAGIT COUNTY COMMISSIONERS this 18 day of May, 2015.

Linda Hammer

Clerk of the Board

Published: Skagit Valley Herald – May 21, May 28, 2015

PAGE

CONTENTS

PAGE

DIVISION 2
EARTHWORK

PAGE

DIVISION 3
AGGREGATE PRODUCTION AND ACCEPTANCE

DIVISION 5
SURFACE TREATMENTS AND PAVEMENTS

DIVISION 6
STRUCTURES

PAGE

DIVISION 8
MISCELLANEOUS CONSTRUCTION

INTRODUCTION

The following Amendments and Special Provisions shall be used in conjunction with the 2014 Standard Specifications for Road, Bridge, and Municipal Construction.

AMENDMENTS TO THE STANDARD SPECIFICATIONS

The following Amendments to the Standard Specifications are made a part of this contract and supersede any conflicting provisions of the Standard Specifications. For informational purposes, the date following each Amendment title indicates the implementation date of the Amendment or the latest date of revision.

Each Amendment contains all current revisions to the applicable section of the Standard Specifications and may include references which do not apply to this particular project.

Section 1-01, Definitions and Terms
August 4, 2014

1-01.3 Definitions

The definition for "**Engineer**" is revised to read:

> The Contracting Agency's representative who directly supervises the engineering and administration of a construction Contract.

The definition for "**Inspector**" is revised to read:

> The Engineer's representative who inspects Contract performance in detail.

The definition for "**Project Engineer**" is revised to read:

> Same as Engineer.

The definition for "**Working Drawings**" is revised to read:

> Drawings, plans, diagrams, or any other supplementary data or calculations, including a schedule of submittal dates for Working Drawings where specified, which the Contractor must submit to the Engineer.

Section 1-02, Bid Procedures and Conditions
April 7, 2014

1-02.8(1) Noncollusion Declaration

The third paragraph is revised to read:

> Therefore, by including the Non-collusion Declaration as part of the signed bid Proposal, the Bidder is deemed to have certified and agreed to the requirements of the Declaration.

Section 1-03, Award and Execution of Contract
January 5, 2015

1-03.3 Execution of Contract
The first paragraph is revised to read:

Within 20 calendar days after the Award date, the successful Bidder shall return the signed Contracting Agency-prepared Contract, an insurance certification as required by Section 1-07.18, and a satisfactory bond as required by law and Section 1-03.4, and shall be registered as a contractor in the state of Washington.

1-03.4 Contract Bond
The last word of item 3 is deleted.

Item 4 is renumbered to 5.

The following is inserted after item 3 (after the preceding Amendments are applied):

4. Be conditioned upon the payment of taxes, increases, and penalties incurred on the project under titles 50, 51, and 82 RCW; and

1-03.5 Failure to Execute Contract
The first sentence is revised to read:

Failure to return the insurance certification and bond with the signed Contract as required in Section 1-03.3, or failure to provide Disadvantaged, Minority or Women's Business Enterprise information if required in the Contract, or failure or refusal to sign the Contract, or failure to register as a contractor in the state of Washington shall result in forfeiture of the proposal bond or deposit of this Bidder.

Section 1-04, Scope of the Work
August 4, 2014

1-04.4 Changes
In the third paragraph, item number 1 and 2 are revised to read:

A. When the character of the Work as altered differs materially in kind or nature from that involved or included in the original proposed construction; or

B. When an item of Work, as defined elsewhere in the Contract, is increased in excess of 125 percent or decreased below 75 percent of the original Contract quantity. For the purpose of this Section, an item of Work will be defined as any item that qualifies for adjustment under the provisions of Section 1-04.6.

The last two paragraphs are deleted.

This section is supplemented with the following new subsections:

1-04.4(2) Value Engineering Change Proposal (VECP)

1-04.4(2)A General

A VECP is a Contractor proposed change to the Contract Provisions which will accomplish the projects functional requirements in a manner that is equal to or better than the requirements in the Contract. The VECP may be: (1) at a less cost or time, or (2) either no cost savings or a minor increase in cost with a reduction in Contract time. The net savings or added costs to the Contract Work are shared by the Contractor and Contracting Agency.

The Contractor may submit a VECP for changing the Plans, Specifications, or other requirements of the Contract. The Engineer's decision to accept or reject all or part of the proposal is final and not subject to arbitration under the arbitration clause or otherwise subject to litigation.

The VECP shall meet all of the following:

1. Not adversely affect the long term life cycle costs.

2. Not adversely impact the ability to perform maintenance.

3. Provide the required safety and appearance.

4. Provide substitution for deleted or reduced Disadvantaged Business Enterprise Condition of Award Work, Apprentice Utilization and Training.

VECPs that provide a time reduction shall meet the following requirements:

1. Time saving is a direct result of the VECP.

2. Liquidated damages penalties are not used to calculate savings.

3. Administrative/overhead cost savings experienced by either the Contractor or Contracting Agency as a result of time reduction accrue to each party and are not used to calculate savings.

1-04.4(2)B VECP Savings

1-04.4(2)B1 Proposal Savings

The incentive payment to the Contractor shall be one-half of the net savings of the proposal calculated as follows:

1. (gross cost of deleted work) – (gross cost of added work) = (gross savings)

2. (gross savings) – (Contractor's engineering costs) – (Contracting Agency's costs) = (net savings)

3. (net savings) / 2 = (incentive pay)

The Contracting Agency's costs shall be the actual consultant costs billed to the Contracting Agency and in-house costs. Costs for personnel assigned to the Engineer's office shall not be included.

1-04.4(2)B2 Added Costs to Achieve Time Savings

The cost to achieve the time savings shall be calculated as follows:

1. (cost of added work) + (Contractor's engineering costs - Contracting Agency's engineering costs) = (cost to achieve time savings)

2. (cost to achieve time savings) / 2 = (Contracting Agency's share of added cost)

If the timesaving proposal also involves deleting work and, as a result, creates a savings for the Contracting Agency, then the Contractor shall also receive one-half of the savings realized through the deletion.

1-04.4(2)C VECP Approval

1-04.4(2)C1 Concept Approval

The Contractor shall submit a written proposal to the Engineer for consideration. The proposal shall contain the following information:

1. An explanation outlining the benefit provided by the change(s).

2. A narrative description of the proposed change(s). If applicable, the discussion shall include a demonstration of functional equivalency or a description of how the proposal meets the original contract scope of work.

3. A cost discussion estimating any net savings. Savings estimates will generally follow the outline below under the section, "Proposal Savings".

4. A statement providing the Contracting Agency with the right to use all or any part of the proposal on future projects without future obligation or compensation.

5. A statement acknowledging and agreeing that the Engineer's decision to accept or reject all or part of the proposal is final and not subject to arbitration under the arbitration clause or otherwise be subject to claims or disputes.

6. A statement giving the dates the Engineer must make a decision to accept or reject the conceptual proposal, the date that approval to proceed must be received, and the date the work must begin in order to not delay the contract. If the Contracting Agency does not approve the VECP by the date specified by the Contractor in their proposal the VECP will be deemed rejected.

7. The submittal will include an analysis on other Work that may have costs that changed as a result of the VECP. Traffic control and erosion control shall both be included in addition to any other impacted Work.

After review of the proposal, the Engineer will respond in writing with acceptance or rejection of the concept. This acceptance shall not be construed as authority to proceed with any change contract work. Concept approval allows the Contractor to proceed with the Work needed to develop final plans and other information to receive formal approval and to support preparation of a change order.

1-04.4(2)C2 Formal Approval

The Contractor's submittal to the Engineer for formal approval shall include the following:

1. Deleted Work – Include the calculated quantities of unit price Work to be deleted. Include the proposed partial prices for portions of lump sum Work deleted. For deletion of force account items include the time and material estimates.

2. Added Work – Include the calculated quantities of unit price Work to be added, either by original unit Contract prices or by new, negotiated unit prices. For new items of Work include the quantities and proposed prices.

3. Contractor's Engineering Costs – Submit the labor costs for the engineering to develop the proposal; costs for Contractor employees utilized in contract operations on a regular basis shall not be included.

4. Schedule Analysis – If the VECP is related to time savings, the Contractor shall submit a partial progress schedule showing the changed Work. The submittal shall also include a discussion comparing the partial progress schedule with the approved progress schedule for the project.

5. Working Drawings – Type 3 Working Drawings shall be submitted; those drawings which require engineering shall be a Type 3E.

Formal approval of the proposal will be documented by issuance of a change order. The VECP change order will contain the following statements which the Contractor agrees to by signing the change order:

1. The Contractor accepts design risk of all features, both temporary and permanent, of the changed Work.

2. The Contractor accepts risk of constructability of the changed Work.

3. The Contractor provides the Contracting Agency with the right to use all or any part of the proposal on future projects without further obligation or compensation.

VECP change orders will contain separate pay items for the items that are applicable to the Proposal. These are as follows:

1. Deleted Work.

2. Added Work.

3. The Contractor's engineering costs, reimbursed at 100 percent of the Contractor's cost.

4. Incentive payment to the Contractor.

When added Work costs exceed Deleted Work costs, but time savings make a viable proposal, then items 3 and 4 above are replaced with the following:

3. The Contracting Agency's share of added cost to achieve time savings.

4. The Contractor's share of savings from deleted Work.

1-04.4(2)C3 Authority to Proceed with Changed Work

The authority for the Contractor to proceed with the VECP Work will be provided by one of the following options:

1. Execution of the VECP change order, or

2. At the Contractor's request the Contracting Agency may provide approval by letter from the Engineer for the Work to proceed prior to execution of a change order. All of the risk for proceeding with the VECP shall be the responsibility of the Contractor. Additionally, the following criteria are required to have been met:

   a) Concept approval has been granted by the Contracting Agency.

   b) All design reviews and approvals have been completed, including plans and specifications.

   c) The Contractor has guaranteed, in writing, the minimum savings to the Contracting Agency.

Section 1-05, Control of Work
August 4, 2014

1-05.1 Authority of the Engineer

In this section, "Project Engineer" is revised to read "Engineer".

The second paragraph (up until the colon) is revised to read:

The Engineer's decisions will be final on all questions including the following:

The first sentence in the third paragraph is revised to read:

The Engineer represents the Contracting Agency with full authority to enforce Contract requirements.

1-05.2 Authority of Assistants and Inspectors

The first paragraph is revised to read:

The Engineer may appoint assistants and Inspectors to assist in determining that the Work and materials meet the Contract requirements. Assistants and Inspectors have the authority to reject defective material and suspend Work that is being done improperly, subject to the final decisions of the Engineer.

In the third paragraph, “Project Engineer” is revised to read “Engineer”.

1-05.3 Plans and Working Drawings

This section’s title is revised to read:

Working Drawings

This section is revised to read:

The Contract may require the Contractor to submit Working Drawings for the performance of the Work. Working Drawings shall be submitted by the Contractor electronically to the Engineer in PDF format; drawing details shall be prepared in accordance with conventional detailing practices. If the PDF format is found to be unacceptable, at the request of the Engineer, the Contractor shall provide paper copies of the Working Drawings with drawings on 11 by 17 inch sheets and calculations/text on 8½ by 11 inch sheets.

Working Drawings will be classified under the following categories:

1. **Type 1** – Submitted for Contracting Agency information. Submittal must be received by the Contracting Agency a minimum of 7 calendar days before work represented by the submittal begins.

2. **Type 2** – Submitted for Contracting Agency review and comment. Unless otherwise stated in the Contract, the Engineer will require up to 20 calendar days from the date the Working Drawing is received until it is returned to the Contractor. The Contractor shall not proceed with the Work represented by the Working Drawing until comments from the Engineer have been addressed.

3. **Type 2E** – Same as a Type 2 Working Drawing with Engineering as described below.

4. **Type 3** – Submitted for Contracting Agency review and approval. Unless otherwise stated in the Contract, the Engineer will require up to 30 calendar days from the date the Working Drawing is received until it is returned to the Contractor. The Contractor shall obtain the Engineer’s written approval before proceeding with the Work represented by the Working Drawing.

5. **Type 3E** – Same as a Type 3 Working Drawing with Engineering as described below.

All Working Drawings shall be considered Type 3 Working Drawings except as specifically noted otherwise in the Contract. Unless designated otherwise by the Contractor, submittals of Working Drawings will be reviewed in the order they are received by the Engineer. In the event that several Working Drawings are received simultaneously, the Contractor shall specify the sequence in which they are to be reviewed. If the Contractor does not submit a review sequence for simultaneous Working Drawing submittals, the review sequence will be at the Engineer's discretion.

Working Drawings requiring Engineering, Type 2E and 3E, shall be prepared by (or under the direction of) a Professional Engineer, licensed under Title 18 RCW, State of Washington, and in accordance with WAC 196-23-020. Design calculations shall carry the Professional Engineer's signature and seal, date of signature, and registration number on the cover page. The cover page shall also include the Contract number, Contract title and sequential index to calculation page numbers.

If more than the specified number of days is required for the Engineer's review of any individual Working Drawing or resubmittal, an extension of time will be considered in accordance with Section 1-08.8.

Review or approval of Working Drawings shall neither confer upon the Contracting Agency nor relieve the Contractor of any responsibility for the accuracy of the drawings or their conformity with the Contract. The Contractor shall bear all risk and all costs of any Work delays caused by rejection or nonapproval of Working Drawings.

Unit Bid prices shall cover all costs of Working Drawings.

Section 1-07, Legal Relations and Responsibilities to the Public
January 5, 2015

1-07.2 State Taxes

This section is revised to read:

The Washington State Department of Revenue has issued special rules on the state sales tax. Sections 1-07.2(1) through 1-07.2(3) are meant to clarify those rules. The Contracting Agency will not adjust its payment if the Contractor bases a Bid on a misunderstood tax liability.

The Contracting Agency may deduct from its payments to the Contractor, retainage or lien the bond, in the amount the Contractor owes the State Department of Revenue, whether the amount owed relates to the Contract in question or not. Any amount so deducted will be paid into the proper State fund on the contractor's behalf. For additional information on tax rates and application refer to applicable RCWs, WACs or the Department of Revenue's website.

1-07.2(1) State Sales Tax: Work Performed on City, County, or Federally-Owned Land

This section including title is revised to read:

1-07.2(1) State Sales Tax: WAC 458-20-171 – Use Tax

For Work designated as Rule 171, **Use Tax,** the Contractor shall include for compensation the amount of any taxes paid in the various unit Bid prices or other Contract amounts. Typically, these taxes are collected on materials incorporated into the

project and items such as the purchase or rental of; tools, machinery, equipment, or consumable supplies not integrated into the project.

The Summary of Quantities in the Contract Plans identifies those parts of the project that are subject to **Use Tax** under Section 1-07.2(1).

1-07.2(2) State Sales Tax: Work on State-Owned or Private Land

This section including title is revised to read:

1-07.2(2) State Sales Tax: WAC 458-20-170 – Retail Sales Tax

For Work designated as Rule 170, **Retail Sales Tax,** the Contractor shall collect from the Contracting Agency, **Retail Sales Tax** on the full Contract price. The Contracting Agency will automatically add this **Retail Sales Tax** to each payment to the Contractor and for this reason; the Contractor shall not include the **Retail Sales Tax** in the unit Bid prices or in any other Contract amount. However, the Contracting Agency will not provide additional compensation to the Prime Contractor or Subcontractor for **Retail Sales Taxes** paid by the Contractor in addition to the **Retail Sales Tax** on the total contract amount. Typically, these taxes are collected on items such as the purchase or rental of; tools, machinery, equipment, or consumable supplies not integrated into the project. Such sales taxes shall be included in the unit Bid prices or in any other Contract amounts.

The Summary of Quantities in the Contract Plans identifies those parts of the project that are subject to **Retail Sales Tax** under Section 1-07.2(2).

1-07.2(3) Services

This section is revised to read:

Any contract wholly for professional or other applicable services is generally not subject to **Retail Sales Tax** and therefore the Contractor shall not collect **Retail Sales Tax** from the Contracting Agency on those Contracts. Any incidental taxes paid as part of providing the services shall be included in the payments under the contract.

1-07.23(1) Construction Under Traffic

In the second paragraph, the following new sentence is inserted after the second sentence:

Accessibility to existing or temporary pedestrian push buttons shall not be impaired.

Section 1-08, Prosecution and Progress

May 5, 2014

1-08.1 Subcontracting

The eighth paragraph is revised to read:

On all projects, the Contractor shall certify to the actual amounts paid to Disadvantaged, Minority, Women's, or Small Business Enterprise firms that were used as Subcontractors, lower tier subcontractors, manufacturers, regular dealers, or service providers on the Contract. This Certification shall be submitted to the Project Engineer on a monthly basis each month between Execution of the Contract and Physical Completion of the contract using the application available at: https://remoteapps.wsdot.wa.gov/mapsdata/tools/dbeparticipation. The monthly report is due 20 calendar days following the end of the month. A monthly report shall be

submitted for every month between Execution of the Contract and Physical Completion regardless of whether payments were made or work occurred.

The ninth paragraph is deleted.

Section 1-09, Measurement and Payment
January 5, 2015

1-09.6 Force Account

In the third paragraph of item number 3, the last sentence is revised to read:

In the event that prior quotations are not obtained and the vendor is not a firm independent from the Contractor or Subcontractor, then after-the-fact quotations may be obtained by the Engineer from the open market in the vicinity and the lowest such quotation may be used in place of submitted invoice.

Section 1-10, Temporary Traffic Control
August 4, 2014

1-10.1(1) Materials

The following material reference is deleted from this section:

Barrier Drums 9-35.8

1-10.1(2) Description

The first paragraph is revised to read:

The Contractor shall provide flaggers, and all other personnel required for labor for traffic control activities and not otherwise specified as being furnished by the Contracting Agency.

1-10.2(1) General

In the third paragraph, the first two sentences are revised to read:

The primary and alternate TCS shall be certified by one of the organizations listed in the Special Provisions. Possession of a current Washington State TCS card and flagging card by the primary and alternate TCS is mandatory.

1-10.2(1)B Traffic Control Supervisor

The first paragraph is revised to read:

A Traffic Control Supervisor (TCS) shall be present on the project whenever flagging or other traffic control labor is being utilized or less frequently, as authorized by the Engineer.

The last paragraph is revised to read:

The TCS may perform the Work described in Section 1-10.3(1)A Flaggers or in Section 1-10.3(1)B Other Traffic Control Labor and be compensated under those Bid items, provided that the duties of the TCS are accomplished.

1-10.2(2) Traffic Control Plans

The first paragraph is revised to read:

> The traffic control plan or plans appearing in the Contract documents show a method of handling vehicle, bicycle, and pedestrian traffic. All construction signs, flaggers, and other traffic control devices are shown on the traffic control plan(s) except for emergency situations. If the Contractor proposes adding the use of flaggers to a plan, this will constitute a modification requiring approval by the Engineer. The modified plans shall show locations for all the required advance warning signs and a safe, protected location for the flagging station. If flagging is to be performed during hours of darkness, the plan shall include appropriate illumination for the flagging station.

In the second paragraph, the second sentence is revised to read:

> Any Contractor-proposed modification, supplement or replacement shall show the necessary construction signs, flaggers, and other traffic control devices required to support the Work.

1-10.2(3) Conformance to Established Standards

In the second paragraph, the second sentence is revised to read:

> The National Cooperative Highway Research Project (NCHRP) Report 350 and the AASHTO Manual for Assessing Safety Hardware (MASH) have established requirements for crash testing.

In the third paragraph, “NCHRP 350” is revised to read “NCHRP 350 or MASH”.

In the fourth paragraph, “NCHRP 350” is revised to read “NCHRP 350 or MASH”.

In the fifth paragraph, “NCHRP 350” is revised to read “NCHRP 350 or MASH”.

1-10.3(1) Traffic Control Labor

The first paragraph is revised to read:

> The Contractor shall furnish all personnel for flagging, for the execution of all procedures related to temporary traffic control and for the setup, maintenance and removal of all temporary traffic control devices and construction signs necessary to control vehicular, bicycle, and pedestrian traffic during construction operations.

1-10.3(1)A Flaggers and Spotters

This section’s title is revised to read:

> **Flaggers**

The first paragraph is revised to read:

> Flaggers shall be posted where shown on approved Traffic Control Plans or where directed by the Engineer. All flaggers shall possess a current flagging card issued by the State of Washington, Oregon, Montana, or Idaho. The flagging card shall be immediately available and shown to the Contracting Agency upon request.

The last paragraph is deleted.

1-10.3(1)B Other Traffic Control Labor

This section is revised to read:

In addition to flagging duties, the Contractor shall provide personnel for all other traffic control procedures required by the construction operations and for the labor to install, maintain and remove any traffic control devices shown on Traffic Control Plans.

1-10.3(3)B Sequential Arrow Signs

This section is supplemented with the following:

A sequential arrow sign is required for all lane closure tapers on a multilane facility. A separate sequential arrow sign shall be used for each closed lane. The arrow sign shall not be used to laterally shift traffic. When used in the caution mode, the four corner mode shall be used.

1-10.3(3)C Portable Changeable Message Signs

This section is revised to read:

Where shown on an approved traffic control plan or where ordered by the Engineer, the Contractor shall provide, operate, and maintain portable changeable message signs (PCMS). A PCMS shall be placed behind a barrier or guardrail whenever possible, but shall at a minimum provide 4 ft. of lateral clearance to edge of travelled lane and be delineated by channelization devices. The Contractor shall remove the PCMS from the clear zone when not in use unless protected by barrier or guardrail.

1-10.3(3)F Barrier Drums

This section including title is deleted in its entirety and replaced with the following:

1-10.3(3)F Vacant

1-10.3(3)K Portable Temporary Traffic Control Signal

The fifth paragraph is revised to read:

The Project Engineer or designee will inspect the signal system at initial installation/operation and approve the signal timing. Final approval will be based on the results of the operational inspection.

1-10.4(2) Item Bids With Lump Sum for Incidentals

In the second paragraph, the first and second sentences are revised to read:

"Flaggers" will be measured by the hour. Hours will be measured for each flagging station, shown on an approved Traffic Control Plan, when that station is staffed in accordance with Section 1-10.3(1)A.

The first sentence of the last bulleted item in this section is revised to read:

Installing and removing Barricades, Traffic Safety Drums, Cones, Tubular Markers and Warning Lights and Flashers to carry out approved Traffic Control Plan(s).

1-10.5(2) Item Bids With Lump Sum for Incidentals

This section is deleted and replaced with the following:

"Traffic Control Supervisor", lump sum.

The lump sum Contract payment shall be full compensation for all costs incurred by the Contractor in performing the Work defined in Section 1-10.2(1)B.

"Pedestrian Traffic Control", lump sum.

The lump sum Contract payment shall be full compensation for all costs incurred by the Contractor in performing the Work for pedestrian traffic control defined in Section 1-10.

"Flaggers", per hour.

The unit Contract price, when applied to the number of units measured for this item in accordance with Section 1-10.4(2), shall be full compensation for all costs incurred by the Contractor in performing the Work defined in Section 1-10.3(1)A.

"Other Traffic Control Labor", per hour.

The unit Contract price, when applied to the number of units measured for this item in accordance with Section 1-10.4(2), shall be full compensation for all labor costs incurred by the Contractor in performing the Work specified for this item in Section 1-10.4(2).

"Construction Signs Class A", per square foot.

The unit Contract price, when applied to the number of units measured for this item in accordance with Section 1-10.4(2), shall be full compensation for all costs incurred by the Contractor in performing the Work described in Section 1-10.3(3)A. In the event that "Do Not Pass" and "Pass With Care" signs must be left in place, a change order, as described in Section 1-04.4, will be required. When the Bid Proposal contains the item "Sign Covering", then covering those signs indicated in the Contract will be measured and paid according to Section 8-21.

"Sequential Arrow Sign", per hour.

The unit Contract price, when applied to the number of units measured for this item in accordance with Section 1-10.4(2), shall be full compensation for all costs incurred by the Contractor in performing the Work described in Section 1-10.3(3)B.

"Portable Changeable Message Sign", per hour.

The unit Contract price, when applied to the number of units measured for this item in accordance with Section 1-10.4(2), shall be full compensation for all costs incurred by the Contractor in performing the Work for procuring all portable changeable message signs required for the project and for transporting these signs to and from the project.

"Transportable Attenuator", per each.

The unit Contract price, when applied to the number of units measured for this item in accordance with Section 1-10.4(2), shall be full compensation for all costs incurred by the Contractor in performing the Work described in Section 1-10.3(3)J except for costs

compensated separately under the items “Operation of Transportable Attenuator” and “Repair Transportable Attenuator”.

“Operation of Transportable Attenuator”, per hour.

The unit Contract price, when applied to the number of units measured for this item in accordance with Section 1-10.4(2), shall be full compensation for all costs incurred by the Contractor in performing the Work for operating transportable attenuators on the project.

“Repair Transportable Attenuator”, by force account.

All costs of repairing or replacing transportable attenuators that are damaged by the motoring public while in use as shown on an approved Traffic Control Plan will be paid for by force account as specified in Section 1-09.6. To provide a common Proposal for all Bidders, the Contracting Agency has estimated the amount of force account for “Repair Transportable Attenuator” and has entered the amount in the Proposal to become a part of the total Bid by the Contractor. Transportable attenuators damaged due to the Contractor’s operation or damaged in any manner when not in use shall be repaired or replaced by the Contractor at no expense to the Contracting Agency.

“Other Temporary Traffic Control”, lump sum.

The lump sum Contract payment shall be full compensation for all costs incurred by the Contractor in performing the Work defined in Section 1-10, and which costs are not compensated by one of the above-listed items.

“Portable Temporary Traffic Control Signal”, lump sum.

The lump sum Contract payment shall be full compensation for all costs incurred by the Contractor in performing the Work as described in Section 1-10.3(3)K, including all costs for traffic control during manual control, adjustment, malfunction, or failure of the portable traffic control signals and during replacement of failed or malfunctioning signals.

Section 2-01, Clearing, Grubbing, and Roadside Cleanup
August 4, 2014

2-01.3(1) Clearing

In the second paragraph, item number 3 (up until the colon) is revised to read:

3. Follow these requirements for all stumps that will be buried deeper than 5 feet from the top, side, or end surface of the embankment or any structure and are in a location that will not be terraced as described in Section 2-03.3(14):

Section 2-02, Removal of Structures and Obstructions
January 5, 2015

2-02.3(2) Removal of Bridges, Box Culverts, and Other Drainage Structures

This section is supplemented with the following new subsections:

2-02.3(2)A Bridge Removal

2-02.3(2)A1 Bridge Demolition Plan Submittal

The Contractor shall submit a Type 2E Working Drawing consisting of a bridge demolition plan, showing the method of removing the existing bridge(s), or portions of bridges, as specified.

The bridge demolition plan shall show all equipment, sequence of operations, and details required to complete the work, including containment, collection, and disposal of all debris. The plan shall include a crane foundation stability analysis and crane load calculations for the work. The plan shall detail the containment, collection, and disposal of all debris. The plan shall show all stages of demolition.

When the bridge removal work includes removal of a truss, and when the Contractor's removal method involves use of a crane or cranes to pick, lift, and remove the truss, the Contractor shall confirm the truss dead load weight prior to beginning the truss removal operation. The operation of confirming the truss dead load shall be performed at both ends of the truss, and shall ensure that the truss is broken free of its support bearings. The Contractor's method of confirming the truss dead load, whether by hydraulic jacks or other means, shall be included in the Contractor's bridge demolition plan submittal.

When the bridge removal work involves removing portions of existing concrete without replacement, the methods and tools used to achieve the smooth surface and profile specified in Section 2-02.3(2)A2 shall be included in the Contractor's bridge demolition plan submittal.

2-02.3(2)A2 Removing Portions of Existing Concrete

Care shall be taken in removing concrete to prevent overbreakage or damage to portions of the existing Structure which are to remain. Before concrete removal begins, a saw cut shall be made into the surface of the concrete at the perimeter of the removal limits. The saw cut shall be 3/4-inch deep when the steel reinforcement is to remain, and may be deeper when the steel reinforcement is removed with the concrete.

Concrete shall be completely removed (exposing the deformed surface of the bar) from existing steel reinforcing bars which extend from the existing members and are specified to remain. Steel reinforcing bars that are not designated to remain shall be cut a minimum of 1-inch behind the final surface. The void left by removal of the steel reinforcing bar shall be filled with mortar conforming to Section 9-20.4(2). The mortar shall match the color of the existing concrete surface as nearly as practicable.

The Contractor shall roughen, clean, and saturate existing concrete surfaces, against which fresh concrete will be placed, in accordance with Section 6-02.3(12)B. When a portion of existing concrete is to be removed without replacement, concrete shall be removed to a clean line with a smooth surface of less than 1/16 inch profile.

2-02.3(2)A3 Use of Explosives for Bridge Demolition

Explosives shall not be used for bridge demolition, except as specifically allowed by the Special Provisions.

2-02.5 Payment

This section is supplemented with the following new Bid items:

"Removing Existing Bridge___", lump sum.

"Removing Existing Structure___", lump sum.

"Removing Portion of Existing Bridge___", lump sum.

"Removing Portion of Existing Structure___", lump sum.

Section 2-03, Roadway Excavation and Embankment
August 4, 2014

2-03.3(14) Embankment Construction

The third paragraph is revised to read:

Hillside Terraces – The Contractor shall terrace the original ground or embankment when the slope of the surface is 2H:1V or steeper unless otherwise directed by the Engineer. The face of each terrace shall be a minimum of 1 foot and a maximum of 5 feet in height and shall be vertical or near vertical as required to remain stable during material placement and compaction. The bench of the terrace shall slope outward to drain and shall not be inclined steeper than 0.05 foot per foot. Terraces damaged during work shall be reestablished. The Engineer may order the Contractor to place gravel backfill, pipe drains or both to drain any seepage.

2-03.3(14)L Embankment Widening for Guardrail

The first sentence is revised to read:

Embankments widened for the installation of beam guardrail shall be terraced in accordance with the requirements for hillside terraces in Section 2-03.3(14).

The second sentence is deleted.

Section 2-09, Structure Excavation
January 5, 2015

2-09.4 Measurement

The seventh paragraph is revised to read:

For pipelines the lower limit in measuring structure excavation will be the foundation level as shown in the Plans or as directed by the Engineer.

Section 2-12, Construction Geosynthetic
January 5, 2015

2-12.3(4) Permanent Erosion Control and Ditch Lining

In the fourth paragraph, "Section 9-13.2" is revised to read "Section 9-13.1(4)".

Section 3-04, Acceptance of Aggregate
April 6, 2015

3-04.5 Payment

In Table 1, the "Maximum Sublot Size (Tons)" value for the item HMA Aggregate is revised to read "2000".

In Table 2, the row containing the item "HMA Aggregate" is revised to read:

9-03.8(2)	HMA Aggregate						15	15	Uncompacted Void Content 15

Section 5-01, Cement Concrete Pavement Rehabilitation
August 4, 2014

5-01.2 Materials

The referenced section for the following item is revised to read:

Dowel Bars 9-07.5

5-01.3(4) Replace Portland Cement Concrete Panel

In the third paragraph, the last sentence is deleted.

The seventeenth paragraph (beginning with "The Contractor shall place a bond-breaking material…") is deleted.

Section 5-02, Bituminous Surface Treatment
August 4, 2014

5-02.3(11) Temporary Raised Pavement Markings

This section's title is revised to read:

Temporary Pavement Markings

The word "raised" is deleted from this section.

Section 5-04, Hot Mix Asphalt
April 6, 2015

5-04.2 Materials

The third through eighth paragraphs are deleted and replaced with the following:

The Contractor may choose to utilize recycled asphalt pavement (RAP) or reclaimed asphalt shingles (RAS) in the production of HMA. The RAP may be from pavements removed under the Contract, if any, or pavement material from an existing stockpile. The RAS may be from reclaimed shingles.

If greater than 20 percent RAP by total weight of HMA or any amount of RAS is utilized in the production of HMA, the Contractor shall sample and test the RAP and RAS during

stockpile construction in accordance with WSDOT FOP for AASHTO T 308 for determination of asphalt binder content and WSDOT FOP for WAQTC/AASHTO T 27/T 11 for gradation of the aggregates. The RAP shall be sampled and tested at a frequency of one sample for every 1,000 tons produced and not less than ten samples per project. The RAS shall be sampled and tested at a frequency of one sample for every 100 tons produced and not less than ten samples per project. The asphalt content and gradation test data shall be reported to the Contracting Agency prior to or when submitting the mix design for approval on the QPL. If utilized, the amount of RAS shall not exceed 5-percent of the total weight of the HMA. The Contractor shall include the RAP and RAS as part of the mix design as defined in these Specifications.

The grade of asphalt binder shall be as required by the Contract. Blending of asphalt binder from different sources is not permitted. For HMA with greater than 20 percent RAP by total weight of HMA or any amount of RAS, the final blended asphalt binder (after inclusion of RAP, RAS, new asphalt binder and recycling agent) shall be the grade as required by the Contract and comply with the requirements of Section 9-02.1(4).

The Contractor may only use warm mix asphalt (WMA) processes in the production of HMA with 20 percent or less RAP by total weight of HMA and no RAS. The Contractor shall submit to the Engineer for approval the process that is proposed and how it will be used in the manufacture of HMA.

When the Contracting Agency provides aggregates or provides a source for the production of aggregates, the Contract Provisions will establish the approximate percentage of asphalt binder required in the mixture for each class of HMA.

Production of aggregates shall comply with the requirements of Section 3-01.

Preparation of stockpile site, the stockpiling of aggregates, and the removal of aggregates from stockpiles shall comply with the requirements of Section 3-02.

5-04.3(1) Hot Mix Asphalt Mixing Plant

The first paragraph is supplemented with the following:

6. **Equipment for Processing RAP and RAS.** When producing HMA for mix designs with greater than 20 percent RAP by total weight of HMA or any amount of RAS the HMA plant shall be equipped with screens or a lump breaker to eliminate oversize RAP/RAS particles from entering the pug mill or drum mixer.

5-04.3(3)A Material Transfer Device/Vehicle

The first paragraph is supplemented with the following new sentence:

At the Contractor's request the Engineer may approve paving without an MTD/V; the Engineer will determine if an equitable adjustment in cost or time is due.

In the last sentence of the second paragraph, "Project Engineer" is revised to read "Engineer".

5-04.3(5)A Preparation of Existing Surfaces

The first sentence of the last paragraph is revised to read:

Unless otherwise approved by the Engineer, the tack coat shall be CSS-1 or CSS-1h emulsified asphalt.

5-04.3(7) Preparation of Aggregates

This section is revised to read:

The aggregates, RAP and RAS shall be stockpiled according to the requirements of Section 3-02. Sufficient storage space shall be provided for each size of aggregate, RAP and RAS. The Contractor may uniformly blend fine aggregate or RAP with the RAS as a method of preventing the agglomeration of RAS particles. The aggregates, RAP and RAS shall be removed from stockpile(s) in a manner to ensure minimal segregation when being moved to the HMA plant for processing into the final mixture. Different aggregate sizes shall be kept separated until they have been delivered to the HMA plant.

5-04.3(7)A1 General

This section is revised to read:

An approved mix design, listed on the Qualified Products List (QPL), is required for all HMA paving. The Contractor shall develop a mix design prior to the initial production of HMA and no more than 3 months prior to submitting for QPL evaluation. The mix design shall be developed in accordance with WSDOT Standard Operating Procedure 732 and meet the requirements of Sections 9-03.8(2) and 9-03.8(6).

Mix designs shall be submitted by the Contractor to the WSDOT State Materials Laboratory on WSDOT Form 350-042EF. If the mix design is approved it will be listed on the QPL for up to 24 consecutive months. Mix designs not listed on the QPL or past the 24 month approved period shall not be used. After a mix design has been on the QPL for 12 months the listing will be extended provided the Contractor submits a certification letter to the Qualified Products Engineer verifying that the aggregate and asphalt binder have not changed. The Contractor may submit the certification one month prior to expiration of the mix design approval. Within 7 calendar days of receipt of the Contractor's certification the QPL will be updated. The maximum duration for approval of a mix design and listing on the QPL will be 24 months from the date of initial approval or as approved by the Engineer.

Changes to the job mix formula of a mix design may require the development of a new mix design and resubmittal for QPL approval. Mix designs that require resubmittal for QPL approval must be approved prior to use.

Changes to aggregate that may require a new mix design include the source of material or a change in the percentage of material from a stockpile greater than 5 percent. Changes to the percentage of material from a stockpile will be calculated exclusive of the RAP content. The Contractor may vary the RAP percentage in accordance with Section 5-04.2.

Changes to asphalt binder that may require a new mix design include the source of the crude petroleum supplied to the refinery, the refining process, and additives or modifiers in the asphalt binder.

The Contractor shall include the brand and type of anti-stripping additive in the mix design submittal and provide certification from the asphalt binder manufacture that the

anti-stripping additive is compatible with the crude source and formulation of asphalt binder proposed in the mix design. All changes to anti-strip require the submittal of a new mix design for approval.

Mix designs with 20 percent RAP or less by total weight of HMA and no RAS will be completed without the inclusion of the RAP. For HMA mix designs with greater than 20 percent RAP by total weight of HMA or any amount of RAS the Contractor shall develop a mix design including RAP, RAS, recycling agent and new asphalt binder. Asphalt binder contributed from RAS shall be determined in accordance with AASHTO PP 78. The total quantity of asphalt binder from the RAP and RAS shall not exceed 40 percent of the total asphalt binder content of the HMA.

Once the RAP and RAS stockpiles have been constructed the Contractor shall extract, recover and test the asphalt residue from the RAP and RAS stockpiles to determine the percent of recycling agent and/or grade of new asphalt binder needed to meet the grade of asphalt binder required by the contract. The asphalt extraction testing shall be performed in accordance with AASHTO T 164 or ASTM D 2172 using reagent grade trichloroethylene. The asphalt recovery shall be performed in accordance with AASHTO R 59 orASTM D 1856. The recovered asphalt residue shall be tested in accordance with AASHTO R 29 to determine the asphalt binder grade in accordance with Section 9-02.1(4). Once the recovered asphalt binder grade is determined the percent of recycling agent and/or grade of new asphalt binder shall be determined in accordance with ASTM D 4887. The final blend of recycling agent, recovered and new asphalt shall be tested in accordance with AASHTO R 29 to confirm that it meets the grade of asphalt binder required by the contract in accordance with Section 9-02.1(4). All recovered and blended asphalt binder test data shall be reported to the Contracting Agency prior to submitting the mix design for approval on the QPL.

5-04.3(7)A2 Statistical or Nonstatistical Evaluation

This section is revised to read:

The Contractor shall submit WSDOT Form 350-041EF to the Engineer for approval to use a mix design from the QPL. The Contractor may include changes to the job mix formula that have been approved on other contracts. The request to use a mix design from the QPL may be rejected if production of the HMA from another contract is not in compliance with Section 5-04.3(11)D.

The Contractor shall submit representative samples of the materials that are to be used in the HMA production to the State Materials Laboratory in Tumwater. For HMA mix designs with 20 percent RAP or less by total weight of HMA and no RAS, the Contractor shall submit representative samples of the mineral materials that are to be used in the HMA production; the submittal of RAP samples is not required for these mix designs. For HMA mix designs with greater than 20 percent RAP by total weight of HMA or any amount of RAS the Contractor shall submit representative samples of the mineral materials, RAP, RAS and 100 grams of recovered asphalt residue from the RAP and RAS that are to be used in the HMA production. The Contracting Agency will use these samples to evaluate the mix design for approval on the QPL in accordance with WSDOT Standard Practice QC-8.

5-04.3(7)A3 Commercial Evaluation

This section is revised to read:

Approval of a Commercial Evaluation mix design for listing on the QPL will be based on a review of the Contractor's submittal of WSDOT Form 350-042 for conformance to the requirements of Section 9-03.8(2). Testing of the HMA by the Contracting Agency for mix design approval is not required. Mix designs for HMA with greater than 20 percent RAP by total weight of HMA or any amount of RAS may be evaluated in accordance with Section 5-04.3(7)A2.

For the Bid item Commercial HMA, the Contractor shall select a class of HMA and design level of Equivalent Single Axle Loads (ESAL's) appropriate for the required use.

5-04.3(8) Mixing

The first sentence of the second paragraph is revised to read:

When discharged, the temperature of the HMA shall not exceed the optimum mixing temperature by more than 25°F as shown on the reference mix design report or as approved by the Engineer.

The last paragraph is supplemented with the following new sentence:

After the required amount of mineral materials, RAP, RAS, new asphalt binder and asphalt rejuvenator have been introduced into the mixer the HMA shall be mixed until complete and uniform coating of the particles and thorough distribution of the asphalt binder throughout the mineral materials, RAP and RAS is ensured.

5-04.3(8)A4 Definition of Sampling and Sublot

The second sentence of the second paragraph is revised to read:

The sublots shall be approximately uniform in size with a maximum sublot size based on original Plan quantity tons as specified in the following table.

This section is supplemented with the following new table:

HMA Original Plan Quantity (tons)	Sublot Size (tons)
<20,000	1,000
20,000 to 30,000	1,500
>30,000	2,000

5-04.3(8)A7 Test Section – HMA Mixtures

This section is revised to read:

For each class of HMA accepted by statistical evaluation with 20 percent RAP or less by total weight of HMA and no RAS, the Contractor may request a single test section to determine whether the mixture meets the requirements of Section 9-03.8(2) and 9-03.8(6). For each HMA mix design accepted by statistical evaluation with greater than 20 percent RAP by weight of HMA or any amount of RAS, the Contractor shall construct a test section to determine whether the mixture meets the requirements of Sections 9-03.8(2) and 9-03.8(6). Test sections shall be constructed at the beginning of paving and will be at least 600 tons and a maximum of 1,000 tons or as approved by the Engineer. For a test section to be acceptable the pay factor (PF) for gradation, asphalt binder and Va shall be 0.95 or greater for each constituent and the remaining test requirements in Section 9-03.8(2) (dust/asphalt ratio, sand equivalent, uncompacted void and fracture)

shall conform to the requirements of that section. No further wearing or leveling HMA will be paved on any of the four calendar days following construction of the test section. The mixture in the test section will be evaluated as a lot with a minimum of three sublots required. If more than one test section is required, each test section shall be a separate lot.

5-04.3(10)A General

In the first paragraph, “checking” and “cracking” are deleted.

In the third paragraph, the following new sentence is inserted after the second sentence:

Coverage with a steel wheel roller may precede pneumatic tired rolling.

In the third paragraph, the following new sentence is inserted before the last sentence:

Regardless of mix temperature, a roller shall not be operated in a mode that results in checking or cracking of the mat.

5-04.3(10)B1 General

In this section, “Project Engineer” is revised to read “Engineer”.

The first paragraph is revised to read:

HMA mixture accepted by statistical or nonstatistical evaluation that is used in traffic lanes, including lanes for ramps, truck climbing, weaving, and speed change, and having a specified compacted course thickness greater than 0.10-foot, shall be compacted to a specified level of relative density. The specified level of relative density shall be a Composite Pay Factor (CPF) of not less than 0.75 when evaluated in accordance with Section 1-06.2, using a minimum of 91 percent of the maximum density. The percent of maximum density shall be determined by WSDOT FOP for AASHTO T 729 when using the nuclear density gauge and WSDOT SOP 736 when using cores to determine density. The specified level of density attained will be determined by the statistical evaluation of the density of the pavement.

The following four new paragraphs are inserted after the first paragraph:

Tests for the determination of the pavement density will be taken in accordance the required procedures for measurement by a nuclear density gauge or roadway cores after completion of the finish rolling.

If the Contracting Agency uses a nuclear density gauge to determine density the test procedures FOP for WAQTC TM 8 and WSDOT SOP T 729 will be used on the day the mix is placed.

Roadway cores for density may be obtained by either the Contracting Agency or the Contractor in accordance with WSDOT SOP 734. The core diameter shall be 4-inches unless otherwise approved by the Engineer. Roadway cores will be tested by the Contracting Agency in accordance with WSDOT FOP for AASHTO T 166.

If the Contract includes the Bid item “Roadway Core” the cores shall be obtained by the Contractor in the presence of the Engineer on the same day the mix is placed and at

locations designated by the Engineer. If the Contract does not include the Bid item "Roadway Core" the Contracting Agency will obtain the cores.

In the sixth paragraph (after the preceding Amendments are applied), the second sentence is revised to read:

Sublots will be uniform in size with a maximum sublot size based on original Plan quantity tons of HMA as specified in the table below.

The following new table is inserted before the second to last paragraph:

HMA Original Plan Quantity (tons)	Sublot Size (tons)
<20,000	100
20,000 to 30,000	150
>30,000	200

5-04.3(10)B4 Test Results

The first paragraph is revised to read:

The results of all compaction acceptance testing and the CPF of the lot after three sublots have been tested will be available to the Contractor through WSDOT's website. Determination of the relative density of the HMA with a nuclear density gauge requires a correlation factor and may require resolution after the correlation factor is known. Acceptance of HMA compaction will be based on the statistical evaluation and CPF so determined.

In the second paragraph, the first sentence is revised to read:

For a sublot that has been tested with a nuclear density gauge that did not meet the minimum of 91 percent of the reference maximum density in a compaction lot with a CPF below 1.00 and thus subject to a price reduction or rejection, the Contractor may request that a core be used for determination of the relative density of the sublot.

In the second sentence of the second paragraph, "moisture-density" is revised to read "density".

In the second paragraph, the fourth sentence is deleted.

5-04.3(20) Anti-Stripping Additive

This section is revised to read:

Anti-stripping additive shall be added to the liquid asphalt by the asphalt supplier prior to shipment to the asphalt mixing plant. Anti-stripping additive shall be added in the amount designated on the QPL for the mix design.

5-04.4 Measurement

The following new paragraph is inserted after the first paragraph:

Roadway cores will be measured per each for the number of cores taken.

The second to last paragraph is deleted.

5-04.5 Payment

The bid item "Removing Temporary Pavement Marking", per linear foot and paragraph following bid item are deleted.

The following new bid item is inserted before the second to last paragraph:

"Roadway Core", per each.

The Contractor's costs for all other Work associated with the coring (e.g., traffic control) shall be incidental and included within the unit Bid price per each and no additional payments will be made.

Section 5-05, Cement Concrete Pavement
April 6, 2015

5-05.3(1) Concrete Mix Design for Paving

In item number 1, the first sentence of the third paragraph is revised to read:

Ground granulated blast furnace slag, if used, shall not exceed 30 percent by weight of the total cementitious material and shall conform to Section 9-23.10.

The second and third rows of the table in item number 3 are revised to read:

Coarse Aggregate	+ 30 Pounds	- 30 Pounds
Fine Aggregate	+ 30 Pounds	- 30 Pounds

5-05.4 Measurement

The fourth paragraph is supplemented with the following new sentence:

Tie bars with drill holes in cement concrete pavement placed under the Contract will not be measured.

5-05.5 Payment

The paragraph following the Bid item "Tie Bar with Drill Hole", per each is supplemented with the following new sentence:

All costs for tie bars with drill holes in cement concrete pavement placed under the Contract shall be included in the unit Contract price per cubic yard for "Cement Conc. Pavement".

Section 6-01, General Requirements for Structures
January 5, 2015

6-01.6 Load Restrictions on Bridges Under Construction

The first sentence of the second paragraph is revised to read:

If necessary and safe to do so, and if the Contractor requests it through a Type 2E Working Drawing, the Engineer may allow traffic on a bridge prior to completion.

In the second paragraph, item number 3 (up until the colon) is revised to read:

3. Provide stress calculations under the design criteria specified in the AASHTO LRFD Bridge Design Specifications, current edition, including at a minimum the following:

6-01.9 Working Drawings

This section is revised to read:

All Working Drawings required for bridges and other Structures shall conform to Section 1-05.3.

6-01.10 Utilities Supported by or Attached to Bridges

In the second paragraph, “bridge structures” is revised to read “bridges”.

6-01.14 Premolded Joint Filler

In the second paragraph, the first sentence is revised to read:

The Contractor may substitute for the nails any adhesive acceptable to the Engineer.

Section 6-02, Concrete Structures
April 6, 2015

6-02.3(1) Classification of Structural Concrete

In paragraph two, item number 1 is revised to read:

Mix design and proportioning specified in Sections 6-02.3(2), 6-02.3(2)A and 6-02.3(2)A1.

Item number 3 is renumbered to 4.

After the preceding Amendments are applied, the following new numbered item is inserted after item number 2:

3. Temperature and time for placement requirements specified in Section 6-02.3(4)D.

6-02.3(2) Proportioning Materials

In the third paragraph, the first sentence is revised to read:

The use of fly ash is required for Class 4000P concrete, except that ground granulated blast furnace slag may be substituted for fly ash at a 1:1 ratio.

In the table titled “Cementitious Requirement for Concrete”, the row beginning with “4000D” is deleted.

The fourth paragraph is revised to read:

When both ground granulated blast furnace slag and fly ash are included in the concrete mix, the total weight of both these materials is limited to 40 percent by weight of the total cementitious material for concrete class 4000A, and 50 percent by weight of the total cementitious material for all other classes of concrete.

6-02.3(2)A Contractor Mix Design

The first paragraph is revised to read:

The Contractor shall provide a mix design in writing to the Engineer for all classes of concrete specified in the Plans except for lean concrete and commercial concrete. No concrete shall be placed until the Engineer has reviewed the mix design. The required average 28-day compressive strength shall be selected in accordance with ACI 301, Chapter 4, Section 4.2.3.3. ACI 211.1 shall be used to determine proportions. All proposed concrete mixes except Class 4000D shall meet the requirements in Cementitious Requirement for Concrete in Section 6-02.3(2).

In the fourth paragraph, the fourth sentence is deleted.

In the sixth paragraph, the first sentence is deleted.

In the seventh paragraph, the last sentence is deleted.

The eighth paragraph is revised to read:

Air content for concrete Class 4000D shall conform to Section 6-02.3(2)A1. For all other concrete, air content shall be a minimum of 4.5 percent and a maximum of 7.5 percent for all concrete placed above the finished ground line.

The following new sub-section is added:

6-02.3(2)A1 Contractor Mix Design for Concrete Class 4000D

All Class 4000D concrete shall be a project specific performance mix design conforming to the following requirements:

1. Aggregate shall use combined gradation in accordance with Section 9-03.1(5) with a nominal maximum aggregate size of 1-1/2 inches.

2. Permeability shall be less than 2,000 coulombs at 56 days in accordance with AASHTO T 277.

3. Freeze-thaw durability shall be provided by one of the following methods:
   a. The concrete shall maintain an air content between 4.5 and 7.5 percent.
   b. The concrete shall maintain a minimum air content that achieves a durability factor of 90 percent, minimum, after 300 cycles in accordance with AASHTO T 161, Procedure A. This air content shall not be less than 3.0 percent. Test samples shall be obtained from concrete batches of a minimum of 3.0 cubic yards.

4. Scaling shall have a visual rating less than or equal to 2 after 50 cycles in accordance with ASTM C 672.

5. Shrinkage at 28 days shall be less than 320 micro strain in accordance with AASHTO T 160.

6. Modulus of elasticity shall be measured in accordance with ASTM C 469.

7. Density shall be measured in accordance with ASTM C 138.

The Contractor shall submit the mix design in accordance with Section 6-02.3(2)A. The submittal shall include test reports for all tests listed above that follow the reporting requirements of the AASHTO/ASTM procedures. Samples for testing may be obtained from either laboratory or concrete plant batches. If concrete plant batches are used, the minimum batch size shall be 3.0 cubic yards. The Contractor shall submit the mix design to the Engineer at least 30 calendar days prior to the placement of concrete in the bridge deck.

6-02.3(4)D Temperature and Time For Placement

The first two sentences are revised to read:

Concrete temperatures shall remain between 55ºF and 90ºF while it is being placed, except that Class 4000D concrete temperatures shall remain between 55°F and 75°F during placement. Precast concrete that is heat cured in accordance with Section 6-02.3(25)D shall remain between 50ºF and 90ºF while being placed.

6-02.3(5)A General

The first paragraph is revised to read:

Concrete for the following applications will be accepted based on a Certificate of Compliance to be provided by the supplier as described in Section 6-02.3(5)B:

1. Lean concrete.
2. Commercial concrete.
3. Class 4000P concrete for Roadside Steel Sign Support Foundations.
4. Class 4000P concrete for Type II, III, and CCTV Signal Standard Foundations that are 12'-0" or less in depth.
5. Class 4000P concrete for Type IV and V Strain Pole Foundations that are 12'-0" or less in depth.
6. Class 4000P concrete for Steel Light Standard Foundations Types A & B.

The following new sentence is inserted at the beginning of the second paragraph:

Slip-form barrier concrete will be accepted based on conformance to the requirements for temperature, air content and compressive strength at 28 days for sublots as tested and determined by the Contracting Agency.

6-02.3(5)B Certification of Compliance

In the list within the first paragraph, "Fly ash (if used) brand and Type" is revised to read "Fly ash (if used) brand and Class".

The first sentence of the second to last paragraph is deleted.

6-02.3(5)G Sampling and Testing Frequency for Temperature, Consistency, and Air Content

In the fifth sentence of the second paragraph, "five truck loads" is revised to read "ten truck loads".

The second paragraph is supplemented with the following:

If the remaining quantity to be placed is less than ten truck loads; then a sample shall be randomly taken from one of the remaining truck loads.

In the last sentence of the third paragraph, “five truck loads” is revised to read “ten truck loads”.

6-02.3(5)H Sampling and Testing for Compressive Strength and Initial Curing

The second paragraph is revised to read:

The Contractor shall provide and maintain a sufficient number of cure boxes in accordance with WSDOT FOP for AASHTO T 23 for curing concrete cylinders. The cure boxes shall be readily accessible and no more than 500 feet from the point of acceptance testing, unless otherwise approved by the Engineer. The Contractor shall also provide, maintain and operate all necessary power sources and connections needed to operate the cure boxes. The cure boxes shall be in-place and functioning at the specified temperature for curing cylinders prior to concrete placement. Concrete cylinders shall be cured in the cure boxes in accordance with WSDOT FOP for AASHTO T 23. The cure boxes shall have working locks and the Contractor shall provide the Engineer with one key to each of the locks. Once concrete cylinders are placed in the cure box, the cure box shall not be disturbed until the cylinders have been removed. The Contractor shall retain the cure box Temperature Measuring Device log and provide it to the Engineer upon request.

The following new paragraph is inserted after the last paragraph:

All cure box costs shall be incidental to the associated item of work.

6-02.3(6)A2 Cold Weather Protection

The first sentence in the first paragraph is revised to read:

This Specification applies when the weather forecast on the day of concrete placement predicts air temperatures below 35°F at any time during the 7 days following placement.

The first sentence of the second paragraph is revised to read:

The temperature of the concrete shall be maintained above 50°F during the entire curing period or 7 days, whichever is greater.

6-02.3(10)A Preconstruction Meeting

This section including title is revised to read:

6-02.3(10)A Pre-Deck Pour Meeting

A pre-deck pour meeting shall be held 5 to 10 working days before placing deck concrete to discuss construction procedures, personnel, equipment to be used, concrete sampling and testing and deck finishing and curing operations. Those attending shall include, at a minimum, the superintendent, foremen in charge of placing and finishing concrete, and representatives from the concrete supplier and the concrete pump truck supplier.

If the project includes more than one bridge deck, and if the Contractor's key personnel change between concreting operations, or at request of the Engineer, additional conferences shall be held before each deck placement.

6-02.3(10)D Concrete Placement, Finishing, and Texturing

This section's content is deleted and replaced with the following new sub-sections:

6-02.3(10)D1 Test Slab Using Bridge Deck Concrete

After the Contractor receives the Engineer's approval for the Class 4000D concrete mix design, and a minimum of seven calendar days prior to the first placement of bridge deck concrete, the Contractor shall construct a test slab using concrete of the approved mix design.

The test slab may be constructed on grade, shall have a minimum thickness of eight-inches, shall have minimum plan dimensions of 10-feet along all four edges, and shall be square or rectangular.

During construction of the test slab, the Contractor shall demonstrate concrete sampling and testing, use of the concrete temperature monitoring system, the concrete fogging system, concrete placement system, and the concrete finishing operation. The Contractor shall conduct the demonstration using the same type of equipment to be used for the production bridge decks, except that the Contractor may elect to finish the test slab with a hand-operated strike-board.

After the construction of the test slab and the demonstration of bridge deck construction operations is complete, the Contractor shall remove and dispose of the test slab in accordance with Sections 2-02.3 and 2-03.3(7)C.

6-02.3(10)D2 Preparation for Concrete Placement

Before placing bridge approach slab concrete, the subgrade shall be constructed in accordance with Sections 2-06 and 5-05.3(6).

Before any concrete is placed, the finishing machine shall be operated over the entire length of the deck/slab to check screed deflection. Concrete placement may begin only if the Engineer approves after this test.

Immediately before placing concrete, the Contractor shall check (and adjust if necessary) all falsework and wedges to minimize settlement and deflection from the added mass of the concrete deck/slab. The Contractor shall also install devices, such as telltales, by which the Engineer can readily measure settlement and deflection.

6-02.3(10)D3 Concrete Placement

The placement operation shall cover the full width of the bridge deck or the full width between construction joints. The Contractor shall locate any construction joint over a beam or web that can support the deck/slab on either side of the joint. The joint shall not occur over a pier unless the Plans permit. Each joint shall be formed vertically and in true alignment. The Contractor shall not release falsework or wedges supporting bridge deck placement sections on either side of a joint until each side has aged as these Specifications require.

Placement of concrete for bridge decks and bridge approach slabs shall comply with Section 6-02.3(6). In placing the concrete, the Contractor shall:

1. Place it (without segregation) against concrete placed earlier, as near as possible to its final position, approximately to grade, and in shallow, closely spaced piles;

2. Consolidate it around reinforcing steel by using vibrators before strike-off by the finishing machine;

3. Not use vibrators to move concrete;

4. Not revibrate any concrete surface areas where workers have stopped prior to screeding;

5. Remove any concrete splashed onto reinforcing steel in adjacent segments before concreting them;

6. Maintain a slight excess of concrete in front of the screed across the entire width of the placement operation;

7. Operate the finishing machine to create a surface that is true and ready for final finish without overfinishing or bringing excessive amounts of mortar to the surface; and

8. Leave a thin, even film of mortar on the concrete surface after the last pass of the finishing machine pan.

Workers shall complete all post screeding operations without walking on the concrete. This may require work bridges spanning the full width of the deck/slab.

After removing the screed supports, the Contractor shall fill the voids with concrete (not mortar).

If the surface left by the finishing machine is porous, rough, or has minor irregularities, the Contractor shall float the surface of the concrete. Floating shall leave a smooth and even surface. Float finishing shall be kept to the minimum number of passes necessary to seal the surface. The floats shall be at least 4-feet long. Each transverse pass of the float shall overlap the previous pass by at least half the length of the float. The first floating shall be at right angles to the strike-off. The second floating shall be at right angles to the centerline of the span. A smooth riding surface shall be maintained across construction joints.

The edge of completed roadway slabs at expansion joints and compression seals shall have a 3/8-inch radius.

After floating, but while the concrete remains plastic, the Contractor shall test the entire deck/slab for flatness (allowing for crown, camber, and vertical curvature). The testing shall be done with a 10-foot straightedge held on the surface. The straightedge shall be advanced in successive positions parallel to the centerline, moving not more than one half the length of the straightedge each time it advances. This procedure shall be repeated with the straightedge held perpendicular to the centerline. An acceptable surface shall be one free from deviations of more than 1/8-inch under the 10-foot straightedge.

If the test reveals depressions, the Contractor shall fill them with freshly mixed concrete, strike off, consolidate, and refinish them. High areas shall be cut down and refinished. Retesting and refinishing shall continue until a surface conforming to the requirements specified above is produced.

6-02.3(10)D4 Monitoring Bridge Deck Concrete Temperature After Placement

The Contractor shall monitor and record the concrete temperature and ambient temperature hourly for seven calendar days after placement. The Contractor shall monitor and record concrete temperature by placing two maturity meter temperature monitoring devices in the bridge deck at locations specified by the Engineer. The Contractor shall monitor ambient temperature using maturity meters near the locations where concrete temperature is being monitored. When the bridge deck is being enclosed and heated to meet cold weather requirements, ambient temperature readings shall be taken within the enclosure. The Contractor shall submit the concrete temperature and ambient temperature data to the Engineer in spreadsheet format within 14 calendar days from placing the bridge deck concrete.

The Contractor shall submit the type and model of maturity meter temperature monitoring device, and the associated devices responsible for recording and documenting the temperature and curing time, to the Engineer at least 14 calendar days prior to the pre-concreting conference for the first bridge deck to be cast. The placement and operation of the temperature monitoring devices and associated devices will be an agenda item at the pre-concreting conference for the first bridge deck to be cast.

6-02.3(10)D5 Bridge Deck Concrete Finishing and Texturing

Except as otherwise specified for portions of bridge decks receiving an overlay or sidewalk under the same Contract, the Contractor shall texture the surface of the bridge deck as follows:

The Contractor shall texture the bridge deck using diamond tipped saw blades mounted on a power driven, self-propelled machine that is designed to texture concrete surfaces. The grooving equipment shall provide grooves that are 1/8" ± 1/64" wide, 3/16" ± 1/16" deep, and spaced at 3/4" ± 1/8". The bridge deck shall not be textured with a metal tined comb.

The Contractor shall submit the type of grooving equipment to be used to the Engineer for approval 30 calendar days prior to performing the work. The Contractor shall demonstrate that the method and equipment for texturing the bridge deck will not chip, spall or otherwise damage the deck. The Contractor shall not begin texturing the bridge deck until receiving the Engineer's approval of the Contractor's method and equipment.

Unless otherwise approved by the Engineer, the Contractor shall texture the concrete bridge deck surface either in a longitudinal direction, parallel with centerline or in a transverse direction, perpendicular with centerline. The Contractor shall texture the bridge deck surface to within 3-inches minimum and 15-inches maximum of the edge of concrete at expansion joints, within 1-foot minimum and 2-feet maximum of the curb line, and within 3-inches minimum and 9-inches maximum of the perimeter of bridge drain assemblies.

The Contractor shall contain and collect all concrete dust and debris generated by the bridge deck texturing process, and shall dispose of the collected concrete dust and debris in accordance with Section 2-03.3(7)C.

If the Plans call for placement of a sidewalk or an HMA or concrete overlay on the bridge deck, the Contractor shall produce the final finish of these areas by dragging a strip of damp, seamless burlap lengthwise over the bridge deck or by brooming it lightly. Approximately 3-feet of the drag shall contact the surface, with the least possible bow in its leading edge. It shall be kept wet and free of hardened lumps of concrete. When the burlap drag fails to produce the required finish, the Contractor shall replace it. When not in use, it shall be lifted clear of the bridge deck.

After the bridge deck has cured, the surface shall conform to the surface smoothness requirements specified in Section 6-02.3(10)D3.

The surface texture on any area repaired to address out-of-tolerance surface smoothness shall match closely that of the surrounding bridge deck area at the completion of the repair. Methods used to remove high spots shall cut through the mortar and aggregate without breaking or dislodging the aggregate or causing spalls.

6-02.3(10)D6 Bridge Approach Slab Finishing and Texturing

Bridge approach slabs shall be textured either in accordance with Section 6-02.3(10)D5, or using metal tined combs in the transverse direction, except bridge approach slabs receiving an overlay in the same Contract shall be finished as specified in Section 6-02.3(10)D5 only.

The comb shall be made of a single row of metal tines. It shall leave striations in the fresh concrete approximately 3/16-inch deep by 1/8-inch wide and spaced approximately 1/2-inch apart. The Engineer will decide actual depths at the site. If the comb has not been approved, the Contractor shall obtain the Engineer's approval by demonstrating it on a test section. The Contractor may operate the combs manually or mechanically, either singly or with several placed end to end. The timing and method used shall produce the required texture without displacing larger particles of aggregate.

Texturing shall end 2-feet from curb lines. This 2-foot untextured strip shall be hand finished with a steel trowel.

Surface smoothness, high spots, and low spots shall be addressed as specified in Section 6-02.3(10)D5. The surface texture on any area cut down or built up shall match closely that of the surrounding bridge approach slab area. The entire bridge approach slab shall provide a smooth riding surface.

6-02.3(10)F Bridge Approach Slab Orientation and Anchors

In the first paragraph, the following sentence is inserted after the first sentence:

Unless otherwise shown in the Plans, the pavement end of the bridge approach slab shall be constructed normal to the Roadway centerline.

The following new paragraph is inserted before the last paragraph:

The compression seal shall be a 2-1/2 inch wide gland selected from the current Qualified Products List.

6-02.3(11) Curing Concrete

Items number 1 through 4 are deleted and replaced with the following 5 new numbered items:

1. Bridge sidewalks, roofs of cut and cover tunnels — curing compound covered by white, reflective type sheeting or continuous wet curing. Curing by either method shall be for at least 10 days.

2. Bridge decks — See Section 6-02.3(11)B.

3. Bridge approach slabs (Class 4000A concrete) - 2 coats of curing compound and continuous wet cure for at least 10-days.

4. Concrete barriers and rail bases – See Section 6-02.3(11)A.

5. All other concrete surfaces — continuous wet cure for at least three days.

In the second paragraph, the first sentence is replaced with the following three new sentences:

During the continuous wet cure, the Contractor shall keep all exposed concrete surfaces saturated with water. Formed concrete surfaces shall be kept in a continuous wet cure by leaving the forms in place. If forms are removed during the continuous wet cure period, the Contractor shall treat the concrete as an exposed concrete surface.

The third paragraph is revised to read:

When curing Class 4000A, two coats of curing compound that complies with Section 9-23.2 shall be applied immediately (not to exceed 15 min.) after tining any portion of the bridge approach slab. The continuous wet cure shall be established as soon as the concrete has set enough to allow covering without damaging the finish.

In the fifth paragraph, the first sentence is revised to read:

If the Plans call for an asphalt overlay on the bridge approach slab, the Contractor shall use the clear curing compound (Type 1, Class B), applying at least 1 gallon per 150 square feet to the concrete surface.

The eighth paragraph is deleted.

6-02.3(11)A2 Slip-Form Barrier

In the fourth paragraph, item number 1, “Type 1D” is revised to read “Type 1”.

6-02.3(11)B Curing Bridge Decks

This new section is supplemented with the following new sub-sections:

6-02.3(11)B1 Equipment

The Contractor shall maintain a wet sheen, without developing pooling or sheeting water, using a fogging apparatus consisting of pressure washers with a minimum nozzle output of 1,500 psi, or other means approved by the Engineer.

The Contractor shall submit a bridge deck curing plan to the Engineer a minimum 14 calendar days prior to the pre-concreting conference. The Contractor's plan shall describe the sequence and timing that will be used to fog the bridge deck, apply pre-soaked burlap, install soaker hoses and cover the deck with white reflective sheeting.

6-02.3(11)B2 Curing

The fogging apparatus shall be in place and charged for fogging prior to beginning concrete placement for the bridge deck.

The Contractor shall presoak all burlap to be used to cover the deck during curing.

Immediately after the finishing machine passes over finished concrete, the Contractor shall implement the following tasks:

1. The Contractor shall fog the bridge deck while maintaining a wet sheen without developing pooling or sheeting water.

2. The Contractor shall apply the presoaked burlap to the top surface to fully cover the deck without damaging the finish, other than minor marring of the concrete surface. The Contractor shall not apply curing compound.

3. The Contractor shall continue to keep the burlap wet by fog spraying until the burlap is covered by soaker hoses and white reflective sheeting. The Contractor shall place the soaker hoses and whiter reflective sheeting after the concrete has achieved initial set. The Contractor shall charge the soaker hoses frequently so as to keep the burlap covering the entire deck wet during the course of curing.

As an alternative to tasks 2 and 3 above, the Contractor may propose a curing system using proprietary curing blankets specifically manufactured for bridge deck curing. Details of the proprietary curing blanket system, including product literature and details of how the system is to be installed and maintained, shall be submitted to the Engineer for approval.

The wet curing regime as described shall remain in place for at least 14 consecutive calendar days.

6-02.3(12)A Construction Joints in New Construction

The third paragraph is deleted and replaced with the following three new paragraphs:

If the Plans require a roughened surface on the joint, the Contractor shall strike it off to leave grooves at right angles to the length of the member. Grooves shall be installed using one of the following options:

1. Grooves shall be ⅛ to 1 inch wide, ¼ to ½ inch deep, and spaced equally at twice the width of the groove. Grooves shall terminate approximately 1 ½-inches from the face of concrete.

2. Grooves shall be 1 to 2 inches wide, a minimum of ½-inch deep, and spaced a maximum of three times the width of the groove. Grooves shall terminate approximately 1 ½-inches from the face of concrete.

If the Engineer approves, the Contractor may use an alternate method to produce a roughened surface on the joint, provided that such an alternate method leaves a roughened surface of at least a ¼-inch amplitude.

If the first strike-off does not produce the required roughness, the Contractor shall repeat the process before the concrete reaches initial set. The final surface shall be clean and without laitance or loose material.

6-02.3(12)B Construction Joints Between Existing and New Construction

The phrase "by method(s) as approved by the Engineer" is deleted from each paragraph in this section.

6-02.3(13) Expansion Joints

The first sentence of the second paragraph is revised to read:

Joints made of a vulcanized, elastomeric compound (with neoprene as the only polymer) shall be installed with a lubricant adhesive as recommended by the manufacturer.

In the third paragraph, "injuring" is revised to read "damaging".

The following two new subsections are added:

6-02.3(13)A Strip Seal Expansion Joint System

The Contractor shall submit Working Drawings consisting of the strip seal expansion joint shop drawings in accordance with Section 6-03.3(7). These plans shall include, at a minimum, the following:

1. Plan, elevation, and sections of the joint system and all components, with dimensions and tolerances.

2. All material designations.

3. Manufacturer's written installation procedure.

4. Corrosion protection system used on the metal components.

5. Locations of welded shear studs, lifting mechanisms, temperature setting devices, and construction adjustment devices.

6. Method of sealing the system to prevent leakage of water through the joint.

The strip seal shall be removable and replaceable.

The metal components shall conform to ASTM A 36, ASTM A 992, or ASTM A 572, and shall be protected against corrosion by one of the following methods:

1. Zinc metallized in accordance with Section 6-07.3(14).

2. Hot-dip galvanized in accordance with AASHTO M 111.

3. Paint in accordance with Section 6-07.3(9). The color of the top coat shall be Federal Standard 595 Color No. 26420. The surfaces embedded in concrete shall be painted only with a shop primer coat of paint conforming to Section 9-08.1(2)C.

The strip seal gland shall be continuous for the full length of the joint with no splices permitted, unless otherwise shown in the Plans.

Other than items shown in the Plans, threaded studs used for construction adjustments are the only items that may be welded to the steel shapes provided they are removed by grinding after use, and the area repaired by application of an approved corrosion protection system.

If the opening between the steel shapes is anticipated to be less than 1-1/2 inches at the time of seal installation, the seal may be installed prior to encasement of the steel shapes in concrete.

After the joint system is installed, the joint shall be flooded with water and inspected, from below the joint, for leakage. If leakage is observed, the joint system shall be repaired by the Contractor, as recommended by the manufacturer.

6-02.3(13)B Compression Seal Expansion Joint System

Compression seal glands shall be selected from the current Qualified Products List and sized as shown in the Plans.

The compression seal expansion joint system shall be installed in accordance with the manufacturer's written recommendations. The Contractor shall submit a Type 1 Working Drawing consisting of the manufacturer's written installation procedure and repair procedures if leakage testing fails.

After the joint system is installed, the joint area shall be flooded with water and inspected, from below the joint, for leakage. If leakage is observed, the joint system shall be repaired by the Contractor, as recommended by the manufacturer.

6-02.3(14) Finishing Concrete Surfaces

The last sentence of the first paragraph is revised to read:

The Contractor shall clean and refinish any stained or discolored surfaces.

The following new subsection is added:

6-02.3(14)D General Requirements for Concrete Surface Finishes Produced by Form Liners

Horizontal and vertical joints shall be spliced in accordance with the manufacturer's printed instructions. The Contractor shall submit a Type 1 Working Drawing consisting of the manufacturer’s joint splice instructions.

Horizontal splicing of ABS and plastic form liners to achieve the required height is not permitted and there shall be no horizontal joints. The concrete formed with ABS and plastic form liners shall be given a light sandblast to remove the glossy finish.

Side forms, traffic barrier forms, and pedestrian barrier forms using these form liners may be removed after 24 hours provided the concrete mix used includes a water-reducing admixture, and the concrete reaches 1,400 psi minimum compressive strength before form removal. Concrete in load supporting forms utilizing these form liners shall be cured in accordance with Section 6-02.3(17)N. Once the forms are removed, the Contractor shall treat the joint areas by patching or light sandblasting as required by the Engineer to ensure that the joints are not visible.

Form liners shall be cleaned, reconditioned, and repaired before each use. Form liners with repairs, patches, or defects which, in the opinion of the Engineer, would result in adverse effects to the concrete finish shall not be used.

Care shall be taken to ensure uniformity of color throughout the textured surface. A change in form release agent will not be allowed.

All surfaces formed by the form liner shall also receive a Class 2 surface finish. Form ties shall be a type that leaves a clean hole when removed. All spalls and form tie holes shall be filled as specified for a Class 2 surface finish.

6-02.3(14)C Pigmented Sealer for Concrete Surfaces

The first sentence (up until the colon) is revised to read:

The Contractor shall submit a Type 1 Working Drawing consisting of the pigmented sealer manufacturer's written instructions covering, at a minimum, the following:

The second paragraph is deleted.

In the last sentence of the third paragraph, "approval" is revised to read "acceptance".

6-02.3(15) Date Numerals

The third sentence in the first paragraph is revised to read:

When an existing Structure is widened or when traffic barrier is placed on an existing Structure, the date shall be for the year in which the original Structure was completed.

6-02.3(16) Plans for Falsework and Formwork

This section is revised to read:

The Contractor shall submit all plans for falsework and formwork as Type 2E Working Drawings. Submittal is not required for footing or retaining wall formwork if the wall is 4 feet or less in height (excluding pedestal height).

The design of falsework and formwork shall be based on:

1. Applied loads and conditions which are no less severe than those described in Section 6-02.3(17)A, Design Loads;

2. Allowable stresses and deflections which are no greater than those described in Section 6-02.3(17)B, Allowable Stresses and Deflections;

3. Special loads and requirements no less severe than those described in Section 6-02.3(17)C, Falsework and Formwork at Special Locations;

4. Conditions required by other Sections of 6-02.3(17), Falsework and Formwork.

The falsework and formwork plans shall be scale drawings showing the details of proposed construction, including: sizes and properties of all members and components; spacing of bents, posts, studs, wales, stringers, wedges and bracing; rates of concrete placement, placement sequence, direction of placement, and location of construction joints; identification of falsework devices and safe working loads as well as identification of any bolts or threaded rods used with the devices including their diameter, length, type, grade, and required torque. The falsework plans shall show the proximity of falsework to utilities or any nearby Structures including underground Structures. Formwork accessories shall be identified according to Section 6-02.3(17)H, Formwork Accessories. All assumptions, dimensions, material properties, and other data used in making the structural analysis shall be noted on the drawing.

The Contractor shall furnish associated design calculations to the Engineer as part of the submittal. The design calculations shall show the stresses and deflections in load supporting members. Construction details which may be shown in the form of sketches on the calculation sheets shall be shown in the falsework or formwork drawings as well. Falsework or formwork plans will be rejected in cases where it is necessary to refer to the calculation sheets for information needed for complete understanding of the falsework and formwork plans or how to construct the falsework and formwork.

Each sheet of falsework and formwork plans shall carry the following:

1. The initials and dates of all participating design professionals.

2. Clear notation of all revisions including identification of who authorized the revision, who made the revision, and the date of the revision.

3. The Contract number, Contract title, and sequential sheet number. These shall also be on any related documents.

4. Identify where the falsework and formwork plan will be utilized by referencing Contract Plan sheet number and related item or detail.

6-02.3(16)A Nonpreapproved Falsework and Formwork Plans

This section, including title, is deleted in its entirety and replaced with the following:

6-02.3(16)A Vacant

6-02.3(16)B Preapproved Formwork Plans

This section, including title, is revised to read:

6-02.3(16)B Pre-Contract Review of Falsework and Formwork Plans

The Contractor may request pre-contract review of formwork plans for abutments, wingwalls, diaphragms, retaining walls, columns, girders and beams, box culverts, railings, and bulkheads. Plans for falsework supporting the bridge deck for interior spans between precast prestressed concrete girders may also be submitted for pre-contract review.

To obtain pre-contract review, the Contractor shall electronically submit drawings and design calculations in PDF format directly to:

BridgeConstructionSupport@wsdot.wa.gov

The Bridge and Structures Office, Construction Support Engineer will return the falsework or formwork plan to the Contractor with review notes, an effective date of review, and any revisions needed prior to use. For each contract on which the pre-reviewed falsework or formwork plans will be used, the Contractor shall submit a copy to the Engineer. Construction shall not begin until the Engineer has given concurrence.

If the falsework or formwork being constructed has any deviations to the preapproved falsework or formwork plan, the Contractor shall submit plan revisions for review and approval in accordance with Section 6-02.3(16).

6-02.3(17)A Design Loads

The fifth paragraph is revised to read:

Live loads shall consist of a minimum uniform load of not less than 25 psf, applied over the entire falsework plan area, plus the greater of:

1. Actual weights of the deck finishing equipment applied at the rails, or;

2. A minimum load of 75 pounds per linear foot applied at the edge of the bridge deck.

6-02.3(17)J Face Lumber, Studs, Wales, and Metal Forms

The second and third to last paragraphs are deleted.

6-02.3(17)K Concrete Forms on Steel Spans

The second sentence of the last paragraph is revised to read:

The Contractor shall fill the holes with fully torqued ASTM A 325 bolts in accordance with Section 6-03.3(33).

6-02.3(17)O Early Concrete Test Cylinder Breaks

The third paragraph is revised to read:

The cylinders shall be cured in the field in accordance with WSDOT FOP for AASHTO T 23 Section 10.2 Field Curing.

6-02.3(20) Grout for Anchor Bolts and Bridge Bearings

The first five paragraphs are deleted and replaced with the following two new paragraphs:

Grout shall conform to Section 9-20.3(2) for anchor bolts and for bearing assemblies with bearing plates. Grout shall conform to Section 9-20.3(3) for elastomeric bearing pads and fabric pad bearings without bearing plates.

Grout shall be a workable mix with a viscosity that is suitable for the intended application. The Contractor shall receive approval from the Engineer before using the grout.

6-02.3(24)C Placing and Fastening

The twelfth paragraph is revised to read:

> In bridge decks, a "mat" is two adjacent and perpendicular layers of reinforcing steel. Top and bottom mats shall be supported adequately to hold both in their proper positions. If No. 4 bars make up the lower layer of steel in a mat, it shall be blocked at not more than 3-foot intervals (or 4-foot intervals for bars No. 5 and larger). Wire ties to girder stirrups shall not be considered as blocking. To provide a rigid mat, the Contractor shall add other supports and tie wires to the top mat as needed.

In the fourteenth paragraph, the description following "2½ inches between" is revised to read:

> Adjacent bars in a layer. Bridge deck and bridge approach slab bars and the top of the slab.

In the fourteenth paragraph, the description following "2 inches between" is supplemented with the following new sentence:

> Bars and the surface of concrete when not specified otherwise in this Section or in the Plans.

In the fourteenth paragraph, the first sentence in the description following "1½ inches between" is deleted.

The fifteenth paragraph is revised to read:

> Except for top cover in bridge decks and bridge approach slabs, cover to ties and stirrups may be ½ inch less than the values specified for main bars but shall not be less than 1 inch.

In the sixteenth paragraph, the first item in the second subparagraph is revised to read:

> The clearance to the top surface of bridge decks and bridge approach slabs +¼ in/-0".

6-02.3(24)E Welding Reinforced Steel

This section is revised to read:

> Welding of steel reinforcing bars shall conform to the requirements of ANSI/AWS D1.4 Structural Welding Code - Reinforcing Steel, latest edition, except where superseded by the Special Provisions, Plans, and these Specifications.
>
> Before any welding begins, the Contractor shall submit a Type 2 Working Drawing consisting of the welding procedure for each type of welded splice to be used, including the weld procedure specifications and joint details. The weld procedure specifications shall be written on a form taken from AWS D1.4 Annex A, or equivalent. Test results of tensile strength, macroetch, and visual examination shall be included. The form shall be signed and dated.
>
> Welders shall be qualified in accordance with AWS D1.4. The Contractor shall be responsible for the testing and qualification of welders, and shall submit Type 2 Working

Drawings consisting of welder qualification and retention records. The weld joint and welding position a welder is qualified in shall be in accordance with AWS D1.4. The welder qualifications shall remain in effect indefinitely unless, (1) the welder is not engaged in a given process of welding for which the welder is qualified for a period exceeding six months, or (2) there is some specific reason to question a welder's ability.

Filler metals used for welding reinforcing bars shall be in accordance with AWS D1.4 Table 5.1. All filler metals shall be low-hydrogen and handled in compliance with low-hydrogen practices specified in the AWS code.

Short circuiting transfer with gas metal arc welding will not be allowed. Slugging of welds will not be allowed.

For the purpose of compatibility with AWS D1.4, welded lap splices for spiral or hoop reinforcing shall be considered Flare-V groove welds, indirect butt joints.

The Contractor is responsible for using a welding sequence that will limit the alignment distortion of the bars due to the effects of welding. The maximum out-of-line permitted will be 1/4 inch from a 3.5-foot straight-edge centered on the weld and in line with the bar.

The ground wire from the welding machine shall be clamped to the bar being welded.

Where epoxy-coated steel reinforcing bars are specified to be spliced by welding, the epoxy coating shall be left off or removed from the surfaces to be heated, but in no cases less than six inches of each bar being welded. After the welding is complete, the Contractor shall apply epoxy patching material to the uncoated portions of the bar in accordance with Section 6-02.3(24)H.

6-02.3(25) Prestressed Concrete Girders

In the first paragraph, the last sentence is revised to read:

WSDOT certification will be granted at, and renewed during, the annual prestressed plant review and approval process in accordance with WSDOT Materials Manual M 46-01.04 Standard Practice QC 6.

6-02.3(25)I Fabrication Tolerances

In the first paragraph, item number 21 is revised to read:

21. Differential Camber Between Girders in a Span (measured in place at the job site):

For deck bulb tee girders and PCPS members with grouted shear keys:	Cambers shall be equalized when the differences in cambers between adjacent girders exceeds ± ¼ inch
For deck bulb tee girders and PCPS members without grouted shear keys:	Cambers shall be equalized when the differences in cambers between adjacent girders exceeds ± ½ inch
For all other prestressed concrete girders:	± ⅛ inch per 10 feet of girder length

6-02.3(25)O Deck Bulb Tee Girder Flange Connection

This section, including title, is revised to read:

Deck Bulb Tee Girder Flange and PCPS Member Connection

The Contractor shall submit a method of equalizing deflections as a Type 1 Working Drawing. Any temporary strands in the top flange shall be cut per Section 6-02.3(25)N prior to equalizing girder deflections.

Deck bulb tee girders and PCPS members with grouted shear keys shall be constructed in the following sequence:

1. Deflections shall be equalized per the Contractor's equalization plan.

2. Intermediate diaphragms shall be placed and weld ties shall be welded. Welding ground shall be attached directly to the steel plates being welded when welding the weld-ties.

3. The keyways shown in the Plans to receive grout shall be filled flush with the surrounding surfaces using a grout conforming to Section 9-20.3(2).

4. Equalization equipment shall not be removed and other construction equipment shall not be placed on the structure until intermediate diaphragms have attained a minimum compressive strength of 2,500 psi and keyway grout has achieved a minimum compressive strength of 4000 psi.

Deck bulb tee girders and PCPS members without grouted shear keys shall be constructed in the following sequence:

1. Deflections shall be equalized per the Contractor's equalization plan.

2. Intermediate diaphragms shall be placed and weld ties shall be welded. Welding ground shall be attached directly to the steel plates being welded when welding the weld-ties.

3. Equalization equipment shall not be removed and other construction equipment shall not be placed on the structure until intermediate diaphragms have attained a minimum compressive strength of 2,500 psi.

6-02.3(26)F Prestressing Reinforcement

The last sentence in the fourth paragraph is revised to read:

If the prestressing reinforcement will not be stressed and grouted for more than 7 calendar days after it is placed in the ducts, the Contractor shall place an approved corrosion inhibitor conforming to Federal Specification MIL-I-22110C in the ducts.

6-02.3(28) Precast Concrete Panels

In the first paragraph, the third sentence is revised to read:

WSDOT Certification will be granted at, and renewed during, the annual precast plant review and approval process in accordance with WSDOT Materials Manual M 46-01.04 Standard Practice QC 7.

6-02.4 Measurement

The following three new paragraphs are inserted before the last paragraph:

Expansion joint system___seal - superstr. will be measured by the linear foot along its completed line and slope.

Expansion joint modification will be measured by the linear foot of expansion joint modified along its completed line and slope.

Prestressed concrete girder will be measured by the linear foot of girder specified in the Proposal.

6-02.5 Payment

In the paragraph following the bid item “Commercial Concrete”, per cubic yard the second sentence is revised to read:

All costs in connection with concrete curing, producing concrete surface finish with form liners, and furnishing and applying pigmented sealer to concrete surfaces as specified, shall be included in the unit contract price per cubic yard for "Conc. Class ____".

The following new paragraph is inserted after the bid item “Superstructure (name bridge)”, lump sum:

All costs in connection with constructing, finishing and removing the bridge deck test slab as specified in Section 6-02.3(10)D1 shall be included in the lump sum Contract price for “Superstructure___” or “Bridge Deck___” for one bridge in each project, as applicable.

In the paragraph following the bid item “Epoxy-Coated St. Reinf. Bar ___”, per pound, the first sentence is revised to read:

Payment for reinforcing steel shall include the cost of drilling holes in concrete for, and setting, steel reinforcing bar dowels with epoxy bonding agent, and furnishing, fabricating, placing, and splicing the reinforcement.

The bid item “Cure Box”, lump sum and paragraph following bid item are deleted.

The following three new bid items are inserted before the bid item “Bridge Approach Slab”, per square yard:

“Expansion Joint System _____ - Superstr.”, per linear foot.

“Expansion Joint Modification - ___”, per linear foot.

“Prestressed Conc. Girder ___”, per linear foot.

Section 6-03, Steel Structures
April 6, 2015

6-03.2 Materials

The first sentence in the fifth paragraph is revised to read:

The Contractor shall submit Type 1 Working Drawings describing the methods for visibly marking the material so that it can be traced.

6-03.3 Construction Requirements

This section is revised to read:

Structural steel fabricators of plate and box girders, floorbeams, truss members, stringers, cross frames, diaphragms, and laterals shall be certified under the AISC Certification Program for Steel Bridge Fabricators, Advanced Bridges Category. When fracture critical members are specified in the contract, structural steel fabricators shall also meet the supplemental requirements F, Bridges with Fracture-Critical Members, under the AISC Certification Program for Steel Bridge Fabricators.

6-03.3(7) Shop Plans

This section is revised to read:

The Contractor shall submit all shop detail plans for fabricating the steel as Type 2 Working Drawings.

If these plans will be submitted directly from the fabricator, the Contractor shall so notify the Engineer in writing.

No material shall be fabricated until: (1) the Working Drawing review is complete, and (2) the Engineer has accepted the materials source.

Before physical completion of the project, the Contractor shall furnish the Engineer one set of reproducible copies of the as-built shop plans. The reproducible copies shall be clear, suitable for microfilming, and on permanent sheets that measure no smaller than 11 by 17-inches. Alternatively, the shop drawings may be provided in an electronic format with the concurrence of the Engineer.

6-03.3(7)A Erection Methods

The first paragraph is revised to read:

Before beginning to erect any steel Structure, the Contractor shall submit Type 2E Working Drawings consisting of the erection plan and procedure describing the methods the Contractor intends to use.

The second paragraph (up until the colon) is revised to read:

The erection plan and procedure shall provide complete details of the erection process including, at a minimum, the following:

The third paragraph (up until the colon) is revised to read:

As part of the erection plan Working Drawings, the Contractor may submit details of an engineered and fabricated lifting bracket bolted to the girder top flanges providing the following requirements are satisfied:

In the third paragraph, the second sentence of item number 4 is revised to read:

Certification documentation from a previous project may be submitted;

The last sentence of the fourth paragraph is deleted.

The last paragraph is deleted.

6-03.3(10) Straightening Bent Material

In the first paragraph, the last sentence is revised to read:

> A limited amount of localized heat may be applied only if carefully planned and supervised, and only in accordance with the heat-straightening procedure Working Drawing submittal.

The third paragraph is revised to read:

> After straightening, the Contractor shall inspect the member for fractures using a method proposed by the Contractor and accepted by the Contracting Agency.

The last paragraph is revised to read:

> The procedure for heat straightening of universal mill (UM) plates by the mill or the fabricator shall be submitted as a Type 2 Working Drawing.

6-03.3(14) Edge Finishing

In the first paragraph, the last sentence is revised to read:

> Corners along exposed edges shall be broken by light grinding or another method acceptable to the Engineer to achieve an approximate 1/16-inch chamfer or rounding.

In the fifth paragraph, the last sentence is revised to read:

> The fabricator shall prevent excessive hardening of flange edges through preheating, post heating, or control of the burning process as recommended by the steel manufacturer.

The sixth paragraph is revised to read:

> Hardness testing shall consist of testing thermal-cut edges with a portable hardness tester. The hardness tester, and its operating test procedures, shall be submitted as a Type 1 Working Drawing. The hardness tester shall be convertible to Rockwell C scale values.

In the last paragraph, the last sentence is revised to read:

> If thermal-cutting operations conform to procedures established by the steel manufacturer, and hardness testing results are consistently within acceptable limits, the Engineer may authorize a reduction in the testing frequency.

6-03.3(15) Planing of Bearing Surfaces

This section is supplemented with the following new paragraph:

Where mill to bear is specified in the Plans, the bearing end of the stiffener shall be flush and square with the flange and shall have at least 75 percent of this area in contact with the flange.

6-03.3(25) Welding and Repair Welding

In the first paragraph, the first sentence is revised to read:

Welding and repair welding of all steel bridges shall comply with the AASHTO/AWS D1.5M/D1.5, latest edition, Bridge Welding Code.

In the second paragraph, the last sentence is revised to read:

No welding, including tack and temporary welds shall be done in the shop or field unless the location of the welds is shown on the shop drawings reviewed and accepted by the Engineer.

In the third paragraph, the first sentence is revised to read:

Welding procedures shall accompany the shop drawing Working Drawing submittal.

In the fourth paragraph, the first sentence is revised to read:

Welding shall not begin until completion of the shop plan Working Drawing review as required in Section 6-03.3(7).

In item number 1 of the ninth paragraph, “approves” is revised to read “concurs”.

6-03.3(25)A3 Ultrasonic Inspection

The following new paragraph is inserted before the last paragraph:

A minimum of 30 percent of complete penetration vertical welds on steel column jackets thicker than 5/16-inch, within 1.50 column jacket diameter of the top and bottom of each column, shall be inspected. If any rejectable flaws are found, 100 percent of the weld within the specified limits shall be inspected. The largest column cross section diameter for tapered column jackets shall constitute one column jacket diameter.

6-03.3(25)A4 Magnetic Particle Inspection

Items number 3 and 4 are revised to read:

3. Complete penetration groove welds on plates $^{5}/_{16}$-inch or thinner (excluding steel column jackets) shall be 100 percent tested by the magnetic particle method. Testing shall apply to both sides of the weld, if backing plate is not used. The ends of each complete penetration groove weld at plate edges shall be tested by the magnetic particle method.

4. A minimum of 30 percent of complete penetration vertical welds on steel column jackets $^{5}/_{16}$-inch or thinner, within 1.50 column jacket diameters of the top and bottom of each column, shall be magnetic particle inspected. The largest column cross section diameter for tapered column jackets shall constitute one column jacket diameter.

The last paragraph is supplemented with the following new sentence:

If any rejectable flaws are found in any test length of item 4 above, 100 percent of the weld within the specified limits shall be inspected.

6-03.3(27) High Strength Bolt Holes

The last paragraph is revised to read:

The Contractor shall submit Type 2 Working Drawings consisting of a detailed outline of the procedures proposed to accomplish the work from initial drilling through shop assembly.

6-03.3(27)C Numerically Controlled Drilled Connections

In the second paragraph, the first sentence is revised to read:

The Contractor shall submit Type 1 Working Drawings consisting of a detailed outline of proposed N/C procedures.

6-03.3(29) Welded Shear Connectors

This section's content is deleted and replaced with the following:

Installation, production control, and inspection of welded shear connectors shall conform to Chapter 7 of the AASHTO/AWS D1.5M/D1.5:2010 Bridge Welding Code. If welded shear connectors are installed in the shop, installation shall be completed prior to applying the shop primer coat in accordance with Section 6-07.3(9)G. If welded shear connectors are installed in the field, the steel surface to be welded shall be prepared to SSPC-SP 11, power tool cleaning, just prior to welding.

6-03.3(33) Bolted Connections

In the second paragraph, the first sentence is revised to read:

The Contractor shall submit Type 1 Working Drawings providing documentation of the bolt tension calibrator, including brand, capacity, model, date of last calibration, and manufacturer's instructions for use.

In the second sentence of the second paragraph, the word "approved" is deleted.

In item number 3 of the fifth paragraph, "approved" is revised to read "specified".

In the center column header of table 1, "AASHTO M 164" is revised to read "ASTM A 325".

In the column headings of table 3, "M 164" is revised to read "A 325".

In the tenth paragraph, item number 3, "approved" is revised to read "accepted" in the second and third sentences of the first paragraph.

In the tenth paragraph, item number 3, the third paragraph is revised to read:

The Contractor shall submit Type 1 Working Drawings of the tension control bolt assembly, including bolt capacities, type of bolt, nut, and washer lubricant, method of packaging and protection of the lubricated bolt, installation equipment, calibration equipment, and installation procedures.

In the first sentence of the last paragraph, “AASHTO M 164” is revised to read “ASTM A 325”.

The second sentence of the last paragraph is revised to read:

Black ASTM A 325 bolts may be reused once if accepted by the Engineer.

In the last paragraph, the fourth sentence is revised to read:

Bolts to be reused shall be relubricated in accordance with the manufacturer’s recommendations.

6-03.3(33)A Pre-Erection Testing

In the fifth sentence of the first paragraph, “approved” is revised to read “accepted”.

The third paragraph is revised to read:

The Contractor shall submit Type 1 Working Drawings consisting of the manufacturer’s detailed procedure for pre-erection (rotational capacity) testing of tension control bolt assemblies.

6-03.3(33)B Bolting Inspection

In the last sentence of the first paragraph, “approved” is revised to read “specified”.

The last paragraph is revised to read:

The Contractor shall submit Type 1 Working Drawings consisting of the manufacturer’s detailed procedure for routine observation to ensure proper use of the tension control bolt assemblies.

6-03.3(42) Surface Condition

The first subparagraph is revised to read:

Painted steel surfaces shall be cleaned by methods required for the type of staining. The Contractor shall submit a Type 1 Working Drawing of the cleaning method.

Section 6-04, Timber Structures
January 5, 2015

6-04.3(3) Shop Details

This section is revised to read:

The Contractor shall submit Type 2 Working Drawings consisting of shop detail plans for all treated timber. These plans shall show dimensions for all cut, framed, or bored timbers.

Section 6-05, Piling
January 5, 2015

6-05.3(2) Ordering Piling

The last paragraph is deleted.

6-05.3(3)A Casting and Stressing

In the second sentence of the first paragraph, “poured” is revised to read “cast”.

6-05.3(4) Manufacture of Steel Casings for Cast-In-Place Concrete Piles

This section is revised to read:

The diameter of steel casings shall be as specified in the Contract. A full-penetration groove weld between welded edges is required.

6-05.3(5) Manufacture of Steel Piles

This section is revised to read:

Steel piles shall be made of rolled steel H-pile sections, steel pipe piles, or of other structural steel sections described in the Contract. A full-penetration groove weld between welded edges is required.

6-05.3(6) Splicing Steel Casings and Steel Piles

This section is revised to read:

The Engineer will normally permit steel piles and steel casings for cast-in-place concrete piles to be spliced. But in each case, the Contractor shall submit Type 2 Working Drawings supporting the need and describing the method for splicing. Welded splices shall be spaced at a minimum distance of 10 feet. Only welded splices will be permitted.

Splice welds for steel piles shall comply with Section 6-03.3(25) and AWS D1.1/D1.1M, latest edition, Structural Welding Code. Splicing of steel piles shall be performed in accordance with an approved weld procedure. The Contractor shall submit a Type 2 Working Drawing consisting of the weld procedure. For ASTM A 252 material, mill certification for each lot of pipe to be welded shall accompany the submittal. The ends of all steel pipe piling shall meet the fit-up requirements of AWS D1.1/D1.1M, latest edition, Structural Welding Code Section 5.22.3.1, “Girth Weld Alignment (Tubular),” when the material is spliced utilizing a girth weld.

Splice welds of steel casings for cast-in-place concrete piles shall be the Contractor’s responsibility and shall be welded in accordance with AWS D1.1/D1.1M, latest edition, Structural Welding Code. A weld procedure submittal is not required for steel casings used for cast-in-place concrete piles. Casings that collapse or are not watertight, shall be replaced at the Contractor’s expense.

6-05.3(7)B Precast Concrete Piles

The second to last sentence of the second paragraph is revised to read:

The Contractor shall submit Type 2 Working Drawings consisting of the method of lifting the piles.

6-05.3(8) Pile Tips and Shoes

In the last paragraph, the second and third sentences are deleted and replaced with the following new sentence:

If pile tips or shoes other than those denoted in the Qualified Products List are proposed, the Contractor shall submit Type 2 Working Drawings consisting of shop

drawings of the proposed pile tip along with design calculations, specifications, material chemistry and installation requirements, along with evidence of a pile driving test demonstrating suitability of the proposed pile tip.

6-05.3(9)A Pile Driving Equipment Approval

In the first paragraph, the first sentence is revised to read:

Prior to driving any piles, the Contractor shall submit Type 2 Working Drawings consisting of details of each proposed pile driving system.

In the second paragraph, the first sentence is revised to read:

The Contractor shall submit Type 2E Working Drawings consisting of a wave equation analysis for all pile driving systems used to drive piling with required ultimate bearing capacities of greater than 300 tons.

In the second paragraph, the second sentence is deleted.

The last paragraph is revised to read:

Changes to the pile driving system after completion of the Working Drawing review require a revised Working Drawing submittal.

6-05.3(9)B Pile Driving Equipment Minimum Requirements

In the first paragraph, the first sentence is revised to read:

For each drop hammer used, the Contractor shall weigh it in the Engineer's presence or submit a Type 1 Working Drawing consisting of a certificate of its weight.

In the third paragraph, the first sentence is revised to read:

For each diesel, hydraulic, steam, or air-driven hammer used, the Contractor shall submit a Type 1 Working Drawing consisting of the manufacturer's specifications and catalog.

In the fourth paragraph, "approval" is revised to read "permission".

The ninth paragraph is revised to read:

These requirements for minimum hammer size may be waived if a Type 2E Working Drawing is submitted consisting of a wave equation analysis demonstrating the ability of the hammer to obtain the required bearing capacity and minimum tip elevation without damage to the pile.

6-05.3(9)C Pile Driving Leads

In the third paragraph, "approved" is revised to read "permitted".

6-05.3(11)F Pile Damage

In the first sentence of the second paragraph, "approved" is revised to read "accepted".

6-05.3(11)G Pile Cutoff

In the first paragraph, "Engineer's approval" is revised to read "Engineer's permission".

6-05.3(11)H Pile Driving From or Near Adjacent Structures

In the first paragraph, item number 3 is revised to read:

3. Type 2E Working Drawings are submitted in accordance with Sections 1-05.3 and 6-02.3(16), showing the structural adequacy of the existing Structure to safely support all of the construction loads.

6-05.3(12) Determination of Bearing Values

In the footnote below the formula, “approved by the Engineer” is revised to read “acceptable to the Engineer”.

6-05.3(13) Treatment of Timber Pile Heads

In the second paragraph, the first sentence is revised to read:

After cutting treated timber piles to correct elevation, the Contractor shall brush three coats of a preservative that meets the requirements of Section 9-09 on all pile heads (except those to be covered with concrete footings or concrete caps).

6-05.3(15) Completion of Cast-In-Place Concrete Piles

In the first paragraph, “approval” is revised to read “acceptance”.

Section 6-06, Bridge Railings
January 5, 2015

6-06.3(2) Metal Railings

The second paragraph is revised to read:

Before fabricating the railing, the Contractor shall submit Type 2 Working Drawings consisting of the shop plans. The Contractor may substitute other rail connection details for those shown in the Plans if details of these changes show in the shop plans and if the Engineer accepts them in the Working Drawing response comments. In reviewing the shop plan Working Drawings, the Engineer indicates only that they are adequate and complete enough. The review does not indicate a check on dimensions.

Section 6-07, Painting
January 5, 2015

6-07.3 Painting

This section is supplemented with the following new subsections:

6-07.3(14) Metallic Coatings

6-07.3(14)A General Requirements

This specification covers the requirements for thermal spray metallic coatings, with and without additional paint coats, as a means to prevent corrosion.

The coating system consists of surface preparation by wash cleaning and abrasive blast cleaning, thermal spray application of a metallic coating using a material made specifically for that purpose, and, when specified, shop primer coat or shop

primer coat plus top coat in accordance with Section 6-07.3(11)A. The system also includes inspection and acceptance requirements.

6-07.3(14)B Reference Standards

SSPC-SP 10/NACE No. 2	Near White Blast Cleaning
SSPC CS 23.00	Guide for Thermal Spray Metallic Coating Systems
ASTM-C-633	Standard Test Method for Adhesion or Cohesion Strength of Thermal Spray Coatings
ASTM D 4417	Standard Test Methods for Field Measurement of Surface Profile of Blast-Cleaned Steel
ASTM D 6386	Standard Practice for Preparation of Zinc (Hot-Dip Galvanized) Coated Iron and Steel Product and Hardware Surfaces for Painting
ASTM D 4541	Standard Test Method for Pull-Off Strength of Coatings Using Portable Adhesion Testers
ANSI/AWS C2.18	Guide for the Protection of Steel with Thermal Sprayed Coatings of Aluminum, Zinc and their Alloys and Composites

6-07.3(14)C Quality Assurance

A representative sample of each lot of the coating material used shall be submitted to the Engineer for analysis prior to use. Zinc shall have a minimum purity of 99.9 percent. Zinc Aluminum 85/15 wire shall be 14 percent minimum to 16 percent maximum aluminum.

The thermal sprayed coating shall have a uniform appearance. The coating shall not contain any blisters, cracks, chips or loosely adhering particles, oil or other surface contaminants, nodules, or pits exposing the substrate.

The thermal spray coating shall adhere to the substrate with a minimum bond of 700 psi. The Contractor's QA program shall include thermal spray coating bond testing.

The Engineer may cut through the coating with a knife or chisel. If upon doing so, any part of the coating lifts away from the base metal 1/4 in. or more ahead of the cutting blade without cutting the metal, then the bond is considered not effective and is rejected.

Coated areas which have been rejected or damaged in the inspection procedure described shall have the defective sections blast cleaned to remove all of the thermal sprayed coating and shall then be recoated. Before resubmittal and inspection, those sections where coating has not reached the required thickness shall be sprayed with additional metal until that thickness is achieved.

6-07.3(14)D Submittals

The Contractor shall submit to the Engineer, prior to abrasive blast cleaning, a 12 inch square steel plate, of the same material and approximate thickness of the steel to be coated, blasted clean in accordance with Section 6-07.3(14)E. The sample plate will be checked for specified angular surface pattern, the abrasive grit size and type used, and the procedure used. This plate shall be used as the visual standard to determine the acceptability of the cleaned surface. In the event the Contractor's cleaning operation is inferior to the sample plate, the Contractor shall

be required to correct the cleaning operation to do a job comparable to the specimen submitted.

At the same time as submitting the abrasive blast cleaned steel plate sample, the Contractor shall submit to the Engineer, a second 12 inch square steel plate of the same material and thickness, cleaned and thermal spray coated in accordance with the same processes and with the same equipment as intended for use in applying the thermal spray coatings. The Engineer may request additional cleaned and thermal spray coated samples to be produced and submitted coincident with thermal spray coating of the items specified in the Plans to receive thermal spray coatings.

6-07.3(14)E Surface Preparation

Surface irregularities (e.g., sharp edges and/or carburized edges, cracks, delaminations, pits, etc.) interfering with the application of the coating shall be removed or repaired, prior to wash cleaning. Thermal cut edges shall be ground to reduce hardness to attain the surface profile required from abrasive blast cleaning.

All dirt, oil, scaling, etc. shall be removed prior to blast cleaning. All surfaces shall be wash cleaned with either clean water at 8000 psi or water and detergent at 2000 psi with two rinses with clean water.

The surface shall be abrasive blast cleaned to near white metal (SSPC-SP 10). The surface profile shall be measured using a surface profile comparator, replica tape, or other method suitable for the abrasive being used in accordance with ASTM D 4417.

Where zinc coatings up to and including 0.009 inch thick are to be applied, one of the following abrasive grits shall be used with pressure blast equipment to produce a 3.0 mils AA anchor tooth pattern:

1. Aluminum oxide or silicon carbide
   mesh size: SAE G-25 to SAE G-40

2. Hardened steel grit
   mesh size: SAE G-25 to SAE G-40

3. Garnet, flint, or crushed nickel or black beauty coal slag
   mesh size: SAE G-25 to SAE G-50

Where zinc coatings greater than 0.010 inch thick are to be applied, one of the following abrasive grits shall be used with pressure blast equipment to produce a 5.0 mils AA anchor tooth pattern:

1. Aluminum oxide or silicon carbide
   mesh size: SAE G-18 to SAE G-25

2. Hardened steel grit
   mesh size: SAE G-18 to SAE G-25

3. Garnet, flint, or crushed nickel or black beauty coal slag
   mesh size: SAE G-18 to SAE G-25

The pressure of the blast nozzle, as measured with a needle probe gauge, with pressure type blasting equipment shall be as follows:

1. With aluminum oxide, silicon carbide, flint, or slag - 50 psi minimum and 60 psi maximum.

2. With garnet or steel grit - 75 psi minimum.

The pressure at the blast nozzle, with siphon blasting (suction blasting), shall be as follows:

1. With aluminum oxide, silicon carbide, flint, or slag - 75 psi maximum.

2. With garnet or steel grit - 90 psi maximum.

The abrasive blast stream shall be directed onto the substrate surface at a spray angle of 75 to 90 degrees, and moved side to side. The nozzle to substrate distance shall be 4 to 12 inches.

6-07.3(14)F Application of Metallic Coating

No surface shall be sprayed which shows any sign of condensed moisture or which does not comply with Section 6-07.3(14)E. If rust bloom occurs within the holding time between abrasive blast cleaning and thermal spraying, the surface shall be reblasted at a blast angle as close to perpendicular to the surface as possible to achieve a 2.0 to 4.0 mil anchor tooth pattern. Thermal spraying shall not take place when the relative humidity is 90% or greater, when the steel temperature is less than 5°F above the dew point, or when the air or steel temperature is less than 40°F.

Clean, dry air shall be used with not less than 50 psi air pressure at the air regulator. Not more than 50 feet of 3/8 in. ID hose shall be used between the air regulator and the metallizing gun. The metallizing gun shall be started and adjusted with the spray directed away from the work. During the spraying operation and depending upon the equipment being used, the gun shall be held as close to perpendicular as possible to the surface from 5 to 8 inches from the surface of the work.

Manual spraying shall be done in a block pattern, typically 2 feet by 2 feet square. The sprayed metal shall overlap on each pass to ensure uniform coverage. The specified thickness of the coating shall be applied in multiple layers. In no case are fewer than two passes of thermal spraying, overlapping at right angles, acceptable.

At least one single layer of coating shall be applied within 4 hours of blasting and the surface shall be completely coated to the specified thickness within 8 hours of blasting.

The minimum coating thickness shall be 6 mils unless otherwise shown in the Plans.

6-07.3(14)G Applications of Shop Coats and Field Coats

The surface shall be wiped clean with solvent immediately before applying the wash primer. The wash primer shall have a low viscosity appropriate for absorption into the thermal spray coating, and shall be applied within 8 hours after completion of thermal spraying or before oxidation occurs. The dry film thickness of the wash primer shall not exceed 0.5 mils or be less than 0.3 mils. It shall be applied using an appropriate spray gun except in those areas where brush or roller application is necessary. The subsequent shop primer or field coats shall be applied no less than one-half hour after a wash primer.

The shop primer coat, when specified, shall be applied in accordance with Section 6-07.3(11)A and the paint manufacturer's recommendations.

All field coats, when specified, shall be applied in accordance with Section 6-07.3(11)A and the paint manufacturer's recommendations. The color of the top coat shall conform to Section 6-03.3(30) as supplemented in these Special Provisions.

6-07.3(2) Submittals

The first paragraph is revised to read:

The Contractor shall submit Type 2 Working Drawings of the painting plan.

6-07.3(10)A Containment

The second paragraph is revised to read:

The containment length shall not exceed the length of a span (defined as pier to pier). The containment system shall not cause any damage to the existing structure. All clamps and other attachment devices shall be padded or designed such that they shall not mark or otherwise damage the steel member to which they are attached. All clamps and other attachment devices shall be fully described in the Contractor's painting plan Working Drawing submittal. Field welding of attachments to the existing structure will not be allowed. The Contractor shall not drill holes into the existing structure or through existing structural members except as shown in the Contractor's painting plan Working Drawing submittal. All provisions for dust collection, ventilation and auxiliary lighting within the containment system shall be fully described the Contractor's painting plan Working Drawing submittal.

In the second to last paragraph, “approved” is revised to read “accepted”.

6-07.3(10)E Surface Preparation – Full Paint Removal

This section is revised to read:

For structures where full removal of existing paint is specified, the Contractor shall remove any visible oil, grease, and road tar in accordance with SSPC-SP 1.

Following preparation by SSPC-SP 1, all steel surfaces to be painted shall be prepared in accordance with SSPC-SP 10, near-white metal blast cleaning. Surfaces inaccessible to near-white metal blast cleaning shall be prepared in accordance with SSPC-SP 11, power tool cleaning to bare metal, as allowed by the Engineer.

6-07.3(10)F Collecting, Testing and Disposal of Containment Waste

In the first paragraph, the last sentence before the numbered list is revised (up until the colon) to read:

> The sealed waste containers shall be stored in accordance with Section 1-06.4, the painting plan, and the following requirements:

In the second paragraph, the first sentence is revised to read:

> All material collected by and removed from the containment system shall be taken to a landside staging area, provided by the Contractor, for further processing and storage prior to transporting for disposal.

The ninth paragraph is revised to read:

> The Contractor shall submit a Type 1 Working Drawing of all TCLP results.

The first sentence of the last paragraph is revised to read:

> The Contractor shall submit a Type 1 Working Drawing consisting of waste disposal documentation within 15 working days of each disposal.

6-07.3(10)K Coating Thickness

The last paragraph is revised to read:

> If the specified number of coats does not produce a combined dry film thickness of at least the sum of the thicknesses required per coat, or if an individual coat does not meet the minimum thickness, or if visual inspection shows incomplete coverage, the coating system will be rejected, and the Contractor shall discontinue painting and surface preparation operations and shall submit a Type 2 Working Drawing of the repair proposal. The repair proposal shall include documentation demonstrating the cause of the less than minimum thickness along with physical test results, as necessary, and modifications to work methods to prevent similar results. The Contractor shall not resume painting or surface preparation operations until receiving the Engineer's acceptance of the completed repair.

6-07.3(10)L Environmental Condition Requirements Prior to Application of Paint

In the last paragraph, the second to last sentence is revised to read:

> If a paint system manufacturer's recommendations allow for application of a paint under environmental conditions other than those specified, the Contractor shall submit a Type 2 Working Drawing consisting of a letter from the paint manufacturer specifying the environmental conditions under which the paint can be applied.

In the last sentence of the last paragraph, "approval" is revised to read "concurrence".

6-07.3(11)B1 Submittals

The first paragraph (up until the colon) is revised to read:

> The Contractor shall submit Type 2 Working Drawings consisting of the following information:

6-07.3(11)B3 Galvanized Surface Cleaning and Preparation

The first paragraph is revised to read:

> Galvanized surfaces receiving the powder coating shall be cleaned and prepared for coating in accordance with ASTM D 6386, and the project-specific powder coating plan.

6-07.3(11)B4 Powder Coating Application and Curing

The first paragraph (up until the colon) is revised to read:

> After surface preparation, the two-component powder coating shall be applied in accordance with the powder coating manufacturer's recommendations, the project-specific powder coating plan, and as follows:

6-07.3(11)B5 Testing

In the fifth sentence of the first paragraph, the phrase "as approved by the Engineer" is deleted.

The second paragraph is revised to read:

> The results of the QC testing shall be documented in a QC report, and submitted as a Type 2 Working Drawing.

In the fourth paragraph, the phrase "as approved by the Engineer" is deleted.

In the last paragraph, "Engineer's approval" is revised to read "Engineer's acceptance".

6-07.3(11)B6 Coating Protection for Shipping

The phrase "as approved by the Engineer" is deleted from this section.

The first sentence of the last paragraph is revised to read:

> After erection, all coating damage due to the Contractor's shipping, storage, handling, and erection operations shall be repaired by the Contractor in accordance with the project-specific powder coating plan.

6-07.5 Payment

The following new paragraph is inserted before the last paragraph:

> All costs in connection with producing the metallic coatings as specified shall be included in the unit contract price for the applicable item or items of work.

Section 6-09, Modified Concrete Overlays
January 5, 2015

6-09.2 Materials

The second sentence of the fifth paragraph is revised to read:

> Microsilica will be accepted based on submittal of a Manufacturer's Certificate of Compliance.

The seventh paragraph is revised to read:

Latex admixture will be accepted based on submittal of a Manufacturer's Certificate of Compliance.

6-09.3(1)H Mobile Mixer for Latex Modified Concrete

In item number 2 of the first paragraph, "An approved recording meter" is revised to read "A recording meter".

In item number 3 of the first paragraph, "an approved flow meter" is revised to read "a flow meter".

6-09.3(1)J Finishing Machine

The last two sentences of the last paragraph are revised to read:

A machine with a vibrating pan as an integral part may be proposed. Other finishing machines will be allowed subject to concurrence of the Engineer.

6-09.3(2) Submittals

This section is revised to read:

The Contractor shall submit the following Working Drawings in accordance with Section 1-05.3:

1. A Type 1 Working Drawing of the type of machine (rotary milling, hydro-demolition, or shot blasting) selected by the Contractor for use in this project to scarify concrete surfaces.

2. A Type 1 Working Drawing of the axle loads and axle spacing of the rotary milling machine (if used).

3. A Type 2 Working Drawing of the Runoff Water Disposal Plan (if a hydro-demolition machine is used). The Runoff Water Disposal Plan shall describe all provisions for the containment, collection, filtering, and disposal of all runoff water and associated contaminants generated by the hydro-demolition process, including containment, collection and disposal of runoff water and debris escaping through breaks in the bridge deck.

4. A Type 2 Working Drawing of the method and materials used to contain, collect, and dispose of all concrete debris generated by the scarifying process, including provisions for protecting adjacent traffic from flying debris.

5. A Type 1 Working Drawing of the mix design for concrete Class M, and either fly ash modified concrete, microsilica modified concrete, or latex modified concrete, as selected by the Contractor for use in this project in accordance with Section 6-09.3(3).

6. A Type 1 Working Drawing of samples of the latex admixture and the portland cement for testing and compatibility (if latex modified concrete is used).

7. A Type 2 Working Drawing of the paving equipment specifications and details of the screed rail support system, including details of anchoring the rails and providing rail continuity.

6-09.3(3)A General

In the last paragraph, the phrase “and as approved by the Engineer” is deleted.

6-09.3(4)B Latex Admixture

In the second sentence of the second paragraph, the phrase “and as approved by the Engineer” is deleted.

6-09.3(5)A General

The second paragraph is deleted.

In the third and fourth paragraphs, the phrase “and as approved by the Engineer” is deleted.

In the fifth paragraph, “approved by the Engineer” is revised to read “acceptable to the Engineer”.

6-09.3(5)B Testing of Hydro-Demolition and Shot Blasting Machines

In the last sentence of the last paragraph, “approval” is revised to read “acceptance”.

6-09.3(5)C Hydro-Demolishing

In the third and fourth paragraphs, the phrase “as approved by the Engineer” is deleted.

6-09.3(6)B Deck Repair Preparation

The second to last paragraph is revised to read the following three new paragraphs:

The exposed steel reinforcing bars and concrete in the repair area shall be sandblasted or hydro-blasted and blown clean just prior to placing concrete.

Where existing steel reinforcing bars inside deck repair areas show deterioration exceeding the limits defined in the Plans, the Contractor shall furnish and place steel reinforcing bars alongside the deteriorated bars in accordance with the details shown in the Plans. Payment for such extra Work will be by force account as provided in Section 1-09.6.

Bridge deck areas outside the repair area or steel reinforcing bar inside or outside the repair area damaged by the Contractor’s operations, shall be repaired by the Contractor at no additional expense to the Contracting Agency, and to the satisfaction of the Engineer.

6-09.3(6)C Placing Deck Repair Concrete

The third paragraph is supplemented with the following:

The Work of Type 1 further deck preparation shall consist of removing and disposing of the concrete within the repair area.

The following new sentence is inserted before the last sentence of the last paragraph:

The Work of Type 2 further deck preparation shall consist of removing and disposing of concrete within the repair area, and furnishing, placing, finishing, and curing the repair concrete.

6-09.3(7) Surface Preparation for Concrete Overlay

The first sentence of the second paragraph is revised to read:

If either a rotary milling machine or a shot blasting machine is used for concrete scarification, then the concrete deck shall be sandblasted or shot blasted, using equipment identified in the Working Drawing submittals, until sound concrete is exposed.

The third paragraph is revised to read:

If a hydro-demolition machine is used for concrete scarification, then the concrete deck shall be cleaned by water blasting with 7,000 psi minimum pressure, until sound concrete is exposed.

In the fourth paragraph, “as approved by the Engineer” is revised to read “accepted by the Engineer”.

In the last sentence of the eighth paragraph, the phrase “as approved by the Engineer” is deleted.

In the first sentence of the last paragraph, “approved” is revised to read “allowed”.

6-09.3(8)B Quality Assurance for Latex Modified Concrete Overlays

The second sentence of the last paragraph is revised to read:

The technical representative shall be capable of performing, demonstrating, inspecting, and testing all of the functions required for placement of the latex modified concrete as specified in Section 6-09.3(11).

The fourth sentence of the last paragraph is revised to read:

Recommendations made by the technical representative on or off the jobsite shall be adhered to by the Contractor at no additional expense to the Contracting Agency.

6-09.3(10)A Survey of Existing Bridge Deck Prior to Scarification

The third sentence of the fourth paragraph is revised to read:

A Type 1 Working Drawing of each day's survey record shall be provided to the Engineer within three working days after the end of the shift.

6-09.3(10)B Establishing Finish Overlay Profile

In the fourth sentence of the first paragraph, “approved by the Engineer” is revised to read “specified by the Engineer”.

In the second paragraph, the phrase “and as approved by the Engineer” is deleted.

6-09.3(11) Placing Concrete Overlay

In the fourth paragraph, the last sentence of item number 3 is revised to read:

If the Contractor elects to work at night to meet these criteria, adequate lighting shall be provided at no additional expense to the Contracting Agency.

6-09.4 Measurement

The last paragraph is deleted and replaced with the following:

Further deck preparation for Type 1 deck repair and for Type 2 deck repair will be measured by the square foot of surface area of deck concrete removed in accordance with Section 6-09.3(6).

6-09.5 Payment

The Bid item "Further Deck Preparation", per cubic foot and the paragraph following this Bid item are deleted and replaced with the following two new Bid items:

"Further Deck Preparation for Type 1 Deck Repair", per square foot.

"Further Deck Preparation for Type 2 Deck Repair", per square foot.

The Bid item "Further Deck Preparation", force account and the paragraph following this Bid item are deleted.

Section 6-10, Concrete Barrier
January 5, 2015

6-10.1 Description

In the second paragraph, "approved" is revised to read "specified".

6-10.3 Construction Requirements

In the first paragraph, "approved" is revised to read "specified".

6-10.3(5) Temporary Concrete Barrier

The last sentence of the first paragraph is deleted.

The second paragraph is revised to read:

If the Contract calls for the removal and resetting of permanent barrier, and the permanent barrier is not required to remain in place until reset, the permanent barrier may be substituted for temporary concrete barrier. Any of the permanent barrier damaged during its use as temporary barrier will become the property of the Contractor and be replaced with permanent barrier when the permanent barrier is reset to its permanent location.

The third paragraph is revised to read:

All barrier shall be in good condition, without cracks, chips, spalls, dirt, or traffic marks. If any barrier segment is damaged during or after placement, the Contractor shall immediately repair it to the Engineer's satisfaction or replace it with an undamaged section.

The following new paragraph is inserted after the third paragraph:

Delineators shall be placed on the traffic face of the barrier 6 inches from the top and spaced a maximum of 40 feet on tangents and 20 feet through curves. The reflector color shall be white on the right side of traffic and yellow on the left side of traffic. The Contractor shall maintain, replace and clean the delineators when ordered by the Engineer.

Section 6-11, Reinforced Concrete Walls
January 5, 2015

6-11.3(1) Submittals

The first paragraph is revised to read:

The Contractor shall submit Type 2E Working Drawings consisting of excavation shoring plans in accordance with Section 2-09.3(3)D.

The second paragraph is revised to read:

The Contractor shall submit Type 2E Working Drawings of falsework and formwork plans in accordance with Sections 6-02.3(16) and 6-02.3(17).

The third paragraph (up until the colon) is revised to read:

If the Contractor elects to fabricate and erect precast concrete wall stem panels, Type 2E Working Drawings of the following information shall be submitted in accordance with Section 6-02.3(28)A:

The last paragraph is deleted.

6-11.3(3) Precast Concrete Wall Stem Panels

In the third paragraph, the phrase “as approved by the Engineer” is deleted.

Section 6-12, Noise Barrier Walls
January 5, 2015

6-12.3(1) Submittals

In the first paragraph, the second sentence is revised to read:

The Contractor shall submit a Type 2 Working Drawing consisting of the noise barrier wall access plan.

The second paragraph (up until the colon) is revised to read:

For construction of all noise barrier walls with shafts, the Contractor shall submit a Type 2 Working Drawing consisting of the shaft construction plan, including at a minimum the following information:

In the third paragraph, the first sentence is revised to read:

For construction of precast concrete noise barrier walls, the Contractor shall submit Type 2 Working Drawings consisting of shop drawings for the precast concrete panels in accordance with Section 6-02.3(28)A.

6-12.3(2) Work Access and Site Preparation

In the first paragraph, the first sentence is revised to read:

> The Contractor shall construct work access in accordance with the work access plan.

6-12.3(3) Shaft Construction

The first paragraph is revised to read:

> The Contractor shall excavate and construct the shafts in accordance with the shaft construction plan.

In the last sentence of the third paragraph, “approved by the Engineer” is revised to read “acceptable to the Engineer”.

The fourth paragraph is revised to read:

> When caving conditions are encountered, the Contractor shall stop further excavation until implementing the method to prevent ground caving as specified in the shaft construction plan.

In the last sentence of the fifth paragraph, “approved” is revised to read “accepted”.

In the seventh paragraph, “approval” is revised to read “acceptance”.

In the eighth paragraph, the third sentence is revised to read:

> The Contractor shall install the steel reinforcing bar cage as specified in the shaft construction plan.

In the second sentence of the last paragraph, “approval” is revised to read “acceptance”.

In the fourth sentence of the last paragraph, the word “approved” is deleted.

6-12.3(6) Precast Concrete Panel Fabrication and Erection

In item number 3, the second paragraph is revised to read:

> After receiving the Engineer’s review of the shop drawings, the Contractor shall cast one precast concrete panel to be used as the sample panel. The Contractor shall construct the sample panel in accordance with the procedure and details specified in the shop drawings. The Contractor shall make the sample panel available to the Engineer for acceptance.

In item number 3, the first sentence of the third paragraph is revised to read:

> Upon receiving the Engineer’s acceptance of the sample panel, the Contractor shall continue production of precast concrete panels for the noise barrier wall.

In item number 3, the third sentence of the third paragraph is revised to read:

> The sample panel shall be retained at the fabrication site until all precast concrete panels have been fabricated and accepted.

6-12.3(10) Finish Line Ground Dressing

In the last sentence of the second paragraph, the phrase “as approved by the Engineer” is deleted.

Section 6-13, Structural Earth Walls
January 5, 2015

6-13.3(1) Quality Assurance

In the first paragraph, the first sentence is revised to read:

> The structural earth wall manufacturer shall provide a qualified and experienced representative to resolve wall construction problems.

In the first paragraph, the last sentence is revised to read:

> Recommendations made by the structural earth wall manufacturer’s representative shall be followed by the Contractor.

In the second paragraph, item number 4 is revised to read:

> 4. The base of the structural earth wall excavation shall be within three inches of the staked elevations, unless otherwise accepted or specified by the Engineer.

In the second paragraph, item number 6 is revised to read:

> 6. The backfill reinforcement layers shall be located horizontally and vertically within one inch of the locations shown in the structural earth wall working drawings.

6-13.3(2) Submittals

In the first paragraph, the first sentence is revised to read:

> The Contractor, or the supplier as the Contractor’s agent, shall furnish a Manufacturer’s Certificate of Compliance certifying that the structural earth wall materials conform to the specified material requirements.

The second paragraph is revised to read:

> A Type 1 Working Drawing of all test results, performed by the Contractor or the Contractor’s supplier, which are necessary to assure compliance with the specifications, shall submitted along with each Manufacturer’s Certificate of Compliance.

In the third paragraph, the first sentence is revised to read:

> Before fabrication, the Contractor shall submit a Type 1 Working Drawing consisting of the field construction manual for the structural earth walls, prepared by the wall manufacturer.

In the fourth paragraph, the first sentence is revised to read:

> The Contractor, through the license/patent holder for the structural earth wall system, shall submit Type 2E Working Drawings consisting of detailed design calculations and details.

The last paragraph is deleted.

6-13.3(3) Excavation and Foundation Preparation

In the first paragraph, the last two sentences are revised to read:

> The foundation for the structure shall be graded level for a width equal to or exceeding the length of reinforcing as shown in the structural earth wall working drawings and, for walls with geogrid reinforcing, in accordance with Section 2-12.3. Prior to wall construction, the foundation, if not in rock, shall be compacted as accepted by the Engineer.

6-13.3(6) Welded Wire Faced Structural Earth Wall Erection

The first two sentences are revised to read:

> The Contractor shall erect the welded wire wall reinforcement in accordance with the wall manufacturer's field construction manual. Construction geotextile for wall facing shall be placed between the backfill material within the reinforced zone and the coarse granular material immediately behind the welded wire wall facing, as shown in the Plans and the structural earth wall working drawings.

6-13.3(7) Backfill

The third paragraph is revised to read:

> Misalignment or distortion of the precast concrete facing panels or concrete blocks due to placement of backfill outside the limits of this specification shall be corrected in a manner acceptable to the Engineer.

In item number 4 of the fifth paragraph, the phrase "as approved by the Engineer" is deleted.

The last paragraph is deleted.

6-13.3(8) Guardrail Placement

In the first sentence of the second paragraph, "approval" is revised to read "permission".

6-13.3(9) SEW Traffic Barrier and SEW Pedestrian Barrier

The first paragraph (up until the colon) is revised to read:

> The Contractor, in conjunction with the structural earth wall manufacturer, shall design and detail the SEW traffic barrier and SEW pedestrian barrier in accordance with Section 6-12.3(2) and the above ground geometry details shown in the Plans. The barrier Working Drawings and supporting calculations shall be Type 2E and shall include, at a minimum, the following:

Section 6-14, Geosynthetic Retaining Walls
January 5, 2015

6-14.2 Materials

In the first paragraph, the section number next to "Anchor rods and associated nuts, washers and couplers" is revised to read:

9-06.5(4)

The following new paragraph is inserted after the first paragraph:

Anchor plate shall conform to ASTM A 36, ASTM A 572 Grade 50, or ASTM A 588.

6-14.3(2) Submittals

The first paragraph (up until the colon) is revised to read:

The Contractor shall submit Type 2 Working Drawings consisting of detailed plans for each wall. As a minimum, the submittals shall include the following:

6-14.3(4) Erection and Backfill

In the second sentence of the second paragraph, “approved by” is revised to read “acceptable to”.

In the last sentence of the fifth paragraph, “approval” is revised to read “permission”.

The sixth paragraph is deleted.

In item number 5 in the eighth paragraph, the phrase “as approved by the Engineer” is deleted.

In the ninth paragraph, the first sentence is revised to read:

The Contractor shall construct wall corners at the locations shown in the Plans, and in accordance with the wall corner construction sequence and method in the Working Drawing submittal.

In the last paragraph, the first sentence is revised to read:

Where required by retaining wall profile grade, the Contractor shall terminate top layers of retaining wall geosynthetic and backfill in accordance with the method in the Working Drawing submittal.

6-14.5 Payment

In the paragraph following the Bid item “Concrete Fascia Panel”, per square foot, “concrete leveling pad” is revised to read “concrete footing”.

Section 6-15, Soil Nail Walls
January 15, 2015

6-15.3(3) Submittals

The first paragraph (excluding the numbered list) is revised to read:

The Contractor shall submit Type 2 Working Drawings of the following information:

6-15.3(6) Soil Nailing

In the first paragraph, the last sentence is revised to read:

Damaged or defective encapsulation shall be repaired in accordance with the manufacturer’s recommendations.

The eighth paragraph is revised to read:

If sections of the wall are constructed at different times than the adjacent soil nail sections, the Contractor shall use stabilizing berms, temporary slopes, or other measures acceptable to the Engineer, to prevent sloughing or failure of the adjacent soil nail sections.

6-15.3(8) Soil Nail Testing and Acceptance

In the first paragraph, the second sentence is revised to read:

The Contractor shall submit Type 1 Working Drawings of all test data.

The last sentence of the seventh paragraph is revised to read:

The Contractor shall submit Type 2E Working Drawings of the reaction frame.

6-15.3(8)A Verification Testing

In the third paragraph, the first sentence is revised to read:

The Contractor shall submit Type 2E Working Drawings consisting of design details of the verification testing, including the system for distributing test load pressures to the excavation surface and appropriate nail bar size and reaction plate.

Section 6-16, Soldier Pile and Soldier Pile Tieback Walls
January 5, 2015

6-16.3(2) Submittals

The first paragraph is revised to read:

The Contractor shall submit Type 2 Working Drawings consisting of shop plans as specified in Section 6-03.3(7) for all structural steel, including the steel soldier piles, and shall submit Type 2 Working Drawings consisting of shop plans and other details as specified in Section 6-17.3(3) for permanent ground anchors.

The second paragraph is revised to read:

The Contractor shall submit Type 1 Working Drawings consisting of the permanent ground anchor grout mix design and the procedures for placing the grout.

The third paragraph (excluding the numbered list) is revised to read:

The Contractor shall submit Type 2E Working Drawings consisting of forming plans for the concrete fascia panels, as specified in Sections 6-02.3(16) and 6-02.3(17).

In the fourth paragraph, the first sentence is revised to read:

The Contractor shall submit Type 2 Working Drawings consisting of a shaft installation plan.

The last paragraph is deleted.

6-16.3(3) Shaft Excavation

In the third paragraph, the last sentence is revised to read:

> A temporary casing, slurry, or other methods specified in the shaft installation plan shall be used if necessary to ensure such safety and stability.

The fourth paragraph is revised to read:

> Where caving in conditions are encountered, no further excavation will be allowed until the Contractor has implemented the method to prevent ground caving as submitted in accordance with item 4 of the Shaft Installation Plan.

The sixth paragraph is revised to read:

> The excavated shaft shall be inspected and receive acceptance by the Engineer prior to proceeding with construction.

6-16.3(6)B Temporary Lagging

The second paragraph (up until the colon) is revised to read:

> The Contractor shall submit Type 2E Working Drawings consisting of the soldier pile wall lagging design details and supporting design calculations. The submittal shall include, at a minimum, the following:

In item number 4 of the second paragraph, “approved by” is revised to read “acceptable to”.

The last paragraph (excluding the table) is revised to read:

> Notwithstanding the requirements of Section 1-06.1, steel materials used by the Contractor as temporary lagging may be salvaged steel provided that the use of such salvaged steel materials shall be subject to visual inspection and acceptance by the Engineer. For salvaged steel materials where the grade of steel cannot be positively identified, the design stresses for the steel shall conform to the Section 6-02.3(17)B requirements for salvaged steel, regardless of whether rivets are present or not.

6-16.3(6)D Installing Lagging and Permanent Ground Anchor

In the last sentence of the second paragraph, the phrase “as approved by the Engineer” is deleted.

In the last sentence of the fourth paragraph, the phrase “as approved by the Engineer” is deleted.

6-16.3(8) Concrete Fascia Panel

In the first paragraph, the phrase “as approved by the Engineer” is deleted.

Section 6-17, Permanent Ground Anchors
January 5, 2015

6-17.3(3) Submittals

The first paragraph is revised to read:

The Contractor shall submit Type 2 Working Drawings consisting of details and structural design calculations for the ground anchor system or systems intended for use.

The second paragraph is revised to read:

The Contractor shall submit a Type 1 Working Drawing consisting of a detailed description of the construction procedure proposed for use.

The third paragraph (up until the colon) is revised to read:

The Contractor shall submit a Type 2 Working Drawing consisting of ground anchor schedule giving:

In the fourth paragraph, the first sentence is revised to read:

The Contractor shall submit a Type 2 Working Drawing detailing the ground anchor tendon and the corrosion protection system.

In the fourth paragraph, item number 3 is revised to read:

3. Unbonded length corrosion protection system, including the permanent rubber seal between the trumpet and the tendon unbonded length corrosion protection and the transition between the tendon bond length and the unbonded tendon length corrosion protection.

The last five paragraphs are deleted and replaced with the following four new paragraphs:

The Contractor shall submit Type 2 Working Drawings consisting of shop plans as specified in Section 6-03.3(7) for all structural steel, including the permanent ground anchors.

The Contractor shall submit Type 1 Working Drawings consisting of the mix design for the grout conforming to Section 9-20.3(4) and the procedures for placing the grout. The Contractor shall also submit the methods and materials used in filling the annulus over the unbonded length of the anchor.

The Contractor shall submit Type 2 Working Drawings consisting of the method proposed to be followed for the permanent ground anchor testing. This shall include all necessary drawings and details to clearly describe the method proposed.

The Contractor shall submit Type 2 Working Drawings consisting of calibration data for each load cell, test jack, pressure gauge and master pressure gauge to be used. The calibration tests shall have been performed by an independent testing laboratory and tests shall have been performed within 60 calendar days of the date submitted.

6-17.3(5) Tendon Fabrication

In the tenth paragraph, the last sentence is deleted.

The twelfth paragraph is revised to read:

The total anchor length shall not be less than that indicated in the Plans or the Working Drawing submittal.

In the last paragraph, the phrase “as approved by the Engineer” is deleted.

6-17.3(7) Installing Permanent Ground Anchor

In the second paragraph, the third sentence is revised to read:

> The Contractor’s method to prevent ground movement shall be submitted as a Type 2 Working Drawing.

In the second paragraph, the second to last sentence is revised to read:

> At the point of entry the ground anchor shall be installed within plus or minus three degrees of the inclination from horizontal shown in the Plans or the Working Drawing submittal.

Section 6-18, Shotcrete Facing
January 5, 2015

6-18.3(1) Submittals

In the first paragraph, the first sentence (up until the colon) is revised to read:

> The Contractor shall submit Type 2 Working Drawings consisting of the following:

In the first paragraph, item number 2 is revised to read:

> 2. Method and equipment used to apply, finish and cure the shotcrete facing.

The last paragraph is deleted.

6-18.3(2) Mix Design

In the first paragraph, the second and third sentences are deleted.

In the last sentence of the second paragraph, “and approved by the Engineer” is deleted.

6-18.3(3)A Preproduction Testing

In the last sentence, “approved” is revised to read “accepted”.

6-18.3(7) Shotcrete Application

In the last paragraph, the first sentence is revised to read:

> If field inspection or testing, by the Engineer, indicates that any shotcrete produced, fails to meet the requirements, the Contractor shall immediately modify procedures, equipment, or system, as necessary to produce specification material.

Section 6-19, Shafts
April 6, 2015

6-19.3(2) Shaft Construction Submittal

The last sentence is revised to read:

The submittals shall be Type 2 Working Drawings, except the shaft slurry technical assistance submittal shall be Type 1.

6-19.3(3) Shaft Excavation

In the first paragraph, the phrase “as approved by the Engineer” is deleted.

6-19.3(3)B4 Temporary Telescoping Shaft Casing

In the first paragraph, the first sentence of item number 1 is revised to read:

The Contractor shall submit the request to use temporary telescoping casing as a Type 2 Working Drawing.

6-19.3(3)D Bottom of Shaft Excavation

In the first sentence of the second paragraph, “approved” is revised to read “accepted”.

6-19.3(3)E Shaft Obstruction

In the last sentence, “approved” is revised to read “accepted”.

6-19.3(3)F Voids Between Permanent Casing and Shaft Excavation

In the last sentence, the words “and as approved by the Engineer” are deleted.

6-19.3(3)G Operating Shaft Excavation Equipment From an Existing Bridge

The second sentence is revised to read:

If necessary and safe to do so, and if the Contractor submits a Type 2 Working Drawing consisting of a written request in accordance with Section 6-01.6, the Engineer may permit operation of drilling equipment on a bridge.

6-19.3(3)H Seals for Shaft Excavation in Water

The first paragraph is revised to read:

When shafts are constructed in water and the Plans show a seal between the casing shoring and the upper portion of the permanent casing of the shaft, the Contractor shall construct a seal in accordance with the shaft installation narrative specified in Section 6-19.3(2)B Item 7.

The last sentence of the last paragraph is revised to read:

If the Contractor uses a casing shoring diameter other than that specified in the Plans, the Contractor shall submit a revised seal design in accordance with Section 6-19.3(2)B Item 7.

6-19.3(4)C Slurry Sampling and Testing

The second to last sentence of the first paragraph is revised to read:

Synthetic slurry shall conform to Section 9-36.2(2), the quality control plan included in the shaft installation narrative in accordance with Section 6-19.3(2)B Item 4.

The second sentence of the second paragraph is revised to read:

These records shall be submitted as a Type 1 Working Drawing once the slurry system has been established in the first drilled shaft on the project.

6-19.3(4)E Maintenance of a Stable Shaft Excavation

In the last sentence of the first paragraph, "approval" is revised to read "review".

6-19.3(4)F Disposal of Slurry and Slurry Contacted Spoils

This section is revised to read:

The Contractor shall manage and dispose of the slurry wastewater in accordance with Section 8-01.3(1)C. Slurry-contacted spoils shall be disposed of as specified in the shaft installation narrative in accordance with Section 6-19.3(2)B, item 8, and in accordance with the following requirements:

1. Uncontaminated spoils in contact with water-only slurry may be disposed of as clean fill.

2. Uncontaminated spoils in contact with water slurry mixed with flocculants approved in Section 8-01.3(1)C3 may be disposed of as clean fill away from areas that drain to surface waters of the state.

3. Spoils in contact with synthetic slurry or water slurry with polymer-based additives or flocculants not approved in Section 8-01.3(1)C3 shall be disposed of in accordance with Section 2-03.3(7)C. With permission of the Engineer, the Contractor may re-use these spoils on-site.

4. Spoils in contact with mineral slurry shall be disposed of in accordance with Section 2-03.3(7)C. With permission of the Engineer, the Contractor may re-use these spoils on-site.

6-19.3(5)A Steel Reinforcing Bar Cage Assembly

In the second to last sentence of the first paragraph, the phrase "as approved by the Engineer" is deleted.

6-19.3(5)D Steel Reinforcing Bar Cage Support at Base of Shaft Excavation

The first sentence is revised to read:

For shafts with temporary casing within 15-feet of the bottom of shaft elevation as specified in the Plans, the Contractor may place quarry spalls or other rock backfill acceptable to the Engineer into the shaft below the specified bottom of shaft elevation as a means to support the steel reinforcing bar cage, provided that the materials and means to accomplish this have been addressed by the shaft installation narrative, as specified in Section 6-19.3(2)B Item 9.

6-19.3(6)C Care for CSL Access Tubes From Erection Through CSL Testing

In the last sentence, "as approved by the Engineer" is revised to read "acceptable to the Engineer".

6-19.3(8)C Requirements for Leaving Temporary Casing in Place

Item number 1 (up until the colon) is revised to read:

1. The Contractor shall submit a Type 2E Working Drawing of the following information:

In item C of item number 1, the phrase “in accordance with Section 6-01.9” is deleted.

Item number 2 is deleted.

6-19.3(9)D Requirements to Continue Shaft Excavation Prior to Acceptance of First Shaft

This section is revised to read:

Except as otherwise noted, the Contractor shall not commence subsequent shaft excavations until receiving the Engineer's acceptance of the first shaft, based on the results and analysis of the crosshole sonic log testing for the first shaft. The Contractor may commence subsequent shaft excavations prior to receiving the Engineer’s acceptance of the first shaft, provided the following condition is satisfied:

The Engineer permits continuing with shaft construction based on the Engineer’s observations of the construction of the first shaft, including, but not limited to, conformance to the shaft installation narrative in accordance with Section 6-19.3(2)B, and the Engineer’s review of Contractor’s daily reports and Inspector’s daily logs concerning excavation, steel reinforcing bar placement, and concrete placement.

6-19.3(9)F Contractor’s Investigation and Remedial Action Plan

This section is revised to read:

For all shafts determined to be unacceptable, the Contractor shall submit a Type 2 Working Drawing consisting of a plan for further investigation or remedial action. All modifications to the dimensions of the shafts, as shown in the Plans, required by the investigation and remedial action plan shall be supported by calculations and working drawings. All investigation and remedial correction procedures and designs shall be submitted.

6-19.3(9)H Cored Holes

The first sentence of the second paragraph is revised to read:

Prior to beginning coring, the Contractor shall submit Type 2 Working Drawings consisting of the method and equipment used to drill and remove cores from shaft concrete.

Section 8-01, Erosion Control and Water Pollution Control
January 5, 2015

8-01.2 Materials

This section is supplemented with the following new paragraph:

For all seed the Contractor shall furnish the Engineer with the following documentation:

1. The state or provincial seed dealer license and endorsements.

2. Copies of Washington State Department of Agriculture (WSDA) test results on each lot of seed. Test results must be within six months prior to the date of application.

8-01.3(1)A Submittals

The first sentence in the second paragraph is revised to read:

Modified TESC Plans shall meet all requirements of the current edition of the WSDOT Temporary Erosion and Sediment Control Manual M 3109.

8-01.3(1)C Water Management

Items number 1 through 3 are deleted.

This section is supplemented with the following new subsections:

8-01.3(1)C1 Disposal of Dewatering Water

When uncontaminated groundwater with a pH range of 6.5 – 8.5 is encountered in an excavation, it may be disposed of as follows:

1. When the turbidity of the groundwater is 25 NTU or less, it may bypass detention and treatment facilities and be discharged into the stormwater conveyance system at a rate that will not cause erosion or flooding in the receiving surface water body.

2. When the turbidity of the groundwater is not more than 25 NTU above or 125% of the turbidity of the site stormwater runoff, whichever is greater, the same detention and treatment facilities as used to treat the site runoff may be used.

3. When the turbidity of the groundwater is more than 25 NTU above or 125% of the turbidity of the site stormwater runoff, whichever is greater, the groundwater shall be treated separately from the site stormwater.

Alternatively, the Contractor may pursue independent disposal and treatment alternatives that do not use the stormwater conveyance system.

8-01.3(1)C2 Process Wastewater

Wastewater generated on-site as a byproduct of a construction process shall not be discharged to surface waters of the State. Some sources of process wastewater may be infiltrated in accordance with the NPDES Construction Stormwater General Permit.

8-01.3(1)C3 Shaft Drilling Slurry Wastewater

Wastewater generated on-site during shaft drilling activity shall be managed and disposed of in accordance with the requirements below. No shaft drilling slurry wastewater shall be discharged to surface waters of the State. Neither the sediment nor liquid portions of the shaft drilling slurry wastewater shall be contaminated, as detectable by visible or olfactory indication (e.g., chemical sheen or smell).

1. Water-only shaft drilling slurry or water slurry with approved flocculants may be infiltrated on-site. Flocculants used shall meet the requirements of Section 9-14.5(1) or shall be chitosan products listed as General Use Level Designation (GULD) on the Department of Ecology's stormwater treatment technologies webpage for construction treatment. Infiltration is permitted if the following requirements are met:

    a. Wastewater shall have a pH of 6.5 – 8.5 prior to discharge.

b. The source water meets drinking water standards or the Groundwater Quality Criteria listed in WAC 173-200-040.

c. The amount of flocculant added to the slurry shall be kept to the minimum needed to adequately settle out solids. The flocculant shall be thoroughly mixed into the slurry.

d. Infiltration locations shall be at least 100 feet away from surface waters, wells, on-site sewage systems, aquifer-sensitive recharge areas, sole source aquifers, and well-head protection areas. Before infiltration begins, there shall be a minimum of 5 feet of unsaturated soil between the soil surface receiving the wastewater for infiltration and the groundwater surface (i.e., saturated soil).

e. The slurry removed from the shaft shall be contained in a leak proof cell or tank for a minimum of 3 hours.

f. Within a 24 hour period, a maximum of 21,000 gallons of slurry wastewater may be infiltrated in an infiltration location. The infiltration rate shall be reduced if needed to prevent wastewater from leaving the infiltration location. The infiltration site shall be monitored regularly during infiltration activity. All wastewater discharged to the ground must fully infiltrate and discharges must stop before the end of each work day.

g. After infiltration activity is complete, loose sediment in the infiltration location that may have resulted from the infiltration activity or the removal of BMPs used to manage infiltration activity shall be stabilized to prevent mobilization by stormwater runoff.

h. Drilling spoils and settled sediments remaining in the containment cell or tank shall be disposed of in accordance with Section 6-19.3(4)F.

i. Infiltration locations shall be marked on the on-site temporary erosion and sediment control (TESC) plan sheets before the infiltration activity begins.

j. Prior to infiltrating water-only shaft drilling slurry or water slurry with approved flocculants, the Contractor shall submit a Shaft Drilling Slurry Wastewater Management and Infiltration Plan as a Type 2 Working Drawing. This Plan shall be kept on-site, adapted if needed to meet the construction requirements, and updated to reflect what is being done in the field. The Working Drawing shall include, at a minimum, the following information:

    i. Plan sheet showing the proposed infiltration location and all surface waters, wells, on-site sewage systems, aquifer-sensitive recharge areas, sole source aquifers, and well-head protection areas within 150 feet.

    ii. The proposed elevation of soil surface receiving the wastewater for infiltration and the anticipated phreatic surface (i.e., saturated soil).

iii. The source of the water used to produce the slurry.

iv. The estimated total volume of wastewater to be infiltrated.

v. The approved flocculant to be used (if any).

vi. The controls or methods (e.g., trenches, traps, berms, silt fence, dispersion, or discharge metering devices) that will be used to prevent surface wastewater runoff from leaving the infiltration location. The Working Drawing shall include all pertinent design details (e.g., sizing of trenches or traps, placement or height of berms, application techniques) needed to demonstrate the proposed controls or methods are adequate to prevent surface wastewater runoff from leaving the infiltration location.

vii. The strategy for removing slurry wastewater from the shaft and containing the slurry wastewater once it has been removed from the shaft.

viii. The strategy for monitoring infiltration activity and adapting methods to ensure compliance.

ix. A contingency plan that can be implemented immediately if it becomes evident that the controls in place or methods being used are not adequate.

x. The strategy for cleaning up the infiltration location after the infiltration activity is done. Cleanup shall include stabilizing any loose sediment on the surface within the infiltration area generated as a byproduct of suspended solids in the infiltrated wastewater or soil disturbance associated with BMP placement and removal.

2. Shaft drilling mineral slurry, synthetic slurry, or slurry with polymer additives not approved for infiltration shall be contained and disposed of by the Contractor at an approved disposal facility in accordance with Section 2-03.3(7)C. Spoils that have come into contact with mineral slurry shall be disposed of in accordance with Section 6-19.3(4)F.

8-01.3(1)C4 Management of Off-Site Water

Prior to disruption of the normal watercourse, the Contractor shall intercept the off-site surface water and pipe it either through or around the project site. This water shall not be combined with on-site stormwater. It shall be discharged at its preconstruction outfall point in such a manner that there is no increase in erosion below the site. The Contractor shall submit a Type 2 Working Drawing consisting of the method for performing this Work.

8-01.3(2)A Preparation for Application

This section's content is deleted and replaced with the following two new subsections:

8-01.3(2)A1 Seeding

Areas to be cultivated are shown in the Plans or specified in the Special Provisions. The areas shall be cultivated to the depths specified to provide a reasonably firm but friable seedbed. Cultivation shall take place no sooner than 2 weeks prior to seeding.

All areas to be seeded, including excavated slopes shall be compacted and prepared unless otherwise specified or ordered by the Engineer. A cleated roller, crawler tractor, or similar equipment that forms longitudinal depressions at least 2 inches deep shall be used for compaction and preparation of the surface to be seeded.

The entire area shall be uniformly covered with longitudinal depressions formed perpendicular to the natural flow of water on the slope. The soil shall be conditioned with sufficient water so the longitudinal depressions remain in the soil surface until completion of the seeding.

Prior to seeding, the finished grade of the soil shall be 1 inch below the top of all curbs, junction and valve boxes, walks, driveways, and other Structures. The soil shall be in a weed free and bare condition.

All bags of seed shall be brought to the site in sealed bags and shall have seed labels attached showing the seed meets the Specifications. Seed which has become wet, moldy, or otherwise damaged in transit or storage will not be accepted.

8-01.3(2)A2 Temporary Seeding

A cleated roller, crawler tractor, or similar equipment that forms longitudinal depressions at least 2 inches deep shall be used for compaction and preparation of the surface to be seeded. The entire area shall be uniformly covered with longitudinal depressions formed perpendicular to the natural flow of water on the slope. The soil shall be conditioned with sufficient water so the longitudinal depressions remain in the soil surface until completion of the seeding.

8-01.3(2)B Seeding and Fertilizing

In the list in the second paragraph, item numbers 1-5 are revised to read:

1. A hydro seeder that utilizes water as the carrying agent, and maintains continuous agitation through paddle blades. It shall have an operating capacity sufficient to agitate, suspend, and mix into a homogeneous slurry the specified amount of seed and water or other material. Distribution and discharge lines shall be large enough to prevent stoppage and shall be equipped with a set of hydraulic discharge spray nozzles that will provide a uniform distribution of the slurry.

2. Blower equipment with an adjustable disseminating device capable of maintaining a constant, measured rate of material discharge that will ensure an even distribution of seed at the rates specified.

3. Helicopters properly equipped for aerial seeding.

4. Power-drawn drills or seeders.

5. Areas in which the above methods are impractical may be seeded by hand methods.

8-01.3(2)C Liming

This section including title is deleted in its entirety and replaced with the following:

8-01.3(2)C Vacant

8-01.3(2)D Mulching

The first sentence of the second paragraph is revised to read:

Distribution of straw mulch material shall be by means that utilizes forced air to blow mulch material on seeded areas.

8-01.3(11) Outlet Protection

In the last sentence, “Section 9-13.6” is revised to read “Section 9-13.1(5)”.

8-01.4 Measurement

In the twelfth paragraph, “liming” is deleted.

8-01.5 Payment

The bid item “Liming”, per acre is deleted.

Section 8-02, Roadside Restoration
January 5, 2015

8-02.3(1) Responsibility During Construction

The last sentence of the second paragraph is revised to read:

This Work shall include keeping the planted and seeded areas free from insect infestation, weeds or unwanted vegetation, litter, and other debris along with retaining the finished grades and mulch in a neat uniform condition.

8-02.3(2) Roadside Work Plan

This section’s title is revised to read:

Work Plans

This section’s content is deleted in its entirety and replaced with the following new subsections:

8-02.3(2)A Roadside Work Plan

Before starting any Work that disturbs the earth and as described in Sections 8-01, 8-02 and 8-03, the Contractor shall submit a roadside work plan. The roadside work plan shall be submitted as a Type 1 Working Drawing and shall define the Work necessary to provide all Contract requirements, including: wetland excavation, soil preparation, habitat structure placement, planting area preparation, seeding area preparation, bark mulch and compost placement, seeding, planting, plant replacement, irrigation, and weed control in narrative form.

The Roadside Work Plan shall also include a copy of the approved progress schedule.

8-02.3(2)B Weed and Pest Control Plan

The Weed and Pest Control Plan shall be submitted as a Type 1 Working Drawing. The weed and pest control plan shall include scheduling and methods of all control measures required under the Contract or proposed by the Contractor including soil preparation methods to meet the required soil surface conditions in the planting, bark mulch, and wetland areas. The weed control plan shall show general weed control including hand, mechanical and chemical methods, timing, application of herbicides including type, rate, use and timing, mowing, and noxious weed control. Target weeds and unwanted vegetation to be removed shall be identified and listed in the weed control plan.

The plan shall be prepared and signed by a licensed Commercial Pest Control Operator or Consultant when chemical pesticides are proposed. The plan shall include methods of weed control; dates of weed control operations; and the name, application rate, and Material Safety Data Sheets of all proposed herbicides. In addition, the Contractor shall furnish the Engineer with a copy of the current product label for each pesticide and spray adjuvant to be used. These product labels shall be submitted with the weed control plan for approval.

8-02.3(2)C Plant Establishment Plan

The Plant Establishment Plan shall be prepared in accordance with the requirements of Section 8-02.3(13) and submitted as a Type 1 Working Drawing. The Plan shall show the proposed scheduling of activities, materials, equipment to be utilized for the first-year plant establishment, and an emergency contact person. The Plan shall include the management of the irrigation system, when applicable. Should the plan become unworkable at any time during the first-year plant establishment, the Contractor shall submit a revised plan prior to proceeding with further Work.

8-02.3(3) Weed and Pest Control

This section is supplemented with the following new paragraph:

Grass, including grass applied in accordance with Section 8-01, growing within the mulch ring of a plant shall be considered a weed and be controlled on the project in accordance with the weed and pest control plan.

8-02.3(4) Topsoil

The last sentence of the first paragraph is revised to read:

After the topsoil has been spread, all large clods, hard lumps, and rocks 2 inches in diameter and larger, and litter shall be raked up, removed, and disposed of by the Contractor.

The following new paragraph is inserted after the first paragraph:

Topsoil stockpiled for project use shall be protected to prevent erosion and weed growth. Weed growth on topsoil stockpile sites shall be immediately eliminated in accordance with the approved Weed and Pest Control Plan.

8-02.3(4)C Topsoil Type C

The last sentence is revised to read:

Topsoil Type C shall meet the requirements of Sections 8-02.3(4), 8-02.3(4)B, and 9-14.1(3).

8-02.3(12) Completion of Initial Planting

Item number 4 in the last paragraph is deleted.

8-02.3(13) Plant Establishment

The first sentence of the second paragraph is deleted.

The second paragraph is supplemented with the following new sentence:

The 1 calendar year shall be extended an amount equal to any periods where the Contractor does not comply with the plant establishment plan.

The first sentence of the fourth paragraph is revised to read:

During the first year of plant establishment under PSIPE (Plant Selection Including Plant Establishment), the Contractor shall meet monthly with the Engineer for the purpose of joint inspection of the planting material on a mutually agreed upon schedule.

The last two paragraphs are deleted.

8-02.4 Measurement

This section is supplemented with the following:

Plant selection will be measured per each.

PSIPE __ (Plant Selection Including Plant Establishment) will be measured per each.

8-02.5 Payment

The paragraph following the bid item "Topsoil Type ____", per acre is revised to read:

The unit Contract price per acre for "Topsoil Type ____" shall be full payment for all costs for the specified Work.

The bid item "PSIPE ___", per each and the paragraph following the bid item are revised to read:

"PSIPE ___", per each.

The unit Contract price for "Plant Selection ___", per each, and "PSIPE ___", per each, shall be full pay for all Work necessary for weed control within the planting area, planting area preparation, fine grading, planting, cultivating, plant storage and protection, fertilizer and root dip, staking, cleanup, and water necessary to complete planting operations as specified to the end of first year plant establishment.

The bid item "Plant Establishment - ____ Year" is deleted.

Section 8-04, Curbs, Gutters, and Spillways
January 5, 2015

8-04.2 Materials
The referenced section for the following item is revised to read:

Hand Placed Riprap 9-13.1(4)

8-04.3(1) Cement Concrete Curbs, Gutters, and Spillways
The first sentence in the fourth paragraph is revised to read:

Expansion joints in the curb or curb and gutter shall be spaced as shown in the Plans, and placed at the beginning and ends of curb returns, drainage Structures, bridges, and cold joints with existing curbs and gutters.

In the third sentence of the fourth paragraph, "¼-inch" is revised to read "⅜-inch".

8-04.3(1)A Extruded Cement Concrete Curb
The second sentence in the second paragraph is revised to read:

Cement concrete curbs shall be anchored to the existing pavement by placing steel reinforcing bars 1 foot on each side of every joint.

The third paragraph is revised to read:

Steel reinforcing bars shall meet the dimensions shown in the Standard Plans.

Section 8-09, Raised Pavement Markers
April 7, 2014

8-09.3(6) Recessed Pavement Marker
The following sentence is inserted after the first sentence of the first paragraph:

The Contractor shall ensure that grinding of the pavement does not result in any damage, (e.g. chipping, spalling or raveling) to the pavement to remain.

Section 8-11, Guardrail
April 7, 2014

8-11.3(1) Beam Guardrail

After the below Amendments to 8-11.3(1)F and 8-11.3(1)G are applied, this section is supplemented with the following new sub-section:

8-11.3(1)F Removing and Resetting Beam Guardrail

The Contractor shall remove and reset existing guardrail posts, rail element, hardware and blocks to the location shown in the Plans. The mounting height of reset rail element shall be at the height shown in the Plans. The void caused by the removal of the post shall be backfilled and compacted.

The Contractor shall remove and replace any existing guardrail posts and blocks that are not suited for re-use, as staked by the Engineer. The void caused by the removal of the post shall be backfilled and compacted. The Contractor shall then furnish and install a new guardrail post to provide the necessary mounting height.

8-11.3(1)A Erection of Posts

The second paragraph in this section is deleted.

8-11.3(1)C Terminal and Anchor Installation

The last sentence in the last paragraph is deleted.

8-11.3(1)F Plans

This section number is revised to:

8-11.3(1)G

8-11.3(1)G Guardrail Construction Exposed to Traffic

This section number is revised to:

8-11.3(1)H

Section 8-18, Mailbox Support
August 4, 2014

8-18.3(1) Type 3 Mailbox Support

In the third paragraph, the first sentence is revised to read:

With the Engineer's consent, a Type 3 Mailbox Support design, made of steel or other durable material, that meets the NCHRP 350 or the Manual for Assessing Safety Hardware (MASH) crash test criteria may be used in place of the design shown in the *Standard Plans*.

Section 8-20, Illumination, Traffic Signal Systems, Intelligent Transportation Systems, and Electrical
April 6, 2015

8-20.2(1) Equipment List and Drawings

The second sentence of the second paragraph is revised to read:

Supplemental data would include such items as catalog cuts, product Specifications, shop drawings, wiring diagrams, etc.

The third paragraph (up until the colon) is revised to read:

If the luminaires are not listed in the Qualified Products List, the Contractor shall submit the following information for each different type of luminaire required on the Contract:

The fourth paragraph (up until the colon) is revised to read:

The Contractor shall submit for approval Type 3E Working Drawings in accordance with Section 1-05.3 for each of the following types of standards called for on this project:

The fifth paragraph is revised to read:

The Contractor will not be required to submit shop drawings for approval for light standards and traffic signal standards conforming to the preapproved plans listed in the Special Provisions. The Contractor may use preapproved plans posted on the WSDOT website with a more current revision date than published in the Special Provisions.

8-20.3(1) General

The following six new paragraphs are inserted after the second paragraph:

If a portion of an existing communication conduit system is damaged due to the Contractor's activities, the affected system shall be restored to original condition. Conduit shall be repaired. Communication cables shall be replaced and the communication system shall be made fully operational within 24 hours of being damaged.

Damaged communication cable shall be replaced between existing termination or splice points. No additional termination or splice points will be allowed. An existing termination or splice point is defined as a location where all existing fiber strands or twisted pair wires are terminated or spliced at one point. Communication cable shall be defined as either copper twisted pair or fiber optic cables. The Contractor may use temporary splices to restore Contracting Agency communication systems until the permanent communication cable system is restored.

When damage to an existing communication system has occurred, the Contractor shall perform the following in addition to other restoration requirements:

1. Inspect the communication raceway system including locate wire or tape to determine the extent of damage.

2. Contact the Engineer for Fiber Optic Cable and Twisted Pair (TWP) Copper Cable acceptance testing requirements and communication system restoration requirements.

3. Initially perform the acceptance tests to determine the extent of damage and also perform the acceptance tests after repairs are completed. Provide written certification that the communication cable system, including the locate wire or tape, is restored to test standard requirements.

Communication cables shall be restored by Contractor personnel that are WSDOT prequalified for communication installation work. Restoration shall be considered electrical work when the path of the communication system interfaces with electrical systems. Electrical work of this nature shall be performed by Contractor personnel that are WSDOT prequalified for work on both electrical and communication systems.

If the Contractor or Subcontractors are unable or unqualified to complete the restoration work, the Engineer may have the communication or electrical systems restored by other means and subtract the cost from the money that will be or is due the Contractor.

When field repair of existing conduit, innerduct or outerduct is required, the repair kits shall be installed per manufacturer's recommendations. Repair kits and each

connection point between the repair kit and the existing raceway system shall be sealed to prevent air leakage during future cable installation.

8-20.3(8) Wiring

The second sentence in the eleventh paragraph is revised to read:

Every conductor at every wire termination, connector, or device shall have an approved wire marking sleeve bearing, as its legend, the circuit number indicated in the Contract.

8-20.3(13)A Light Standards

In the third paragraph, the last sentence of item number 1 is revised to read:

Conduit shall extend a maximum of 1 inch above the top of the foundation, including grounding end bushing or end bell bushing.

In the fourth paragraph, the second sentence of item number 1 is revised to read:

Conduits shall be cut to a maximum height of 2 inches above the foundation including grounding end bushing or end bell bushing.

Section 8-21, Permanent Signing
April 6, 2015

8-21.3(9)F Foundations

The first sentence of the first paragraph is revised to read:

The excavation and backfill shall conform to the requirements of Section 2-09.3.

Section 8-22, Pavement Marking
April 6, 2015

8-22.3(6) Removal of Pavement Markings

The second and third sentences of the first paragraph are revised to read:

Grinding to remove pavement markings is allowed prior to application of a Bituminous Surface Treatment. Grinding to remove pavement marking from hot mix asphalt and cement concrete pavements is allowed to a depth just above the pavement surface, then water blasting or shot blasting shall be required to remove the remaining markings.

Section 8-23, Temporary Pavement Markings
January 5, 2015

This section's content is deleted in its entirety and replaced with the following new sub-sections:

8-23.1 Description

The Work consists of furnishing, installing, and removing temporary pavement markings. Temporary pavement markings shall be provided where noted in the Plans; for all lane shifts and detours resulting from construction activities; or when permanent markings are removed because of construction operations.

8-23.2 Materials

Materials for temporary markings shall be paint, plastic, tape, raised pavement markers or flexible raised pavement markers. Materials for pavement markings shall meet the following requirements:

Raised Pavement Markers	9-21
Temporary Marking Paint	9-34.2(6)
Plastic	9-34.3
Glass Beads for Pavement Marking Materials	9-34.4
Temporary Pavement Marking Tape	9-34.5
Temporary Flexible Raised Pavement Markers	9-34.6

8.23.3 Construction Requirements

8-23.3(1) General

The Contractor shall select the type of pavement marking material in accordance with the Contract.

8-23.3(2) Preliminary Spotting

All preliminary layout and marking in preparation for application or removal of temporary pavement markings shall be the responsibility of the Contractor.

8-23.3(3) Preparation of Roadway Surface

Surface preparation for temporary pavement markings shall be in accordance with the manufacturer's recommendations.

8-23.3(4) Pavement Marking Application

8-23.3(4)A Temporary Pavement Markings – Short Duration

Temporary pavement markings – short duration shall meet the following requirements:

Temporary Center Line – A BROKEN line used to delineate adjacent lanes of traffic moving in opposite directions. The broken pattern shall be based on a 40-foot unit, consisting of a 4-foot line with a 36-foot gap if paint or tape is used. If temporary raised pavement markers are used, the pattern shall be based on a 40-foot unit, consisting of a grouping of three temporary raised pavement markers, each spaced 3 feet apart, with a 34 foot gap.

Temporary Edge Line – A SOLID line used on the edges of Traveled Way. The line shall be continuous if paint or tape is used. If temporary raised pavement markers are used, the line shall consist of markers installed continuously at 5-foot spacing.

Temporary Lane Line – A BROKEN line used to delineate adjacent lanes with traffic traveling in the same direction. The broken pattern shall be based on a 40-foot unit, consisting of a 4-foot line with a 36-foot gap, if paint or tape is used. If temporary raised pavement markers are used, the pattern shall be based on a 40-foot unit, consisting of a grouping of three temporary raised pavement markers, each spaced 3 feet apart, with a 34 foot gap.

Lane line and right edge line shall be white in color. Center line and left edge line shall be yellow in color. Edge lines shall be installed only if specifically required in the Contract. All temporary pavement markings shall be retroreflective.

8-23.3(4)A1 Temporary Pavement Marking Paint

Paint used for short duration temporary pavement markings shall be applied in one application at a thickness of 15 mils or 108 square feet per gallon. Glass beads shall be in accordance with Section 8-22.3(3)G.

8-23.3(4)A2 Temporary Pavement Marking Tape

Application of temporary pavement marking tape shall be in conformance with the manufacturer's recommendations.

Black mask pavement marking tape shall mask the existing line in its entirety.

8-23.3(4)A3 Temporary Raised Pavement Markers

Temporary raised pavement markers are not allowed on bituminous surface treatments.

8-23.3(4)A4 Temporary Flexible Raised Pavement Markers

Flexible raised pavement markers are required for new applications of bituminous surface treatments. Flexible raised pavement markers are not allowed on other pavement types unless otherwise specified or approved by the Engineer. Flexible raised pavement markers shall be installed with the protective cover in place. The cover shall be removed immediately after spraying asphaltic material.

8-23.3(4)B Temporary Pavement Markings – Long Duration

Application of paint, pavement marking tape and plastic for long duration pavement markings shall meet the requirements of Section 8-22.3(3); application of raised pavement markers shall meet the requirements of Section 8-09.3; and application of flexible pavement markings shall be in conformance with the manufacturer's recommendations.

8-23.3(4)C Tolerance for Lines

Tolerance for lines shall conform to Section 8-22.3(4).

8-23.3(4)D Maintenance of Pavement Markings

Temporary pavement markings shall be maintained in serviceable condition throughout the project until permanent pavement markings are installed. As directed by the Engineer; temporary pavement markings that are damaged, including normal wear by traffic, shall be repaired or replaced immediately. Repaired and replaced pavement markings shall meet the requirements for the original pavement marking.

8-23.3(4)E Removal of Pavement Markings

Removal of temporary paint is not required prior to paving; all other temporary pavement markings shall be removed.

All temporary pavement markings that are required on the wearing course prior to construction of permanent pavement markings and are not a part of the permanent markings shall be completely removed concurrent with or immediately subsequent to the construction of the permanent pavement markings. Temporary flexible raised pavement markers on bituminous surface treatment pavements shall be cut off flush with the surface if their location conflicts with the alignment of the permanent pavement markings. All other temporary pavement markings shall be removed in accordance with Section 8-22.3(6).

All damage to the permanent Work caused by removing temporary pavement markings shall be repaired by the Contractor at no additional cost to the Contracting Agency.

8-23.4 Measurement

Temporary pavement markings will be measured by the linear foot of each installed line or grouping of markers, with no deduction for gaps in the line or markers and no additional measurement for the second application of paint required for long duration paint lines. Short duration and long duration temporary pavement markings will be measured for the initial installation only.

8-23.5 Payment

Payment will be made in accordance with Section 1-04.1, for each of the following Bid items that are included in the Proposal:

"Temporary Pavement Marking – Short Duration", per linear foot.

"Temporary Pavement Marking – Long Duration", per linear foot.

The unit Contract price per linear foot for "Temporary Pavement Marking – Short Duration" and "Temporary Pavement Marking – Long Duration" shall be full pay for all Work.

Section 9-01, Portland Cement
January 5, 2015

9-01.2(3) Low Alkali Cement

This section is revised to read:

When low alkali portland cement is required, the percentage of alkalies in the cement shall not exceed 0.60 percent by weight calculated as Na_20 plus 0.658 K_20. This limitation shall apply to all types of portland cement.

9-01.2(4) Blended Hydraulic Cement

The first paragraph is revised to read:

Blended hydraulic cement shall be either Type IP(X)(MS) or Type IS(X)(MS) cement conforming to AASHTO M 240 or ASTM C 595, except that the portland cement used to produce blended hydraulic cement shall not contain more than 0.75 percent alkalies by weight calculated as Na_20 plus 0.658 K_20 and shall meet the following additional requirements:

1. Type IP(X)(MS) - Portland-Pozzolan Cement where (X) equals the targeted percentage of fly ash, the fly ash is limited to a maximum of 35 percent by weight of the cementitious material; (MS) indicates moderate sulfate resistance.

2. Type IS(X)(MS) - Portland Blast- Furnace Slag Cement, where: (X) equals the targeted percentage of ground granulated blast-furnace slag, the ground granulated blast furnace slag is limited to a maximum of 50 percent by weight of the cementitious material; (MS) indicates moderate sulfate resistance.

The first sentence of the second paragraph is revised to read:

The source and weight of the fly ash or ground granulated blast-furnace slag shall be certified on the cement mill test report or cement certificate of analysis and shall be reported as a percent by weight of the total cementitious material.

9-01.3 Tests and Acceptance

The first paragraph is revised to read:

Cement may be accepted by the Engineer based on the cement mill test report number or cement certificate of analysis number indicating full conformance to the Specifications. All shipments of the cement to the Contractor or concrete supplier shall identify the applicable cement mill test report number or cement certificate of analysis number and shall be provided by the Contractor or concrete supplier with all concrete deliveries.

The second paragraph is revised to read:

Cement producers/suppliers that certify portland cement or blended cement shall participate in the Cement Acceptance Program as described in WSDOT Standard Practice QC 1.

9-01.4 Storage on the Work Site

This section is revised to read:

At the request of the Engineer, the Contractor shall provide test data to show that cement stored on site for longer than 60 days meets the requirements of 9-01. Tests shall be conducted on samples taken from the site in the presence of the Engineer. Test results that meet the requirements of 9-01 shall be valid for 60 days from the date of sampling, after which the Engineer may require further testing.

Section 9-02, Bituminous Materials
April 6, 2015

9-02.1(4) Performance Graded Asphalt Binder (PGAB)

The first paragraph is supplemented with the following:

For HMA with greater than 20 percent RAP by total weight of HMA or any amount of RAS the new asphalt binder, recycling agent and recovered asphalt (RAP and/or RAS) when blended in the proportions of the mix design shall meet the PGAB requirements of AASHTO M 320 Table 1 for the grade of asphalt binder specified by the Contract.

This section is supplemented with the following:

The recycling agent used to rejuvenate the recovered asphalt from recycled asphalt pavement (RAP) and reclaimed asphalt shingles (RAS) shall meet the specifications in Table 1:

Table 1		RA 1		RA 5		RA 25	
Test	**ASTM Test Method**	**Min.**	**Max.**	**Min.**	**Max.**	**Min.**	**Max.**
Viscosity @ 140˚F cSt	D2170 or D2171	50	150	200	800	1000	4000
Flashpoint COC, ˚F	D92	400		400		400	
Saturates, Wt. %	D2007		30		30		30
Specific Gravity	D70 or D2198	Report		Report		Report	
Tests on Residue from RTFC	D2872						
Viscosity Ratio[1]			3		3		3
Mass Change ± %			4		4		4

[1]Viscosity Ratio = RTFC Viscosity @ 140˚F, cSt / Original Viscosity @ 140˚F, cSt

9-02.1(6)A Polymerized Cationic Emulsified Asphalt CRS-2P

In the ninth row of the table, “Test” is revised to read “Tests”.

The eleventh row in the table is revised to read:

Elastic Recovery %	T 301[2]	50	

The last two rows of the table are deleted.

Footnote 2 below the table is revised to read:

2 The residue material for T 301 shall come from the modified distillation per note 1.

Footnote 3 below the table is deleted.

The last paragraph is deleted.

Section 9-03, Aggregates
April 6, 2015

9-03.1(2)C Use of Substandard Gradings

This section including title is deleted in its entirety and replaced with the following:

Vacant

9-03.1(4)C Grading

In the second paragraph, the first sentence is deleted.

The third paragraph is deleted.

9-03.1(5)B Grading

The last paragraph is revised to read:

The Contracting Agency may sample each aggregate component prior to introduction to the weigh batcher or as otherwise determined by the Engineer. Each component will be sieve analyzed separately in accordance with WSDOT FOP for WAQTC/AASHTO Test Method T-27/11. All aggregate components will be mathematically re-combined by the proportions (percent of total aggregate by weight) provided by the Contractor on Concrete Mix Design Form 350-040.

9-03.8(1) General Requirements

The first paragraph up until the colon is revised to read:

Preliminary testing of aggregates for source approval shall meet the following test requirements:

The list in the first paragraph is supplemented with the following:

Sand Equivalent	45 min.

The following new paragraph is inserted after the first paragraph:

Aggregate sources that have 100 percent of the mineral material passing the No. 4 sieve shall be limited to no more than 5 percent of the total weight of aggregate.

9-03.8(2) HMA Test Requirements

The second paragraph (up until the colon) is revised to read:

The mix design shall produce HMA mixtures when combined with RAP, RAS, coarse and fine aggregate within the limits set forth in Section 9-03.8(6) and mixed in the laboratory with the designated grade of asphalt binder, using the Superpave gyratory compactor in accordance with WSDOT FOP for AASHTO T 312, and at the required gyrations for N initial, N design, and N maximum with the following properties:

The third paragraph is revised to read:

The mix criteria for Hamburg Wheel-Track Testing and Indirect Tensile Strength do not apply to HMA accepted by commercial evaluation.

9-03.8(3)B Gradation – Recycled Asphalt Pavement and Mineral Aggregate

This section is supplemented with the following:

For HMA with greater than 20 percent RAP by total weight of HMA the RAP shall be processed to ensure that 100 percent of the material passes a sieve twice the size of the maximum aggregate size for the class of mix to be produced.

When any amount of RAS is used in the production of HMA the RAS shall be milled, crushed or processed to ensure that 100 percent of the material passes the ½ inch sieve. Extraneous materials in RAS such as metals, glass, rubber, soil, brick, tars, paper, wood and plastic shall not exceed 2.0 percent by mass as determined on material retained on the No. 4 sieve.

9-03.14(3) Common Borrow

This section is revised to read:

Material for common borrow shall consist of granular or nongranular soil and/or aggregate which is free of deleterious material. Deleterious material includes wood, organic waste, coal, charcoal, or any other extraneous or objectionable material. The material shall not contain more than 3 percent organic material by weight. The plasticity index shall be determined using test method AASHTO T 89 and AASHTO T 90.

The material shall meet one of the options in the soil plasticity table below.

Soil Plasticity Table

Option	Sieve	Percent Passing	Plasticity Index
1	No. 200	0 - 12	N/A
2	No. 200	12.1 - 35	6 or Less
3	No. 200	Above 35	0

All percentages are by weight.

If requested by the Contractor, the plasticity index may be increased with the approval of the Engineer.

9-03.14(4) Gravel Borrow for Structural Earth Wall

In the second table, the row beginning with “pH” is revised to read:

pH	WSDOT Test Method T 417	4.5 - 9	5 – 10

9-03.21(1) General Requirements

The following new paragraph is inserted after the second paragraph:

Reclaimed asphalt shingles samples shall contain less than the maximum percentage of asbestos fibers based on testing procedures and frequencies established in conjunction with the specifying jurisdiction and state or federal environmental regulatory agencies.

Section 9-04, Joint and Crack Sealing Materials
January 5, 2015

9-04.1(4) Elastomeric Expansion Joint Seals
In this section, “AASHTO M 220” is revised to read “ASTM D 2628”.

9-04.2(1) Hot Poured Joint Sealants
In the first paragraph, “AASHTO M 324” is revised to read “ASTM D 6690”.

9-04.2(2) Poured Rubber Joint Sealer
In item number 9, “WSDOT Test Method No. 412” is revised to read “ASTM D 5329”.

Section 9-05, Drainage Structures and Culverts
April 7, 2014

9-05.13 Ductile Iron Sewer Pipe
The first paragraph is deleted.

Section 9-06, Structural Steel and Related Materials
January 5, 2015

9-06.5(4) Anchor Bolts
The third sentence of the second paragraph is revised to read:

> Nuts for ASTM F 1554 Grade 36 or 55 black or galvanized anchor bolts shall conform to ASTM A 563, Grade A or DH.

Section 9-07, Reinforcing Steel
January 6, 2014

9-07.5(1) Epoxy-Coated Dowel Bars (for Cement Concrete Pavement Rehabilitation)
This section is revised to read:

> Epoxy-coated dowel bars shall be round plain steel bars of the dimensions shown in the Standard Plans. They shall conform to AASHTO M 31, Grade 60 or ASTM A 615, Grade 60 and shall be coated in accordance with ASTM A 1078 Type 2 coating, except that the bars may be cut to length after being coated. Cut ends shall be coated in accordance with ASTM A 1078 with a patching material that is compatible with the coating, inert in concrete and recommended by the coating manufacturer. The thickness of the epoxy coating shall be 10 mils plus or minus 2 mils. The Contractor shall furnish a written certification that properly identifies the coating material, the number of each batch of coating material used, quantity represented, date of manufacture, name and address of manufacturer, and a statement that the supplied coating material meets the requirements of ASTM A 1078 Type 2 coating. Patching material, compatible with the coating material and inert in concrete and recommended by the manufacturer shall be supplied with each shipment for field repairs by the Contractor.

9-07.5(2) Corrosion Resistant Dowel Bars (for Cement Concrete Pavement)
This section’s title is revised to read:

9-07.5(2) Corrosion Resistant Dowel Bars (for Cement Concrete Pavement and Cement Concrete Pavement Rehabilitation)

Section 9-08, Paints and Related Materials
January 5, 2015

9-08.1(2)H Top Coat, Single Component, Moisture-Cured Polyurethane

The second paragraph is revised to read:

Color and Gloss: As specified in the Plans or Special Provisions

The last item in the requirements list is revised to read:

The top coat shall be a gloss or semi-gloss

9-08.1(8) Standard Colors

The second paragraph is deleted.

The third paragraph is revised to read:

Unless otherwise specified, all top or finish coats shall be gloss or semi-gloss, with the paint falling within the range of greater than 70 for gloss and 35 to 70 for semi-gloss on the 60-degree gloss meter.

Section 9-09, Timber and Lumber
January 6, 2014

9-09.3(1) General Requirements

The fourth paragraph is revised to read:

All orders of treated timber and lumber shall be accompanied by a Certificate of Treatment record. The Certificate of Treatment showing conformance to this specification and AWPA standards shall include the following information:

Name and location of the wood preserving company,

Customer identification,

Date of treatment and charge number,

Type of chemical used and amount of retention,

Treating process and identification of the Specification used,

Boring records verifying treatment penetration for timber and lumber with a nominal dimension of 6” x 6” or larger,

Description of material that was treated, and

Signature of a responsible plant official.

The fifth paragraph is deleted.

The first sentence in the last paragraph is revised to read:

All timber and lumber to be used in aquatic environments, unless specified otherwise in the Contract, shall be chemically treated using Western Wood Preservers Institute Best Management Practices (BMPs).

Section 9-10, Piling
March 3, 2014

9-10.5 Steel Piling

This section is revised to read:

The material for rolled steel piling H-piling and pile splices shall conform to ASTM A 36, ASTM A 572 or ASTM A 992. The material for steel pipe piling and splices shall conform to one of the following requirements except as specifically noted in the Plans:

1. API 5L Grade X42 or X52 material may be used for longitudinal seam welded or helical (spiral) seam submerged-arc welded pipe piles of any diameter.

2. ASTM A 252 Grade 2 or 3 material may be used for longitudinal seam welded or helical (spiral) seam submerged-arc welded pipe piles of any diameter. For the purposes of welding and prequalification of base metal, steel pipe pile designated as ASTM A 252 may be treated as prequalified provided the chemical composition conforms to a prequalified base metal classification listed in Table 3.1 of the AWS D1.1/D1.1M, latest edition, Structural Welding Code, the grade of pipe piling meets or exceeds the grade specified in the Plans, and the carbon equivalent (CE) is a maximum of 0.45-percent.

3. ASTM A 572 or ASTM A 588 material may be used for longitudinal seam welded piles of any diameter.

For helical (spiral) seam submerged-arc welded pipe piles, the maximum radial offset of strip/plate edges shall be 1/8 inch. The offset shall be transitioned with a taper weld and the slope shall not be less than a 1 in 2.5 taper. The weld reinforcement shall not be greater than 3/16 inches and misalignment of weld beads shall not exceed 1/8 inch.

Steel soldier piles, and associated steel bars and plates, shall conform to ASTM A 36, ASTM A 572 or ASTM A 992, except as otherwise noted in the Plans.

All steel piling may be accepted by the Engineer based on the Manufacturer's Certificate of Compliance submitted in accordance with Section 1-06.3. The manufacturer's certificate of compliance submittal for steel pipe piles shall be accompanied by certified mill test reports, including chemical analysis and carbon equivalence, for each heat of steel used to fabricate the steel pipe piling.

Section 9-13, Riprap, Quarry Spalls, Slope Protection, and Rock for Erosion and Scour Protection and Rock Walls
January 5, 2015

This section's content is deleted.

9-13.1 Loose Riprap

This section's content, including title and subsections, is revised to read the following:

9-13.1 Riprap and Quarry Spalls

9-13.1(1) General

Riprap and quarry spalls shall consist of broken stone or broken concrete rubble and shall be free of rock fines, soil, or other extraneous material. Concrete rubble shall not be contaminated by foreign materials such as fibers, wood, steel, asphalt, sealant, soil, plastic and other contaminants or deleterious material. Concrete rubble that is imported to the job site will require testing and certification for toxicity characteristics per Section 9-03.21(1).

The grading of the riprap shall be determined by the Engineer by visual inspection of the load before it is dumped into place, or, if so ordered by the Engineer, by dumping individual loads on a flat surface and sorting and measuring the individual rocks contained in the load. Should the riprap contain insufficient spalls, as defined in Section 9-13.1(5), the Contractor shall furnish and place supplementary spall material.

Riprap and quarry spalls shall be free from segregation, seams, cracks, and other defects tending to destroy its resistance to weather and shall conform to the following requirements for quality.

Aggregate Property	Test Method	Requirement
Degradation Factor	WSDOT T 113	15 minimum
Los Angeles Wear, 500 Rev.	AASHTO T 96	50% maximum
Specific Gravity, SSD	AASHTO T 85	2.55 minimum

9-13.1(2) Heavy Loose Riprap

Heavy loose riprap shall meet the following requirements for grading:

	Minimum Size	Maximum Size
40% to 90%	1 ton (½ cubic yd.)	
70% to 90%	300 lbs. (2 cu. ft.)	
10% to 30%	3 inch	50 lbs. (spalls)

9-13.1(3) Light Loose Riprap

Light loose riprap shall meet the following requirements for grading:

	Size Range	Maximum Size
20% to 90%	300 lbs. to 1 ton (2 cu. ft. to ½ cu. yd.)	
15% to 80%	50 lbs. to 1 ton (⅓ cu. ft. to ½ cu. yd.)	
10% to 20%	3 inch	50 lbs. (spalls)

9-13.1(4) Hand Placed Riprap

Hand placed riprap shall be as nearly rectangular as possible, 60 percent shall have a volume of not less than 1 cubic foot. No stone shall be used which is less than 6 inches thick, nor which does not extend through the wall.

9-13.1(5) Quarry Spalls

Quarry spalls shall meet the following requirements for grading:

Sieve Size	Percent Passing
8″	100
3″	40 max.
¾″	10 max.

9-13.2 Hand Placed Riprap

This section, including title, is deleted in its entirety and replaced with the following:

9-13.2 Vacant

9-13.4 Rock for Erosion Control and Scour Protection

The last sentence is revised to read:

The use of recycled materials and concrete rubble is not permitted for this application.

9-13.6 Quarry Spalls

This section, including title, is deleted in its entirety and replaced with the following:

9-13.6 Vacant

Section 9-14, Erosion Control and Roadside Planting
January 5, 2015

9.14.1 Soil

This section, including title, is revised to read:

9-14.1 Topsoil

Topsoil shall not contain any recycled material, foreign materials, or any listed Noxious and Nuisance weeds of any Class designated by authorized State or County officials. Aggregate shall not comprise more than 10% by volume of Topsoil and shall not be greater than two inches in diameter.

9-14.1(2) Topsoil Type B

The last sentence of the second paragraph is deleted.

9-14.2 Seed

This section is revised to read:

Seed of the type specified shall be certified in accordance with WAC 16-302. Seed mixes shall be commercially prepared and supplied in sealed containers. The labels shall show:

(1) Common and botanical names of seed

(2) Lot number

(3) Net weight

(4) Pounds of Pure live seed (PLS) in the mix

(5) Origin of seed

All seed vendors must have a business license issued by supplier's state or provincial Department of Licensing with a "seed dealer" endorsement.

9-14.4(3) Bark or Wood Chips

This section's title is revised to read:

Bark or Wood Chip Mulch

The first paragraph is revised to read:

Bark or wood chip mulch shall be derived from fir, pine, or hemlock species. It shall not contain resin, tannin, or other compounds in quantities that would be detrimental to plant life. Sawdust shall not be used as mulch. Mulch produced from finished wood products or construction debris will not be allowed.

9-14.4(6) Gypsum

The first sentence is revised to read:

Gypsum shall consist of Calcium Sulfate ($CaSO_4 \cdot 2H_2O$) in a pelletized or granular form.

9-14.4(7) Tackifier

This section is revised to read:

Tackifiers are used as a tie-down for soil, compost, seed, and/or mulch. Tackifiers shall contain no growth or germination-inhibiting materials and shall not reduce infiltration rates. Tackifiers shall hydrate in water and readily blend with other slurry materials.

The Contractor shall provide test results documenting the tackifier meets the requirements for Acute Toxicity, Solvents, and Heavy Metals as required in Table 1 in Section 9-14.4(2). The tests shall be performed at the manufacturer's recommended application rate.

9-14.4(8) Compost

The second paragraph is revised to read:

Compost production and quality shall comply with WAC 173-350.

9-14.4(8)A Compost Submittal Requirements

Item 2 is revised to read:

5. A copy of the Solid Waste Handling Permit issued to the manufacturer by the Jurisdictional Health Department in accordance with WAC 173-350 (Minimum Functional Standards for Solid Waste Handling).

9-14.6(1) Description

Item number 3 in the fourth paragraph is revised to read:

6. Live pole cuttings shall have a diameter between 2 inches and 3.5 inches. Live poles shall have no more than three branches which must be located at the top end of the pole and those branches shall be pruned back to the first bud from the main stem.

9-14.6(2) Quality

The second and third paragraphs in this section are revised to read:

All plant material shall comply with State and Federal laws with respect to inspection for plant diseases and insect infestation. Plants must meet Washington State Department of Agriculture plant quarantines and have a certificate of inspection. Plants originating in Canada must be accompanied by a phytosanitary certificate stating the plants meet USDA health requirements.

All plant material shall be purchased from a nursery licensed to sell plants in their state or province.

Section 9-15, Irrigation System
August 4, 2014

9-15.18 Detectable Marking Tape

In the second paragraph, the table is supplemented with the following new row:

Non-Potable Water	Purple

Section 9-16, Fence and Guardrail
August 4, 2014

9-16.2(1)B Wood Fence Posts and Braces

In the table, the row beginning with “ACA” is deleted.

Section 9-29, Illumination, Signal, Electrical
April 6, 2015

9-29.1 Conduit, Innerduct, and Outerduct

This section is supplemented with the following new subsection:

9-29.1(9) Repair

Manufacturer repair kits shall be used for field repair of existing conduit, innerduct and outerduct. The conduit repair kit shall be manufactured specifically for the repair of existing damaged conduit, inner duct and outer duct. The repair kit shall be prepackaged and include the split conduit and split couplings necessary to restore the damaged conduit to the original inside dimensions including a water and air tight seal.

9-29.2(1)B Heavy Duty Junction Boxes

The second paragraph is revised to read:

The Heavy-Duty Junction Box steel frame, lid support and lid fabricated from steel plate and shapes shall be painted with a shop applied, inorganic zinc primer in accordance with Section 6-07.3. Ductile iron and gray iron castings shall not be painted.

The following new paragraph is inserted after the second paragraph:

The concrete used in Heavy-Duty Junction Boxes shall have a minimum compressive strength of 4,000 psi.

In the fourth paragraph (after the preceding Amendment is applied), the table is revised to read:

Materials	**Requirement**
Concrete	Section 6-02
Reinforcing Steel	Section 9-07
Lid	ASTM A 786 diamond plate steel, rolled from plate complying with ASTM A 572, grade 50 or ASTM A 588, and having a min. CVN toughness of 20 ft-lb at 40 degrees F. Or Ductile iron casting meeting Section 9-05.15
Frame and stiffener plates	ASTM A 572 grade 50 or ASTM A 588, both with min. CVN toughness of 20 ft-lb at 40 degrees F Or Gray iron casting meeting Section 9-05.15
Anchors (studs)	Section 9-06.15
Threaded Anchors for Gray Iron Frame	ASTM F1554 grade 55 Headed Anchor Requirements
Bolts, Studs, Nuts, Washers	ASTM F 593 or A 193, Type 304 or 316, or Stainless steel grade 302, 304, or 316 in accordance with approved shop drawings
Hinges and Locking and Latching Mechanism and associated Hardware and Bolts	In accordance with approved shop drawings
Safety Bars	In accordance with approved shop drawings

The last paragraph is revised to read:

The bearing seat and lid perimeter shall be free from burrs, dirt, and other foreign debris that would prevent solid seating. Bolts and nuts shall be liberally coated with anti-seize compound. Bolts shall be installed snug tight. The bearing seat and lid perimeter shall be machined to allow a minimum of 75 percent of the bearing areas to be seated with a tolerance of 0.0 to 0.005 inches measured with a feeler gage. The bearing area percentage will be measured for each side of the lid as it bears on the frame.

9-29.2(2) Standard Duty and Heavy-Duty Cable Vaults and Pull Boxes

This section's title is revised to read:

Small Cable Vaults, Standard Duty Cable Vaults, Heavy-Duty Cable Vaults, Standard Duty Pull Boxes, and Heavy-Duty Pull Boxes

In the first paragraph, the first sentence is revised to read:

Small, Standard Duty and Heavy-Duty Cable Vaults and Standard Duty and Heavy-Duty Pull Boxes shall be constructed as a concrete box and as a concrete lid.

9-29.2(2)A Standard Duty Cable Vaults and Pull Boxes

This section's title is revised to read:

Small Cable Vaults, Standard Duty Cable Vaults, and Standard Duty Pull Boxes

The first paragraph is revised to read:

Small and Standard Duty Cable Vaults and Standard Duty Pull boxes shall be concrete and have a minimum load rating of 22,500 pounds and be tested in accordance with Section 9-29.2(1)C for concrete Standard Duty Junction Boxes.

In the second paragraph, the first sentence is revised to read:

Concrete for Small and Standard Duty Cable Vaults and Standard Duty Pull Boxes shall have a minimum compressive strength of 4,000 psi.

In the third paragraph, the first sentence is revised to read:

All Small and Standard Duty Cable Vaults and Standard Duty Pull Boxes placed in sidewalks, walkways, and shared-use paths shall have slip-resistant surfaces.

The fourth paragraph (up until the colon) is revised to read:

Materials for Small and Standard Duty Cable Vaults and Standard Duty Pull Boxes shall conform to the following:

9-29.3 Fiber Optic Cable, Electrical Conductors, and Cable

This section is supplemented with the following new subsection:

9-29.3(3) Wire Marking Sleeves

Wire marking sleeves shall be full-circle in design, non-adhesive, printable using an indelible ink and shall fit snugly on the wire or cable. Marking sleeves shall be made from a PVC or polyolefin, and provide permanent identification for wires and cables.

9-29.3(2)A4 Location Wire

This section is revised to read:

Location wire shall be steel core copper clad minimum size AWG 14 insulated conductor. The insulation shall be orange High Molecular Weight High Density Polyethylene (HMHDPE).

9-29.16 Vehicular Signal Heads, Displays, and Housing

The last sentence of the last paragraph is revised to read:

A 1-inch-wide strip of yellow retro-reflective, type IV prismatic sheeting, conforming to the requirements of Section 9-28.12, shall be applied around the perimeter of each backplate with the exception of installations where all sections of the display will be dark as part of normal operation such as ramp meters, hawk signals and tunnels.

Section 9-31, Elastomeric Bearing Pads
August 4, 2014

This section's title is revised to read:

Elastomeric Pads

9-31.1 Requirements

In the first paragraph, the word "bearing" is deleted from the first sentence.

In the first sentence of the second paragraph, the word "bearing" is deleted and replaced with "elastomeric".

In the last sentence of the second paragraph, the word "Bearing" is deleted and replaced with "Elastomeric".

In the third paragraph, the word "bearing" is deleted and replaced with the word "elastomeric".

Section 9-32, Mailbox Support
August 4, 2014

9-32.7 Type 2 Mailbox Support

The first sentence is revised to read:

Type 2 mailbox supports shall be 2-inch 14-gage steel tube and shall meet the NCHRP 350 or the Manual for Assessing Safety Hardware (MASH) crash test criteria.

Section 9-34, Pavement Marking Material
January 5, 2015

9-34.2 Paint

The second paragraph is revised to read:

Blue and black paint shall comply with the requirements of yellow paint in Section 9-34.2(4) and Section 9-34.2(5), with the exception that blue and black paints do not need to meet the requirements for titanium dioxide, directional reflectance, and contrast ratio.

9-34.4 Glass Beads for Pavement Marking Materials

In the third paragraph, the table titled "Metal Concentration Limits" is revised to read:

Metal Concentration Limits		
Element	**Test Method**	**Max. Parts Per Million (ppm)**
Arsenic	EPA 3052 SW-846 6010C	10.0
Barium	EPA 3052 SW-846 6010C	100.0
Cadmium	EPA 3052 SW-846 6010C	1.0
Chromium	EPA 3052 SW-846 6010C	5.0
Lead	EPA 3052 SW-846 6010C	50.0
Silver	EPA 3052 SW-846 6010C	5.0
Mercury	EPA 3052 SW-846 7471B	4.0

9-34.5 Temporary Pavement Marking Tape

This section is revised to read:

Biodegradable tape with paper backing is not allowed.

This section is supplemented with the following new sub-sections:

9-34.5(1) Temporary Pavement Marking Tape – Short Duration

Temporary pavement marking tape for short duration shall conform to ASTM D4592 Type II except that black tape, black mask tape and the black portion of the contrast removable tape, shall be non-reflective.

9-34.5(2) Temporary Pavement Marking Tape – Long Duration

Temporary pavement marking tape for long duration shall conform to ASTM D4592 Type I. Temporary pavement marking tape for long duration, except for black tape, shall have a minimum initial coefficient of retroreflective luminance of 200 $mcd*m^{-2}*lx^{-1}$ when measured in accordance with ASTM E 2832 or ASTM E 2177. Black tape, black mask tape and the black portion of the contrast removable tape, shall be non-reflective.

9-34.6 Temporary Raised Pavement Markers

This section's title is revised to read:

Temporary Flexible Raised Pavement Markers

The second paragraph is deleted.

Section 9-35, Temporary Traffic Control Materials
August 4, 2014

9-35.0 General Requirements

The following item is deleted from the list of temporary traffic control materials:

Barrier Drums

The last sentence of the second paragraph is revised to read:

Certification for crashworthiness according to NCHRP 350 or the Manual for Assessing Safety Hardware (MASH) will be required as described in Section 1-10.2(3).

9-35.2 Construction Signs

The first sentence is revised to read:

Construction signs shall conform to the requirements of the MUTCD and shall meet the requirements of NCHRP Report 350 for Category 2 devices or MASH.

9-35.7 Traffic Safety Drums

The third paragraph is revised to read:

Drums and light units shall meet the crashworthiness requirements of NCHRP 350 or MASH as described in Section 1-10.2(3).

9-35.8 Barrier Drums

This section including title is deleted in its entirety and replaced with the following:

9-35.8 Vacant

9-35.12 Transportable Attenuator

In the first paragraph, the fourth sentence is revised to read:

The Contractor shall provide certification that the transportable attenuator complies with NCHRP 350 Test level 3 or MASH Test Level 3 requirements.

9-35.13 Tall Channelizing Devices

In the sixth paragraph, the last sentence is revised to read:

The method of attachment must ensure that the light does not separate from the device upon impact and light units shall meet the crashworthiness requirements of NCHRP 350 or MASH as described in Section 1-10.2(3).

1

SPECIAL PROVISIONS

The following Special Provisions are made a part of this contract and supersede any conflicting provisions of the 2014 Standard Specifications for Road, Bridge and Municipal Construction and the foregoing Amendments to the Standard Specifications.

Several types of Special Provisions are included in this contract; General, Region, Bridges and Structures, and Project Specific. Special Provisions types are differentiated as follows:

(date)	General Special Provision
(******)	Notes a revision to a General Special Provision and also notes a Project Specific Special Provision.
(Regions[1] date)	Region Special Provision
(BSP date)	Bridges and Structures Special Provision
(APWA GSP)	Local Agency General Special Provision, which has been approved by the APWA Div. 1 Subcommittee.
(date) Sk. Co.	Skagit County General Special Provision

General Special Provisions are similar to Standard Specifications in that they typically apply to many projects, usually in more than one Region. Usually, the only difference from one project to another is the inclusion of variable project data, inserted as a "fill-in".

Region Special Provisions are commonly applicable within the designated Region. Region designations are as follows:

Regions[1]	
NWR	Northwest Region
WSF	Washington State Ferries Division

Bridges and Structures Special Provisions are similar to Standard Specifications in that they typically apply to many projects, usually in more than one Region. Usually, the only difference from one project to another is the inclusion of variable project data, inserted as a "fill-in".

Project Specific Special Provisions normally appear only in the contract for which they were developed.

Skagit County General Special Provisions are only applicable in Skagit County Public Works contracts.

1

Division 1
General Requirements

DESCRIPTION OF WORK

(March 13, 1995)
This Contract provides for the improvement of approximately 2.2 miles of La Conner Whitney Road. The work to be performed under this contract includes: planing of existing pavement and hauling planings to a County owned pit; application of a 2" HMA Cl ½" PG 64-22 wearing course over the existing roadway; providing erosion control, trimming and cleanup, traffic control, signage; and other work, all in accordance with the attached Contract Plans, these Contract Provisions, and the Standard Specifications.

DEFINITIONS AND TERMS

Definitions
(March 8, 2013 APWA GSP)

Section 1-01.3 is revised as follows:

Delete the heading **Completion Dates** and the three paragraphs that follow it, and replace them with the following:

Dates

Bid Opening Date
The date on which the Contracting Agency publicly opens and reads the Bids.

Award Date
The date of the formal decision of the Contracting Agency to accept the lowest responsible and responsive Bidder for the Work.

Contract Execution Date
The date the Contracting Agency officially binds the Agency to the Contract.

Notice to Proceed Date
The date stated in the Notice to Proceed on which the Contract time begins.

Substantial Completion Date
The day the Engineer determines the Contracting Agency has full and unrestricted use and benefit of the facilities, both from the operational and safety standpoint, any remaining traffic disruptions will be rare and brief, and only minor incidental work, replacement of temporary substitute facilities, plant establishment periods, or correction or repair remains for the Physical Completion of the total Contract.

Physical Completion Date
The day all of the Work is physically completed on the project. All documentation required by the Contract and required by law does not necessarily need to be furnished by the Contractor by this date.

Completion Date
The day all the Work specified in the Contract is completed and all the obligations of the Contractor under the contract are fulfilled by the Contractor. All documentation required by the Contract and required by law must be furnished by the Contractor before establishment of this date.

Final Acceptance Date
The date on which the Contracting Agency accepts the Work as complete.

Supplement this Section with the following:

All references in the Standard Specifications, Amendments, or WSDOT General Special Provisions, to the terms “State”, “Department of Transportation”, “Washington State Transportation Commission”, “Commission”, “Secretary of Transportation”, “Secretary”, “Headquarters”, and “State Treasurer” shall be revised to read “Contracting Agency”.

All references to “State Materials Laboratory” shall be revised to read “Contracting Agency designated location”.

All references to “final contract voucher certification” shall be interpreted to mean the final payment form established by the Contracting Agency.

The venue of all causes of action arising from the advertisement, award, execution, and performance of the contract shall be in the Superior Court of the County where the Contracting Agency’s headquarters are located.

Additive
A supplemental unit of work or group of bid items, identified separately in the Bid Proposal, which may, at the discretion of the Contracting Agency, be awarded in addition to the base bid.

Alternate
One of two or more units of work or groups of bid items, identified separately in the Bid Proposal, from which the Contracting Agency may make a choice between different methods or material of construction for performing the same work.

Business Day
A business day is any day from Monday through Friday except holidays as listed in Section 1-08.5.

Contract Bond
The definition in the Standard Specifications for “Contract Bond” applies to whatever bond form(s) are required by the Contract Documents, which may be a combination of a Payment Bond and a Performance Bond.

Contract Documents
See definition for “Contract”.

Contract Time
The period of time established by the terms and conditions of the Contract within which the Work must be physically completed.

Notice of Award
The written notice from the Contracting Agency to the successful Bidder signifying the Contracting Agency’s acceptance of the Bid Proposal.

Notice to Proceed
The written notice from the Contracting Agency or Engineer to the Contractor authorizing and directing the Contractor to proceed with the Work and establishing the date on which the Contract time begins.

Traffic
Both vehicular and non-vehicular traffic, such as pedestrians, bicyclists, wheelchairs, and equestrian traffic.

Bid Procedures and Conditions

Prequalification of Bidders

Delete Section 1-02.1 and replace it with the following:

1-02.1 Qualifications of Bidder
(January 24, 2011 APWA GSP)

Before award of a public works contract, a bidder must meet at least the minimum qualifications of RCW 39.04.350(1) to be considered a responsible bidder and qualified to be awarded a public works project.

Plans and Specifications
(June 27, 2011 APWA GSP)

Delete Section 1-02.2 and replace it with the following:

Information as to where Bid Documents can be obtained or reviewed can be found in the Call for Bids (Advertisement for Bids) for the work.

After award of the contract, plans and specifications will be issued to the Contractor at no cost as detailed below:

To Prime Contractor	No. of Sets	Basis of Distribution
Reduced plans (11" x 17")	4	Furnished automatically upon award.
Contract Provisions	4	Furnished automatically upon award.
Large plans (e.g., 24" x 36")	2	Furnished only upon request.

Additional plans and Contract Provisions may be obtained by the Contractor from the source stated in the Call for Bids, at the Contractor's own expense.

Preparation of Proposal

(August 2, 2004)
The fifth and sixth paragraphs of Section 1-02.6 are deleted.

Delivery of Proposal

(5/15/13) Sk. Co.

Delete Section 1-02.9 and replace it with the following:

Each proposal shall be submitted in a sealed envelope, with the Project Name and Project Number as stated in the Call for Bids clearly marked on the outside of the envelope, or as otherwise required in the Bid Documents, to ensure proper handling and delivery.

The Contracting Agency will not open or consider any Bid Proposal that is received after the time specified in the Call for Bids for receipt of Bid Proposals, or received in a location other than that specified in the Call for Bids.

Public Opening Of Proposal

(10/27/10) Sk. Co.

Section 1-02.12 is supplemented with the following:

Sealed bids shall be received at the time and location specified in the call for bids, unless modified by addenda.

Irregular Proposals

(March 13, 2012 APWA GSP)

Item 1 in Section 1-02.13 is revised to read:

1. A proposal will be considered irregular and will be rejected if:

    a. The Bidder is not prequalified when so required;
    b. The authorized proposal form furnished by the Contracting Agency is not used or is altered;
    c. The completed proposal form contains any unauthorized additions, deletions, alternate Bids, or conditions;
    d. The Bidder adds provisions reserving the right to reject or accept the award, or enter into the Contract;
    e. A price per unit cannot be determined from the Bid Proposal;
    f. The Proposal form is not properly executed;
    g. The Bidder fails to submit or properly complete a Subcontractor list, if applicable, as required in Section 1-02.6;
    h. The Bidder fails to submit or properly complete a Disadvantaged Business Enterprise Certification, if applicable, as required in Section 1-02.6;
    i. The Bidder fails to submit written confirmation from each DBE firm listed on the Bidder's completed DBE Utilization Certification that they are in agreement with the bidders DBE participation commitment, if applicable, as required in Section 1-02.6, or if the written confirmation that is submitted fails to meet the requirements of the Special Provisions;
    j. The Bidder fails to submit DBE Good Faith Effort documentation, if applicable, as required in Section 1-02.6, or if the documentation that is

submitted fails to demonstrate that a Good Faith Effort to meet the Condition of Award was made;

k. The Bid Proposal does not constitute a definite and unqualified offer to meet the material terms of the Bid invitation; or

l. More than one proposal is submitted for the same project from a Bidder under the same or different names.

Disqualification of Bidders

(March 8, 2013 APWA GSP)

Section 1-02.14 is deleted and replaced with the following:

A Bidder will be deemed not responsible if the Bidder does not meet the mandatory bidder responsibility criteria in RCW 39.04.350(1), as amended; or does not meet the following Supplemental Criteria:

1. **Delinquent State Taxes**

A Criterion: The Bidder shall not owe delinquent taxes to the Washington State Department of Revenue without a payment plan approved by the Department of Revenue.

B. Documentation: The Bidder shall not be listed on the Washington State Department of Revenue's "Delinquent Taxpayer List" website: http://dor.wa.gov/content/fileandpaytaxes/latefiling/dtlwest.aspx , or if they are so listed, they must submit a written payment plan approved by the Department of Revenue, to the Contracting Agency by the deadline listed below.

2. **Federal Debarment**

A Criterion: The Bidder shall not currently be debarred or suspended by the Federal government.

B. Documentation: The Bidder shall not be listed as having an "active exclusion" on the U.S. government's "System for Award Management" database (www.sam.gov).

3. **Subcontractor Responsibility**

A Criterion: The Bidder's standard subcontract form shall include the subcontractor responsibility language required by RCW 39.06.020, and the Bidder shall have an established procedure which it utilizes to validate the responsibility of each of its subcontractors. The Bidder's subcontract form shall also include a requirement that each of its subcontractors shall have and document a similar procedure to determine whether the sub-tier subcontractors with whom it contracts are also "responsible" subcontractors as defined by RCW 39.06.020.

B. Documentation: The Bidder, if and when required as detailed below, shall submit a copy of its standard subcontract form for review by the Contracting Agency, and a written description of its procedure for validating the responsibility of subcontractors with which it contracts.

4. **Prevailing Wages**

A Criterion: The Bidder shall not have a record of prevailing wage violations as determined by WA Labor & Industries in the five years prior to the bid submittal date, that demonstrates a pattern of failing to pay workers prevailing wages, unless there are extenuating circumstances and such circumstances are deemed acceptable to the Contracting Agency.

B. Documentation: The Bidder, if and when required as detailed below, shall submit a list of all prevailing wage violations in the five years prior to the bid submittal date, along with an explanation of each violation and how it was resolved. The Contracting Agency will evaluate these explanations and the resolution of each complaint to determine whether the violation demonstrate a pattern of failing to pay its workers prevailing wages as required.

5. **Claims Against Retainage and Bonds**

A Criterion: The Bidder shall not have a record of excessive claims filed against the retainage or payment bonds for public works projects in the three years prior to the bid submittal date, that demonstrate a lack of effective management by the Bidder of making timely and appropriate payments to its subcontractors, suppliers, and workers, unless there are extenuating circumstances and such circumstances are deemed acceptable to the Contracting Agency.

B. Documentation: The Bidder, if and when required as detailed below, shall submit a list of the public works projects completed in the three years prior to the bid submittal date that have had claims against retainage and bonds and include for each project the following information:

- Name of project
- The owner and contact information for the owner;
- A list of claims filed against the retainage and/or payment bond for any of the projects listed;
- A written explanation of the circumstances surrounding each claim and the ultimate resolution of the claim.

6. **Public Bidding Crime**

A Criterion: The Bidder and/or its owners shall not have been convicted of a crime involving bidding on a public works contract in the five years prior to the bid submittal date.

B. Documentation: The Bidder, if and when required as detailed below, shall sign a statement (on a form to be provided by the Contracting Agency) that the Bidder and/or its owners have not been convicted of a crime involving bidding on a public works contract.

7. **Termination for Cause / Termination for Default**

A Criterion: The Bidder shall not have had any public works contract terminated for cause or terminated for default by a government agency in the five years prior to the bid submittal date, unless there are extenuating circumstances and such circumstances are deemed acceptable to the Contracting Agency.

B. Documentation: The Bidder, if and when required as detailed below, shall sign a statement (on a form to be provided by the Contracting Agency) that the Bidder has not had any public works contract terminated for cause or terminated for default by a government agency in the five years prior to the bid submittal date; or if Bidder was terminated, describe the circumstances. .

8. **Lawsuits**

A Criterion: The Bidder shall not have lawsuits with judgments entered against the Bidder in the five years prior to the bid submittal date that demonstrate a pattern of failing to meet the terms of contracts, unless there are extenuating circumstances and such circumstances are deemed acceptable to the Contracting Agency

B. Documentation: The Bidder, if and when required as detailed below, shall sign a statement (on a form to be provided by the Contracting Agency) that the Bidder has not had any lawsuits with judgments entered against the Bidder in the five years prior to the bid submittal date that demonstrate a pattern of failing to meet the terms of contracts, or shall submit a list of all lawsuits with judgments entered against the Bidder in the five years prior to the bid submittal date, along with a written explanation of the circumstances surrounding each such lawsuit. The Contracting Agency shall evaluate these explanations to determine whether the lawsuits demonstrate a pattern of failing to meet of terms of construction related contracts

As evidence that the Bidder meets the mandatory and supplemental responsibility criteria stated above, the apparent two lowest Bidders must submit to the Contracting Agency by 12:00 P.M. (noon) of the second business day following the bid submittal deadline, a written statement verifying that the Bidder meets all of the mandatory and supplemental criteria together with supporting documentation including but not limited to that detailed above (sufficient in the sole judgment of the Contracting Agency) demonstrating compliance with all mandatory and supplemental responsibility criteria. The Contracting Agency reserves the right to request such documentation from other Bidders as well, and to request further documentation as needed to assess Bidder responsibility. The Contracting Agency also reserves the right to obtain information from third-parties and independent sources of information concerning a Bidder's compliance with the mandatory and supplemental criteria, and to use that information in their evaluation. The Contracting Agency may (but is not required to) consider mitigating factors in determining whether the Bidder complies with the requirements of the supplemental criteria.

The basis for evaluation of Bidder compliance with these mandatory and supplemental criteria shall include any documents or facts obtained by Contracting Agency (whether

from the Bidder or third parties) including but not limited to: (i) financial, historical, or operational data from the Bidder; (ii) information obtained directly by the Contracting Agency from others for whom the Bidder has worked, or other public agencies or private enterprises; and (iii) any additional information obtained by the Contracting Agency which is believed to be relevant to the matter.

If the Contracting Agency determines the Bidder does not meet the bidder responsibility criteria above and is therefore not a responsible Bidder, the Contracting Agency shall notify the Bidder in writing, with the reasons for its determination. If the Bidder disagrees with this determination, it may appeal the determination within two (2) business days of the Contracting Agency's determination by presenting its appeal and any additional information to the Contracting Agency. The Contracting Agency will consider the appeal and any additional information before issuing its final determination. If the final determination affirms that the Bidder is not responsible, the Contracting Agency will not execute a contract with any other Bidder until at least two business days after the Bidder determined to be not responsible has received the Contracting Agency's final determination.

Request to Change Supplemental Bidder Responsibility Criteria Prior To Bid: Bidders with concerns about the relevancy or restrictiveness of the Supplemental Bidder Responsibility Criteria may make or submit requests to the Contracting Agency to modify the criteria. Such requests shall be in writing, describe the nature of the concerns, and propose specific modifications to the criteria. Bidders shall submit such requests to the Contracting Agency no later than five (5) business days prior to the bid submittal deadline and address the request to the Project Engineer or such other person designated by the Contracting Agency in the Bid Documents.

Scope of the Work

Coordination of Contract Documents, Plans, Special Provisions, Specifications, and Addenda

(March 13, 2012 APWA GSP)

The second paragraph of Section 1-04.2 is revised to read:

Any inconsistency in the parts of the contract shall be resolved by following this order of precedence (e.g., 1 presiding over 2, 2 over 3, 3 over 4, and so forth):

1. Addenda,
2. Proposal Form,
3. Special Provisions,
4. Contract Plans,
5. Amendments to the Standard Specifications,
6. Standard Specifications,
7. Contracting Agency's Standard Plans or Details (if any), and
8. WSDOT Standard Plans for Road, Bridge, and Municipal Construction.

Control of Work

Superintendents, Labor and Equipment of Contractor
(August 14, 2013 APWA GSP)

Delete the sixth and seventh paragraphs of Section 1-05.13.

Method of Serving Notices
(March 25, 2009 APWA GSP)

The second paragraph of Section 1-05.15 is revised to read:

All correspondence from the Contractor shall be directed to the Project Engineer. All correspondence from the Contractor constituting any notification, notice of protest, notice of dispute, or other correspondence constituting notification required to be furnished under the Contract, must be in paper format, hand delivered or sent via mail delivery service to the Project Engineer's office. Electronic copies such as e-mails or electronically delivered copies of correspondence will not constitute such notice and will not comply with the requirements of the Contract.

Add the following new section:

1-05.16 Water and Power
(October 1, 2005 APWA GSP)

The Contractor shall make necessary arrangements, and shall bear the costs for power and water necessary for the performance of the work, unless the contract includes power and water as a pay item.

Add the following new section:

1-05.17 Oral Agreements
(October 1, 2005 APWA GSP)

No oral agreement or conversation with any officer, agent, or employee of the Contracting Agency, either before or after execution of the contract, shall affect or modify any of the terms or obligations contained in any of the documents comprising the contract. Such oral agreement or conversation shall be considered as unofficial information and in no way binding upon the Contracting Agency, unless subsequently put in writing and signed by the Contracting Agency.

Legal Relations and Responsibilities to the Public

Permits and Licenses

Section 1-07.6 is supplemented with the following:

(1/6/11) Sk. Co.
No hydraulic permits are required for this project unless the Contractor's operations use, divert, obstruct, or change the natural flow or bed of any river or stream, or utilize any of the waters of the State or materials from gravel or sand bars, or from stream beds.

Load Limits

Section 1-07.7 is supplemented with the following:

(March 13, 1995)
If the sources of materials provided by the Contractor necessitates hauling over roads other than State Highways, the Contractor shall, at the Contractor's expense, make all arrangements for the use of the haul routes.

Contractor's Responsibility for Work

Repair of Damage

Section 1-07.13(4) is revised to read:

(August 6, 2001)
The Contractor shall promptly repair all damage to either temporary or permanent work as directed by the Engineer. For damage qualifying for relief under Sections 1-07.13(1), 1-07.13(2) or 1-07.13(3), payment will be made in accordance with Section 1-04.4. Payment will be limited to repair of damaged work only. No payment will be made for delay or disruption of work.

Utilities and Similar Facilities

Section 1-07.17 is supplemented with the following:

(April 2, 2007)
Locations and dimensions shown in the Plans for existing facilities are in accordance with available information obtained without uncovering, measuring, or other verification.

The following addresses and telephone numbers of utility companies known or suspected of having facilities within the project limits are supplied for the Contractor's convenience:

***

Puget Sound Energy
Contact: Jane Major
1660 Park Lane
Burlington, WA 98233
Phone: 360-766-5571

Public Utility District No. 1 of Skagit County
Contact: Mike Benton
1415 Freeway Drive
Mount Vernon, WA 98273
Phone: 360-424-7104

Frontier Communications
Contact: Bret Murdock
595 Pease Road
Burlington, WA 98233
Phone: 360-707-0641

Comcast
Contact: Bill Inama
400 Sequoia Drive
Bellingham, WA 98226
360-527-8243

Cascade Natural Gas
Contact: James Hobbs
1520 S. 2nd Street
Mount Vernon, WA 98273
Phone: 360-941-0499

Town of La Conner Public Works
Contact: Brian Lease
604 N. Third Street
La Conner, WA 98257
Phone: 360-466-3933

Utility Location Center
(One Call Center)
1-800-424-5555

***

Public Convenience and Safety

Construction Under Traffic

(6/24/11) Sk. Co

In the second paragraph of Section 1-07.23(1), the following new sentence is inserted after the first sentence:

No vehicle trip through the work zone may be stopped for more than 10 minutes without the prior approval of the Engineer.

Section 1-07.23(1) is supplemented with the following:

(January 2, 2012)
Work Zone Clear Zone
The Work Zone Clear Zone (WZCZ) applies during working and nonworking hours. The WZCZ applies only to temporary roadside objects introduced by the Contractor's operations and does not apply to preexisting conditions or permanent Work. Those work operations that are actively in progress shall be in accordance with adopted and approved Traffic Control Plans, and other contract requirements.

During nonworking hours equipment or materials shall not be within the WZCZ unless they are protected by permanent guardrail or temporary concrete

barrier. The use of temporary concrete barrier shall be permitted only if the Engineer approves the installation and location.

During actual hours of work, unless protected as described above, only materials absolutely necessary to construction shall be within the WZCZ and only construction vehicles absolutely necessary to construction shall be allowed within the WZCZ or allowed to stop or park on the shoulder of the roadway.

The Contractor's nonessential vehicles and employees private vehicles shall not be permitted to park within the WZCZ at any time unless protected as described above.

Deviation from the above requirements shall not occur unless the Contractor has requested the deviation in writing and the Engineer has provided written approval.

Minimum WZCZ distances are measured from the edge of traveled way and will be determined as follows:

Regulatory Posted Speed	Distance From Traveled Way (Feet)
35 mph or less	10 *
40 mph	15
45 to 55 mph	20
60 mph or greater	30

* or 2-feet beyond the outside edge of sidewalk

Minimum Work Zone Clear Zone Distance

(******)

There shall be no restrictions or interruptions for traffic on the day prior to a holiday or holiday weekend through the last day of the holiday or holiday weekend.

Lane restrictions shall be held to a minimum time and length needed for the immediate work. If the Engineer determines that the lane restrictions are causing congestion, the Contractor shall be required to open all lanes to traffic until the congestion is eliminated at no additional cost to the Contracting Agency.

Lane closures shall be allowed only during the following hours:

La Conner Whitney Road in the section between the roundabout at Morris Street in La Conner to 200 feet North of the Sullivan Slough Bridge including any operations that would impact the function of the roundabout at Morris Street:

Sun	7:00 pm	to	Mon	5:00 am
Mon	7:00 pm	to	Tues	5:00 am
Tues	7:00 pm	to	Wed	5:00 am
Wed	7:00 pm	to	Thurs	5:00 am
Thurs	7:00 pm	to	Fri	5:00 am

Should high volume hours differ from those specified as determined by the Engineer, the Contractor shall be required to adjust the hours of work accordingly. Exceptions to these restrictions may be considered by the Engineer on a case by case basis following a written request by the Contractor.

When the Contractor's construction operations are actually in progress, traffic may be restricted to one lane, subject to the above specifications.

Special events that generate increased traffic volumes may occur during the life of this project. Lane restrictions may be denied if severe traffic congestion is expected.

Prosecution and Progress

Add the following new section:

1-08.0 Preliminary Matters

(May 25, 2006 APWA GSP)

Add the following new section:

1-08.0(1) Preconstruction Conference

(October 10, 2008 APWA GSP)

Prior to the Contractor beginning the work, a preconstruction conference will be held between the Contractor, the Engineer and such other interested parties as may be invited. The purpose of the preconstruction conference will be:

1. To review the initial progress schedule;
2. To establish a working understanding among the various parties associated or affected by the work;
3. To establish and review procedures for progress payment, notifications, approvals, submittals, etc.;
4. To establish normal working hours for the work;
5. To review safety standards and traffic control; and
6. To discuss such other related items as may be pertinent to the work.

The Contractor shall prepare and submit at the preconstruction conference the following:

1. A breakdown of all lump sum items;
2. A preliminary schedule of working drawing submittals; and
3. A list of material sources for approval if applicable.

(******)

4. The SPCC Plan
5. A list of Emergency Contacts including those for after working hours.
6. The TESC plan.
7. Any Traffic Control Plans that the Contractor plans to submit.

Subcontracting

(******)

Section 1-08.1 is revised as follows:

The eighth paragraph is deleted.

Prosecution of Work

Delete Section 1-08.4 in its entirety, and replace it with the following:

1-08.4 Notice to Proceed and Prosecution of Work

(June 27, 2011 APWA GSP)

Notice to Proceed will be given after the contract has been executed and the contract bond and evidence of insurance have been approved and filed by the Contracting Agency. The Contractor shall not commence with the work until the Notice to Proceed has been given by the Engineer. The Contractor shall commence construction activities

on the project site within ten days of the Notice to Proceed Date, unless otherwise approved in writing. The Contractor shall diligently pursue the work to the physical completion date within the time specified in the contract. Voluntary shutdown or slowing of operations by the Contractor shall not relieve the Contractor of the responsibility to complete the work within the time(s) specified in the contract.

When shown in the Plans, the first order of work shall be the installation of high visibility fencing to delineate all areas for protection or restoration, as described in the Contract. Installation of high visibility fencing adjacent to the roadway shall occur after the placement of all necessary signs and traffic control devices in accordance with 1-10.1(2). Upon construction of the fencing, the Contractor shall request the Engineer to inspect the fence. No other work shall be performed on the site until the Contracting Agency has accepted the installation of high visibility fencing, as described in the Contract.

Time for Completion

(11/4/10) Sk. Co.

Revise the third paragraph of Section 1-08.5 to read:

Contract time shall begin on the tenth working day following the Notice to Proceed Date. If the Contractor starts Work on the project at an earlier date, then Contract time shall begin on the first working day when onsite Work begins.

Section 1-08.5 is supplemented with the following:

(March 13, 1995)
This project shall be physically completed within *** 15 working days. ***

Liquidated Damages
(August 14, 2013 APWA GSP)

The fourth paragraph of Section 1-08.9 is revised to read:

When the Contract Work has progressed to Substantial Completion as defined in the Contract, the Engineer may determine that the work is Substantially Complete. The Engineer will notify the Contractor in writing of the Substantial Completion Date. For overruns in Contract time occurring after the date so established, the formula for liquidated damages shown above will not apply. For overruns in Contract time occurring after the Substantial Completion Date, liquidated damages shall be assessed on the basis of direct engineering and related costs assignable to the project until the actual Physical Completion Date of all the Contract Work. The Contractor shall complete the remaining Work as promptly as possible. Upon request by the Project Engineer, the Contractor shall furnish a written schedule for completing the physical Work on the Contract.

Measurement and Payment

Weighing Equipment

General Requirements for Weighing Equipment

(2/24/06) Sk. Co.

Section 1-09.2(1), is revised as follows:

In the **Weighers** subsection, the last sentence of the first paragraph is deleted.

Section 1-09.2(1), is supplemented with the following:

The Engineer will not provide a scale person to weigh and record the weights of any material. The Contractor shall provide a licensed weigher as an operator to weigh and record the weights of all material. All weight tickets shall have the weigher's seal placed on them. All expenses incurred to provide a weigher shall be incidental to the various bid items. The Contractor shall supply the Engineer with a copy of the scale certifications, and the licenses of the weigh master and the weigher.

Force Account
(October 10, 2008 APWA GSP)

Section 1-09.6 is supplemented with the following:

The Contracting Agency has estimated and included in the Proposal, dollar amounts for all items to be paid per force account, only to provide a common proposal for Bidders. All such dollar amounts are to become a part of Contractor's total bid. However, the Contracting Agency does not warrant expressly or by implication, that the actual amount of work will correspond with those estimates. Payment will be made on the basis of the amount of work actually authorized by Engineer.

(10/17/12) Sk. Co

Payment for unanticipated work performed during construction shall be made using the estimated Bid item "Unanticipated Site Work". Measurement and payment will be made in accordance with Section 1-09.6.

Claims Resolution

Claims $250,000 or Less
(October 1, 2005 APWA GSP)

Delete Section 1-09.13(3) and replace it with the following:

The Contractor and the Contracting Agency mutually agree that those claims that total $250,000 or less, submitted in accordance with Section 1-09.11 and not resolved by nonbinding ADR processes, shall be resolved through litigation unless the parties mutually agree in writing to resolve the claim through binding arbitration.

Administration of Arbitration
(October 1, 2005 APWA GSP)

The third paragraph of Section 1-09.13(3)A is revised to read:

The Contracting Agency and the Contractor mutually agree to be bound by the decision of the arbitrator, and judgment upon the award rendered by the arbitrator may be entered in the Superior Court of the county in which the Contracting Agency's headquarters are located. The decision of the arbitrator and the specific basis for the decision shall be in writing. The arbitrator shall use the contract as a basis for decisions.

Temporary Traffic Control

Traffic Control Management

General

Section 1-10.2(1) is supplemented with the following:

(December 1, 2008)
Only training with WSDOT TCS card and WSDOT training curriculum is recognized in the State of Washington. The Traffic Control Supervisor shall be certified by one of the following:

The Northwest Laborers-Employers Training Trust
27055 Ohio Ave.
Kingston, WA 98346
(360) 297-3035

Evergreen Safety Council
401 Pontius Ave. N.
Seattle, WA 98109
1-800-521-0778 or
(206) 382-4090

The American Traffic Safety Services Association
15 Riverside Parkway, Suite 100
Fredericksburg, Virginia 22406-1022
Training Dept. Toll Free (877) 642-4637
Phone: (540) 368-1701

Traffic Control Supervisor
(3/7/12) Sk. Co.

The third paragraph of Section 1-10.2(1) B is supplemented with the following:

8. Patrolling and maintaining traffic control as described in Section 1-10.3(2)E.

Measurement

Item Bids With Lump Sum for Incidentals

Section 1-10.4(2) is supplemented with the following:

(August 2, 2004)
The bid proposal does not contain the item "Project Temporary Traffic Control," lump sum. The provisions of Section 1-10.4(2) shall apply.

(6/21/11) Sk. Co

Section 1-10.4(2) has been revised as follows:

The second paragraph is revised to read:

No specific unit of measurement will apply to the lump sum item of "Traffic Control Supervisor." Duties of the Traffic Control Supervisor will include patrolling and maintaining traffic control measures as described in Section 1-10.3(2)E.

The third and fourth paragraphs are revised to read:

"Traffic Control Labor" will be measured by the hour. Time spent on activities other than those described herein will not be measured under this item. Hours will be measured for each person engaged in any one of the following activities:

- Flagging and Spotting. Hours will be measured for each flagging or spotting station, shown on an approved Traffic Control Plan, when that station is staffed in accordance with Section 1-10.3(1)A. When a flagging station is staffed on an intermittent basis, no deduction will be made in measured hours provided that the person staffing the station is in a standby mode and is not performing other duties.

- Operating a pilot vehicle during one-way piloted traffic control.

- Operating a traffic control vehicle or a chase vehicle during a rolling slowdown operation.

- Operating a vehicle or placing/removing traffic control devices during the setup or takedown of a lane closure. Performing preliminary work to prepare for placing and removing these devices.

- Operating any of the moving traffic control equipment, or adjusting signing during a mobile operation as described in Section 1-10.3(2)D.

- Placing and removing Class B construction signs. Performing preliminary work to prepare for placing and removing these signs.

- Relocation of Portable Changeable Message Signs within the project limits.

- Installing and removing Barricades, Traffic Safety Drums, Barrier Drums, Cones, Tubular Markers and Warning Lights and Flashers to carry out approved Traffic Control Plan(s). Performing preliminary work to prepare for installing these devices.

Payment

Item Bids with Lump Sum for Incidentals

(6/21/11) Sk. Co

.

Section 1-10.5(2) has been revised as follows:

The second paragraph has been revised to read:

The lump sum contract payment shall be full compensation for all costs incurred by the Contractor in performing the contract work defined in Section 1-10.2(1)B and Section 1-10.3(2)E.

The third, fourth, fifth, and sixth paragraphs have been revised to read:

" Traffic Control Labor", per hour.

The unit contract price, when applied to the number of units measured for this item in accordance with Section 1-10.4(2), shall be full compensation for all costs incurred by the Contractor in performing the contract work defined in Section 1-10.3(1)A and as specifically described for this item in Section 1-10.4(2).

Division 2
Earthwork

Removal of Structures and Obstructions

Description

Section 2-02.1 is supplemented with the following:

(March 13, 1995)
This work shall consist of removing miscellaneous traffic items.

Construction Requirements

Section 2-02.3 is supplemented with the following:

(March 13, 1995)
Removing Miscellaneous Traffic Items
The following miscellaneous traffic items shall be removed and disposed of:

*** All raised pavement markers within the paving limits.***

Payment

Section 2-02.5 is supplemented with the following:

(September 30, 1996)
"Removing Miscellaneous Traffic Item", lump sum.

HAUL

Description

Section 2-04.1 is revised as follows:

(2/28/06) Sk. Co.
The second and third paragraphs are deleted.

Payment

(7/9/10) Sk. Co.
This section is deleted and replaced with the following:

All haul costs (on and off-site) shall be included in the unit contract prices for all items requiring hauling.

TRIMMING AND CLEANUP

Description

(9/11/2013) Sk. Co.

Section 2-11.1 is revised to read:

This work consists of dressing and trimming the entire Roadway(s) improved under the Contract, including Frontage Roads, connecting ramps, Auxiliary Lanes, and approach roads. This Work extends to Roadbeds, Shoulders, lawns and ditches.

The Contractor shall also trim and clean up the staging areas and any other area the Contractor uses for construction operations.

(******)

Shoulder rock dressing to match new asphalt grade will be completed by Skagit County following construction.

Construction Requirements

(4/11/2012) Sk. Co.

Item number four in the first paragraph of Section 2-11.3 is revised to read:

4. Remove and dispose of all weeds, brush, refuse, rocks or asphalt chunks, survey stakes, or any other debris remaining after the work regardless as to origin that lie on the Roadbed, Shoulders, ditches, and slopes.

Division 3
Aggregate Production and Acceptance

Acceptance of Aggregates

(11/28/12) Sk. Co.

Section 3-04 is deleted in its entirety and replaced with the following:

3-04.1 Description

There will be no price adjustments for aggregates. Aggregates having all constituents falling within the specification limits and meeting all other requirements of the standard specifications, these specifications, and the plans shall be accepted at the unit contract price.

3-04.2 Rejected Work

3-04.2(1) General

Work that is defective or does not conform to Contract requirements shall be rejected. Any material rejected by the Contractor or the Agency shall not be paid for and shall be removed and disposed of by the Contractor at no expense to the Contracting Agency.

3-04.2(2) Rejection by Contractor

The Contractor may, prior to sampling, elect to remove any defective material and replace it with new material. Any such new material will be sampled, tested, and evaluated for acceptance.

3-04.2(3) Rejection Without Testing

The Project Engineer may, without sampling, reject any load or stockpile that appears defective. Material rejected before placement shall not be incorporated into the work. Any rejected work shall be removed.

No payment will be made for the rejected material unless the Contractor requests that the rejected material be tested. If the Contractor elects to have the rejected material tested, a representative sample shall be obtained and tested by the Engineer. If the rejected material fails the test, no payment will be made for the rejected material; in addition, the cost of sampling and testing shall be borne by the Contractor. If the rejected material passes the test, the material will be considered accepted and shall be paid for at the unit bid price; in addition, all costs for sampling and testing shall be borne by the Contracting Agency.

Division 5
Surface Treatments and Pavements

Hot Mix Asphalt

Materials

Section 5-04.2 is supplemented with the following:

(January 3, 2011)
ESAL's
The number of ESAL's for the design and acceptance of the HMA shall be *** 1.34 *** million.

Construction Requirements

Section 5-04.3 is supplemented with the following:

(BSP August 23, 2010)
Bridges Classified as Unrestricted for Paving
The following bridge(s), located within the paving limits and specified to be paved in this Project, are classified as unrestricted for paving:

*** Sullivan Slough Bridge on La Conner Whitney Road ***

The above bridge(s) shall conform to all requirements for planing and HMA paving on bridge decks as specified in these Special Provisions, except for the additional requirements specified for bridges classified as restricted for paving.

(BSP April 4, 2011)
General Requirements for Planing and HMA Paving on Bridge Decks

Partial or Full Depth Removal of Existing Surfacing on Bridge Decks
Bridges specified to receive either partial or full depth removal of existing surfacing from their decks prior to receiving HMA overlay shall be planed in accordance with Section 5-04.3(14) as supplemented in these Special Provisions.

Bridge Deck Repair of Exposed Concrete Bridge Deck
Bridge decks of exposed concrete, either by existing condition or by full depth surfacing removal as shown in the Plans, shall receive bridge deck repair in accordance with Section 6-02.3(10)D as supplemented in these Special Provisions.

Placing Membrane Waterproofing on Exposed Concrete Bridge Deck
Bridge decks of exposed concrete, either by existing condition or by full depth surfacing removal as shown in the Plans, shall, after completion of bridge deck repair as specified above, receive a waterproofing membrane in accordance with Section 6-08 as supplemented in these Special Provisions.

Paving Bridge Decks with HMA
Prior to placing HMA on a bridge deck, the Contractor shall clearly establish sawcut alignment points at both ends of the bridge transverse joint seals to be placed at the bridge ends, and at interior contraction joints, as specified. The sawcut

alignment points shall be established in such a manner that they remain functional for use in aligning the sawcut after HMA placement.

Conditioning of Existing Surface

(4/24/13) Sk. Co.

5-04.3(5)A Preparation of Existing Surfaces

Section 5-04.3(5)A is revised as follows:

The third sentence of the first paragraph of Section 5-04.3(5)A is revised to read:

> All pavements, bituminous surfaces, concrete surfaces, and shoulders shall be thoroughly cleaned of dust, soil, plant or organic material, pavement grindings, and other foreign matter.

(******)

5-04.3(5)E Pavement Repair

Section 5-04.3(5)E is revised as follows:

The Contractor shall excavate "2" Depth Planing Repair" areas per section 5-04.3(14), in accordance with the details shown in the Plans and as staked. "2" Depth Planing Repair" widths shall be as shown on the plans or as specified by the Project Engineer. The Contractor shall conduct the "2" Depth Planing Repair" in a manner that will protect the pavement that is to remain. Pavement not designated to be removed that is damaged as a result of the Contractor's operations shall be repaired by the Contractor to the satisfaction of the Project Engineer at no cost to the Contracting Agency. The Contractor shall plane only within one lane at a time unless approved otherwise by the Project Engineer. The Contractor shall not plane more area than can be completely finished during the same shift. The excavated material will become the property of the Contracting Agency and shall be hauled to the designated County site per section 5-04.3(14). The Contractor shall backfill "2" Depth Planing Repair" areas with HMA . Asphalt for tack coat shall be required as specified in Section 5-04.3(5)A. A heavy application of tack coat shall be applied to all surfaces of existing pavement in the "2" Depth Planing Repair" areas. The HMA shall be thoroughly compacted by a mechanical tamper or a roller

"Pavement Repair Excavation" areas shall be four feet in width or as specified by the Project Engineer. The Project Engineer will determine the excavation depth, which may vary up to one foot. Before any excavation, the existing pavement shall be sawcut or shall be removed by a pavement grinder. Excavated materials shall become the property of the Contractor and shall be disposed of in a Contractor-provided site off the Right of Way. The Contractor shall not excavate more area than can be completely finished during the same shift. Placement of Crushed Surfacing Base Course backfill shall be accomplished in lifts not to exceed 0.5 foot compacted depth and shall be placed as shown in the Plans. Placement of HMA

backfill shall be accomplished in lifts not to exceed 0.35 foot compacted depth and shall be placed as shown in the Plans. HMA and Crushed Surfacing Base Course shall be thoroughly compacted by a mechanical tamper or a roller. Asphalt for tack coat shall be required as specified in Section 5-04.3(5)A. A heavy application of tack coat shall be applied to all surfaces of existing pavement in the pavement repair area.

Preparation of Aggregates

Mix Design

Statistical or Nonstatistical Evaluation

Delete this section and replace it with the following:

Nonstatistical Evaluation

(January 16, 2014 APWA GSP)

Mix designs for HMA accepted by Nonstatistical evaluation shall;

- Be submitted to the Project Engineer on WSDOT Form 350-042
- Have the aggregate structure and asphalt binder content determined in accordance with WSDOT Standard Operating Procedure 732 and meet the requirements of Sections 9-03.8(2) and 9-03.8(6).
- Have anti-strip requirements, if any, for the proposed mix design determined in accordance with WSDOT Test Method T 718 or based on historic anti-strip and aggregate source compatibility from WSDOT lab testing. Anti-strip evaluation of HMA mix designs utilized that include RAP will be completed without the inclusion of the RAP.

At or prior to the preconstruction meeting, the contractor shall provide one of the following mix design verification certifications for Contracting Agency review;

- The proposed mix design indicated on a WSDOT mix design/anti-strip report that is within one year of the approval date
- The proposed HMA mix design submittal (Form 350-042) with the seal and certification (stamp & signature) of a valid licensed Washington State Professional Engineer.
- The proposed mix design by a qualified City or County laboratory mix design report that is within one year of the approval date.

The mix design will be performed by a lab accredited by a national authority such as Laboratory Accreditation Bureau, L-A-B for Construction Materials Testing, The Construction Materials Engineering Council (CMEC's) ISO 17025 or AASHTO Accreditation Program (AAP) and shall supply evidence of participation in the AASHTO Material Reference Laboratory (AMRL) program.

At the discretion of the Engineer, agencies may accept mix designs verified beyond the one year verification period with a certification from the Contractor that the materials and sources are the same as those shown on the original mix design.

Mixing

Acceptance Sampling and Testing - HMA Mixture

General

(January 16, 2014 APWA GSP)

Delete Section 5-04.3(8)A1 and replace it with the following:

Acceptance of HMA shall be as defined under nonstatistical or commercial evaluation.

Nonstatistical evaluation will be used for all HMA not designated as Commercial HMA in the contract documents.

The mix design will be the initial JMF for the class of HMA. The Contractor may request a change in the JMF. Any adjustments to the JMF will require the approval of the Project Engineer and must be made in accordance with Section 9-03.8(7).

Commercial evaluation may be used for Commercial HMA and for other classes of HMA in the following applications: sidewalks, road approaches, ditches, slopes, paths, trails, gores, prelevel, and pavement repair. Other nonstructural applications of HMA accepted by commercial evaluation shall be as approved by the Project Engineer. Sampling and testing of HMA accepted by commercial evaluation will be at the option of the Project Engineer. Commercial HMA can be accepted by a contractor certificate of compliance letter stating the material meets the HMA requirements defined in the contract.

Definition of Sampling Lot and Sublot

(1/15/14) Sk. Co.

The last sentence in the first paragraph of Section 5-04.3(8)A4 is revised to read:

For a lot in progress with constituents falling outside the tolerance limits of the job mix formula, a new lot will begin at the Contractor's request after the Project Engineer is satisfied that material conforming to the Specifications can be produced.

Section 5-04.3(8)A4 is supplemented following:

For HMA in a structural application, sampling and testing for total project quantities less than 400 tons is at the discretion of the engineer. For HMA used in a structural application and with a total project quantity less than 800 tons but more than 400 tons, a minimum of one acceptance test shall

be performed. If test results are found to be within specification requirements, additional testing will be at the Engineer's discretion.

Test Results
(7/23/13) Sk. Co.

Section 5-04.3(8)A5 is revised to read:

The results of all acceptance testing performed in the field will be available to the Contractor after completion of the testing.

The Contractor may request a sublot be retested. To request a retest, the Contractor shall submit a written request within 7 calendar days after the specific test results have been given to the Contractor. A split of the original acceptance sample will be sent for testing to a testing agency designated by the Project Engineer. The split of the sample will not be tested with the same equipment or by the same tester that ran the original acceptance test. The sample will be tested for a complete gradation analysis, asphalt binder content, and Va, and the results of the retest will be used for the acceptance of the HMA in place of the original sublot sample test results. The cost of testing will be deducted from any monies due or that may come due the Contractor under the Contract at the rate of $250 per sample.

Test Methods
(5/6/14) Sk. Co.

Section 5-04.3(8)A6 is revised to read:

Testing of HMA for compliance of Va will be at the option of the Contracting Agency. If tested, compliance of Va will be by WSDOT Standard Operating Procedure SOP 731. Testing for compliance of asphalt binder content will be by WSDOT FOP for AASHTO T 308. Testing for compliance of gradation will be by WAQTC FOP for AASHTO T 27/T 11.

Compaction

Control

General

(1/14/14) Sk. Co.

The first two paragraphs of Section 5-04.3(10)B1 are revised to read:

HMA mixture accepted by nonstatistical evaluation that is used in traffic lanes including lanes for ramps, truck climbing, weaving, speed change and shoulders having a specified compacted course thickness greater than 0.10-foot, shall be compacted to a specified level of relative density. The specified level of relative density shall be a minimum of 91 percent of the reference maximum density as determined by WSDOT FOP for

AASHTO T 209. The specified level of density attained will be determined by the non-statistical evaluation of tests taken in accordance with WSDOT FOP for WAQTC TM 8 and WSDOT SOP 729 on the day the mix is placed (after completion of the finish rolling).

A lot is represented by randomly selected samples of the same mix design that will be tested for acceptance, with a maximum of 15 sublots per lot; the final lot for a mix design may be increased to 25 sublots. Sublots will be uniform in size with a maximum of approximately 80 tons per sublot; the final sublot of the day may be increased to 120 tons. The sublot locations within each density lot will be determined by the stratified random sampling procedure conforming to WSDOT Test Method T 716. For a lot in progress with constituents falling outside the tolerance limits of the job mix formula, a new lot will begin at the Contractor's request after the Project Engineer is satisfied that material conforming to the Specifications can be produced.

Test Results
(1/14/14) Sk. Co.

Section 5-04.3(10)B4 is revised to read:

The nuclear moisture-density gauge results of all compaction acceptance testing will be available to the Contractor after completion of the testing. Determination of the relative density of the HMA with a nuclear moisture - density gauge requires a correlation factor determined in accordance with WSDOT SOP 730 and may require resolution after the correlation factor is known. When a core is taken for gauge correlation at the location of a sublot, the relative density of the core will be used for the sublot test result and is exempt from challenge testing. Acceptance of HMA compaction will be based on the non-statistical evaluation so determined.

For a sublot that did not meet the minimum of 91 percent of the reference maximum density in a compaction lot and thus subject to rejection, the Contractor may request that a core be used for determination of the relative density of the sublot. The relative density of the core will replace the relative density determined by the nuclear moisture-density gauge for the sublot and will be used for acceptance of HMA compaction lot. When cores are taken by the Contracting Agency at the request of the Contractor, they shall be requested by noon of the next workday after the test results for the sublot have been provided or made available to the Contractor. The core will be taken at approximately the same location as the nuclear moisture-density gauge test in the compaction sublot being challenged. Traffic control shall be provided by the Contractor as requested by the Project Engineer. Failure by the Contractor to provide the requested traffic control will result in forfeiture of the request for cores. When the compaction for the lot based on the results of the HMA cores does not meet the minimum of 91 percent of the reference maximum density, the cost for the coring will be deducted from any monies due or that may become due the Contractor under the Contract at the rate of $200 per core and the Contractor shall pay for the cost of the traffic control.

Reject Work

Rejection Without Testing
(7/23/13) Sk. Co.

Section 5-04.3(11)C is revised to read:

The Project Engineer may, without sampling, reject any batch, load, or section of Roadway that appears defective. Material rejected before placement shall not be incorporated into the pavement. Any rejected section of Roadway shall be removed.

No payment will be made for the rejected materials or the removal of the materials unless the Contractor requests that the rejected material be tested. If the Contractor elects to have the rejected material tested, a minimum of three representative samples will be obtained and tested. Acceptance of rejected material will be based on conformance with the specifications. If the rejected material fails the testing, no payment will be made for the rejected material; in addition, the cost of sampling and testing shall be borne by the Contractor. If the rejected material passes the testing, the cost of sampling and testing will be borne by the Contracting Agency. If the material is rejected before placement and the rejected material passes the testing, compensation for the rejected material will be at the full bid price. If rejection occurs after placement and the rejected material passes the testing, compensation for the rejected material will be at the full bid price with an addition of 25 percent of the unit Contract price added for the cost of removal and disposal.

General
(7/23/13) Sk. Co.

A Lot in Progress

Section 5-04.3(11)D3 is revised to read:

The Contractor shall shut down operations and shall not resume HMA placement until such time as the Project Engineer is satisfied that material conforming to the Specifications can be produced when the testing of a lot in progress fails and the Contractor is taking no corrective action.

An Entire Lot

Section 5-04.3(11)D4 is revised to read:

An entire lot which fails testing will be rejected.

Surface Smoothness

The second sentence of Section 5-04.3(13) is deleted and replaced with the following:

(March 13, 1995)

The completed surface of the wearing course of all sections of Roadway shall not vary more then 1/8 inch from the lower edge of a 10-foot straightedge placed on the surface parallel to centerline.

Planing Bituminous Pavement

The second paragraph of Section 5-04.3(14) is deleted.

The last paragraph of Section 5-04.3(14) is revised to read:

All planing material derived from the contractor's operations shall be delivered and stockpiled at the following site:

Hermway Heights Gravel Pit:
Entrance is located at the intersection of
Hermway Heights Road and State Route 534
Located near:
20513 SR 534, Mount Vernon, WA

All details of the delivery, including the location within the pit for stockpiling, shall be coordinated with the Engineer at least 5 working days prior to delivery.

Section 5-04.3(14) is supplemented with the following:

(January 5, 2004)
The Contractor shall perform the butt joint planing operations no more than *** Five (5) *** working days ahead of the time the planed area is to be paved with HMA, unless otherwise allowed by the Engineer in writing.

(BSP August 23, 2010)
Partial Depth Removal of Existing Surfacing from Bridge Deck

Requirements for All Bridges Receiving Planing of Existing Surfacing

The Contractor shall use a rotary milling machine to remove the specified layer of HMA surfacing to the limits shown in the Plans, except as noted below for surfacing within 12 inches of bridge expansion joints. The rotary milling machine shall conform to Section 1-07.7 with a maximum operating weight of 35 tons.

The top layer of surfacing within 12 inches of the edge of an existing bridge expansion joint header or steel expansion joint assembly without a header, and surfacing inaccessible to the rotary milling machines, shall be removed by hand or by low impact hand tools as approved by the Engineer. Use of rotary milling machines to remove the top layer of surfacing in these areas will not be allowed. All damage to existing expansion joint headers and expansion joint components due to the Contractor's operations shall be repaired in accordance with Section 1-07.13.

If rotary milling operations contact existing bridge deck steel reinforcing bars at any time, the Contractor shall immediately cease planing operations and notify the Engineer. The Contractor shall reduce the planing depth for that bridge deck by 0.02 feet or as otherwise specified by the Engineer, and shall not resume rotary milling operations until completing the appropriate adjustments to the rotary milling machine and receiving the Engineer's approval to proceed.

All bridge deck concrete, bridge deck waterproofing membrane and bridge deck steel reinforcing bar damage due to the Contractor's surfacing removal operations shall be repaired by the Contractor in accordance with the **Repair of Damage to Bridge Decks due to Surfacing Removal Operations** subsection of these Special Provisions.

After planing, the Contractor shall remove all loose and unsound surfacing not firmly bonded to the bridge deck, as specified by the Engineer, using methods and equipment that does not damage the existing concrete bridge deck, as approved by the Engineer.

Additional Requirements for Bridges Classified as Restricted for Paving

After receiving the final grade paving profile and maximum planing depths from the Engineer, the Contractor shall use a rotary milling machine to remove the specified layer of HMA surfacing to the limits shown in the Plans, in accordance with the following:

1. The rotary milling machine shall have independent grade control to a tolerance of ± 0.02 feet of final grade, and transverse slope control conforming to the tolerance specified in this Section.

2. The rotary milling machine shall have cutting teeth that leave a uniform plane surface at all times. All teeth on the mill head shall be even and maintained during milling to a maximum differential tolerance of 3/8 inch between the shortest and longest tooth, as measured by a straight edge placed the full width of the rotary milling head.

3. All rotary milling machine cutting tips shall remain sharp during milling operations. A tip is considered dull if it is worn close to the lower base of the cutting tip material or if less than 30 percent of the total length of the cutting tip material remains.

4. The depth of surfacing removal at each bridge, as measured to the bottom of the lowest milling groove generated by the rotary milling machine, shall not exceed the maximum planing depth specified for that bridge deck by the Engineer.

Prior to beginning bridge deck surfacing planing operations for each day, and whenever requested by the Engineer, the Contractor shall confirm to the satisfaction of the Engineer that the rotary head cutting teeth are within the specified tolerance for maximum differential tolerance and maximum planing depth.

(BSP April 16, 2012)

Repair of Damage to Bridge Decks due to Surfacing Removal Operations

All bridge deck concrete, bridge pavement seat, bridge deck steel reinforcing bar, and bridge deck waterproofing membrane (when specified to remain), damage due to the Contractor's surfacing removal operations shall be repaired by the Contractor in accordance with Section 1-07.13, except that damaged steel reinforcing bars at depths less than 0.02 feet below the maximum surfacing removal depth as specified by the Engineer shall be considered bridge deck repair in accordance with Section 6-02.3(10)D as supplemented in these Special Provisions.

Damage to existing concrete is defined as an area of concrete removed to depths equal to or greater than 0.02 feet below the maximum depth of surfacing removal specified by the Engineer. A single line of removed concrete, caused by one or more extended teeth on a rotary milling machine milling head shall be measured as one square foot of damage per foot of line removal. The Contractor shall mitigate the damaged concrete by the following method:

Damaged areas of concrete shall be repaired by removing the concrete to a depth 3/4 inches around the top steel reinforcing bar and placing bridge deck repair material approved by the Engineer to the maximum surfacing removal depth specified by the Engineer and parallel to the final grade paving profile.

Damage to existing steel reinforcing bar is defined as mill head contact with bars at surfacing removal depths equal to or greater than 0.02 feet below the maximum depth of surfacing removal specified by the Engineer. Damaged steel reinforcing bar shall be repaired as follows:

1. Damage to epoxy coating, when present on existing steel reinforcing bars, shall be repaired in accordance with Section 6-02.3(24)H.

2. Damage to steel reinforcing bar resulting in a section loss less than 20 percent of the bar with no damage to the surrounding concrete shall be left in place and shall be repaired by removing the concrete to a depth 3/4 inches around the top steel reinforcing bar and placing bridge deck repair material approved by the Engineer to the maximum surfacing removal depth specified by the Engineer and parallel to the final grade paving profile.

3. Damage to steel reinforcing bar resulting in a section loss of 20 percent or more in one location, bars partially or completely removed from the bridge deck, or where there is a lack of bond to the concrete, shall be repaired by removing the adjacent concrete and splicing a new bar of the same size. Concrete shall be removed to provide a 3/4 inch minimum clearance around the bars. The splice bars shall extend a minimum of 40 bar diameters beyond each end of the damage.

Damaged waterproofing membrane is defined as cut or ruptured membrane at surfacing removal depths equal to or greater than 0.02 feet below the maximum depth of surfacing removal specified by the Engineer. Damaged waterproofing membranes shall be repaired by removing the surfacing by hand methods to provide an area at least six inches wider than the rupture in all directions. The ruptured area shall be sealed with an approved primer and membrane with at least six inches of overlap with the existing membrane.

Measurement

(******)

Section 5-04.4 is supplemented with the following:

Two Inch Depth Planing Repair will be measured by the square yard.

Payment

(******)

Section 5-04.5 is revised as follows:

The paragraph following ""Pavement Repair Excavation Incl. Haul", per square yard" that begins with "The unit contract price per square yard for "Pavement Repair Excavation Incl. Haul……." is revised to read:

The unit contract price per square yard for "Pavement Repair Excavation Incl. Haul" shall be full payment for all costs incurred to perform the work described in Section 5-04.3(5)E as revised by these specifications, as shown in the PAVEMENT REPAIR DETAIL, and in all other plans and details, including all costs involved in the furnishing, delivery, placement, and compaction of HMA patch, all costs associated with planing or sawcutting required for Pavement Repair, all costs associated with furnishing, delivery, placement, and compaction of Crushed Surfacing, and all preparation prior to paving.

Section 5-04.5 is supplemented with the following:

The unit Contract price per ton for "HMA Cl. ½ PG 64-22" shall include full compensation for all costs incurred in the paving of driveway approach aprons including excavation, addition of crushed surfacing and any other required grading. It shall also include all costs for preparing existing pavements, bituminous surfaces, and gravel shoulders, including approaches, for paving as described in section 5-04.3(5)A. It shall also include all costs for placement of HMA adjacent to and around preplaced water valve risers.

The unit Contract price per square yard for "2" Depth Planing Repair" shall be full payment for all costs incurred to perform the work as shown in these specifications, as shown in the 2" DEPTH PLANING REPAIR DETAIL and in all other plans and details, including all costs involved in the furnishing, delivery, placement and compaction of HMA; all costs associated with planning, haul and disposal of planed materials; and all costs in surface preparation prior to paving.

Quality Assurance Price Adjustments

Section 5-04.5(1) is deleted in its entirety and replaced with the following:
(5/19/10) Sk. Co.

There will be no price adjustments for HMA. HMA having all constituents falling within the tolerance limits of the job mix formula and meeting the compaction requirements of Section 5-04.3(10) shall be accepted at the unit contract price with no further evaluation.

Division 6
Structures

Waterproofing

Description

Section 6-08.1 is supplemented with the following:

(January 3, 2011)
This work consists of furnishing and placing an approved waterproofing membrane system over a properly prepared concrete bridge deck prior to placing the HMA overlay.

The waterproofing membrane system shall consist of an impermeable sheet membrane that prevents passage of water from the overlay surfacing to the bridge deck substrate. The system shall also include a primer to bond the membrane to the bridge deck substrate, regardless of bridge deck temperature, except for circumstances when the waterproofing membrane system manufacturer specifically prohibits the use of a primer.

Materials

Section 6-08.2 is supplemented with the following:

(January 3, 2011)
Primer for Membrane Waterproofing (Deck Seal)
The membrane waterproofing (deck seal) primer shall be compatible for use with the membrane manufacturer's sheet membrane, and shall be appropriate for bonding the sheet membrane to the bridge deck surface.

Waterproofing Fabric
Section 9-11.2 is supplemented with the following:

(January 2, 2012)
Membrane waterproofing (deck seal) sheet membrane shall conform to ASTM D 6153 Type III, and the following additional material properties:

Property	Specification	Minimum Value
Minimum Tensile Stress (At tear or breaking load for Thin Polymer Sheets)	ASTM D 882	50 pounds per inch
Minimum Grab Tensile Strength (At breaking load for Geotextiles and Fabric)	ASTM D 4632	50 pounds
Minimum Puncture Capacity (For Thin Polymer Sheets, Geotextiles and Fabric)	ASTM E 154	200 pounds

Membrane waterproofing (deck seal) sheet membrane will be accepted based on manufacturers certificate of compliance that the material furnished conforms to these specifications. The Contractor shall submit the manufacturer's certificate of compliance to the Engineer in accordance with Section 1-06.3.

Construction Requirements

Preparation of Surface

Section 6-08.3(2) is supplemented with the following:

(January 3, 2011)
Preparation of Bridge Deck
The entire bridge deck and the sides of the curb and expansion joint headers to the height of the HMA overlay shall be essentially free of all foreign material such as dirt, grease, etc. Prior to applying the primer or sheet membrane, all dust and loose material shall be removed from the bridge deck with compressed air. All surface defects such as spalled areas, cracks, protrusions, holes, sharp edges, ridges, etc., and other imperfections greater than 3/8 inch that will decrease the effectiveness of the membrane by puncturing, stretching, etc., shall be corrected prior to application of the membrane.

Weather and Moisture Limitations
Work shall not be done during wet weather conditions, or when the bridge deck and ambient air temperatures are below 50F. The bridge deck shall be surface-dry at the time of the application of the primer or sheet membrane.

The Engineer may order work to be suspended in accordance with Section 1-08.6 because of the above weather and moisture limitations.

New Concrete Areas
All areas of the bridge deck that have less than 28 day old concrete shall be allowed to cure for a period of time recommended by the membrane manufacturer or as specified by the Engineer before application of the membrane.

Concrete Protection
The Contractor shall use care to protect all concrete surfaces from damage. Any damage to exposed surfaces shall be repaired in accordance with Section 1-07.13.

Application of Waterproofing

Section 6-08.3(3) is supplemented with the following:

(January 3, 2011)
Membrane Waterproofing (Deck Seal)
The primer and membrane waterproofing shall extend from the bridge deck up onto the curb face and expansion joint header face the thickness of the HMA overlay. Special care shall be used at the curb face and expansion joint header face to see that the membrane adheres to the vertical surface.

The Contractor shall not begin application of membrane waterproofing deck seal to the bridge deck until demonstrating, to the satisfaction of the Engineer, that all labor, equipment, and materials necessary to apply the membrane and HMA overlay are either on hand or readily available to complete the work in a timely manner.

The primer shall be applied to the cleaned concrete surfaces at the rate and according to the procedure recommended by the membrane manufacturer. All surfaces to be covered by the membrane shall be thoroughly and uniformly coated with primer. Precautionary measures shall be taken to ensure that pools and thick layers of primer are not left on the deck surface to scum over. Drying time prior to applying the membrane shall normally be as recommended by the manufacturer, however, the membrane shall not be applied until substantially all volatile material has dissipated from the primer.

The prefabricated membrane shall be applied to the primed curb and bridge deck surfaces by either hand methods or mechanical applicators. Membrane application shall begin at the bridge deck low point and continue in a shingled pattern so that any water which accumulates will drain toward the curb and the bridge deck drains (if present) without accumulation against the membrane seams. Each strip shall be overlapped a minimum of six inches or as recommended by the manufacturer. An adhesive or a wide tipped torch to cause tackiness shall be used, if necessary, to assure a good seal of the joints. Hand rollers or other satisfactory pressure apparatus shall be used on the applied membrane to assure firm and uniform contact with the primed concrete surfaces.

Any torn or cut areas, or narrow overlaps, shall be patched using a satisfactory adhesive and by placing sections of the membrane over the defective area in such a manner that the patch extends at least six inches beyond the defect. The patch shall be rolled or firmly pressed onto the surface.

The fabric shall be neatly cut and contoured at all joints as specified by the Engineer.

After the membrane waterproofing application has been completed, the membrane shall be cut with two right angle cuts at all bridge deck drains (if present). The cuts shall be made to the inside diameter of the bridge deck drain outlet, after which the corners of the membrane waterproofing shall be turned down into the drains and laid in a coating of asphalt binder.

The waterproofing membrane will be visually inspected by the Engineer for uniformity of application, tears, punctures, bonding, bubbles, wrinkles and other defects as described in the membrane manufacturer's literature. All such deficiencies shall be repaired as recommended by the membrane manufacturer and approved by the Engineer prior to placement of the HMA overlay.

Protection Course

Section 6-08.3(4) is supplemented with the following:

(January 3, 2011)
General Membrane Protection
The membrane material shall be protected from damage due to the paving operations. The method of membrane protection shall be as recommended by the manufacturer of the membrane system and approved by the Engineer.

No traffic or equipment except that required for the actual waterproofing and paving operations will be permitted to travel or rest on the membrane waterproofing until it is covered by the HMA overlay.

HMA Overlay

The membrane manufacturer's recommendations shall be thoroughly considered in the application of the HMA overlay particularly as to the type of paving machine, laydown temperature of the HMA, protection of membrane while paving, rolling temperature and technique, and other items unique to each membrane. Differences in application procedure shall be resolved by the Engineer and the Engineer's decision shall be final. Vibratory rollers shall not be used on bridge decks.

Measurement

Section 6-08.4 is supplemented with the following:

(March 6, 2000)

Membrane waterproofing will be measured by the square yard of the bridge deck and curb which is satisfactorily sealed and accepted.

Payment

Section 6-08.5 is supplemented with the following:

(August 2, 2004)

"Membrane Waterproofing (Deck Seal)", per square yard.

The unit contract price per square yard for "Membrane Waterproofing (Deck Seal)" shall be full pay for performing the work as specified, including repairing any damaged or defective waterproofing membrane and damaged HMA overlay.

Division 8
Miscellaneous Construction

Erosion Control and Water Pollution Control

Construction Requirements

(******)

Section 8-01.3 is supplemented with the following:

At the Preconstruction Meeting, the Contractor shall submit the temporary erosion and sediment control (TESC) Plan. The TESC Plan shall include the identification of the ESC Lead.

Measurement

(******)

Section 8.01-4 is supplemented with the following:

Erosion Control will be measured by lump sum.

Payment

(******)

Section 8-01.5 is supplemented with the following:

"Erosion Control", per lump sum.

The lump sum Contract price for "Erosion Control" shall be full pay for all costs in providing any and all erosion control work, including preparing, submitting, and updating the TESC Plan; and any other work needed to meet the requirements of the Standard Specifications and the current version of the Stormwater Management Manual for Western Washington.

TEMPORARY PAVEMENT MARKINGS

Measurement

Section 8-23.4 is revised to read:

(11/26/2013) Sk. Co.

Placement of temporary striping and removal of temporary striping will not be measured and shall be incidental to the unit bid price for HMA Cl ½" PG 64-22.

SANITARY REQUIREMENTS

(8/11/06) SK. CO.

Description

This item consists of placing portable sanitary stations on site.

Measurement

There is no measurement item for this requirement.

Payment

There is no unit price for this item. All costs shall be incidental to all bid items throughout the life of the project.

APPENDICES
(July 12, 1999)

The following appendices are attached and made a part of this contract:

APPENDIX A:
Standard Plans

APPENDIX B:
State Prevailing Wage Rates

APPENDIX C:
Construction Contract and Contract Bond – Informational Only

APPENDIX D:
Proposal Forms – Informational Only

APPENDIX E:
Vicinity Map and Plans

(April 6, 2015)
Standard Plans

The State of Washington Standard Plans for Road, Bridge and Municipal Construction M21-01 transmitted under Publications Transmittal No. PT 14-046, effective August 4, 2014 is made a part of this contract.

The Standard Plans are revised as follows:

A-40.20
Plan Title, Bridge Transverse Joint Seals is revised to read: Bridge Paving Joint Seals
Note 3, replace the phrase “sawing out” with “saw cutting”
Add Note 4. For Details 1, 2, 3, and 4 the item “HMA Sawcut and Seal” shall be used for payment. For Details 5 and 6, the item “Paved Panel Joint Seal” shall be used for payment. For Detail 7, the item “Sealing Existing Longitudinal and Transverse Joint” shall be used for payment.
Details 5 and 6, callout “Waterproofing Membrane (Deck Seal)” delete “(Deck Seal)”

A-50.10
Sheet 2 of 2, Plan, with Single Slope Barrier, reference C-14a is revised to C-70.10

A-50.20
Sheet 2 of 2, Plan, with Anchored Barrier, reference C-14a is revised to C-70.10

A-50.30
Sheet 2 of 2, Plan (top), reference C-14a is revised to C-70.10

A-60.10
Sheet 2, Section B, callout, WAS-“New Tie Bar ~ #5 x 30” (IN) Epoxy Coated Reinforcing Bar” is revised to read: “New Tie Bar ~ #5 x 30” (IN)”

B-10.20 and B-10.40
Substitute “step” in lieu of “handhold” on plan

B-15.60
Table, Maximum Knockout Size column, 120” Diam., 42” is revised to read; 96”

B-25.20
Add Note 7. See Standard Specification Section 8-04 for Curb and Gutter requirements

B-55.20
Metal Pipe elevation, title is revised to read; “Metal Pipe and Steel Rib Reinforced Polyethylene Pipe”

B-90.40
Offset & Bend details, add the subtitle, “Plan View” above titles

C-1
Assembly Detail, Steel Post, (post) callout – was - ”W6 x 9 or W6 x 15” is revised to read; “W6 x 8.5 or W6 x 9 or W6 x 15”

C-1a

General Note 1, first sentence, was – "Type 10 post shall be 6x8 timber or W6x9." Is revised to read; "Type 10 post shall be 6 x 8 timber, or W6 x 9 or W6 x 8.5 steel."

C-1b

General Note 3, first sentence, was – "W6x9 steel posts and timber blocks are alternates for 6 x 8 timber posts and blocks." Is revised to read; "W6 x 8.5 or W6 x 9 steel posts and timber blocks are alternates for 6 x 8 timber posts and blocks."

Sheet 2, steel post detail, dimension, was – "1 1/8" for W6x9" is revised to read;"1 1/8" for W6 x 9 or W6 x 8.5"

C-10

General Note 1, first sentence, was – "Length of W8 x 35 and W6 x 9 shall be determined by measurement from top of ground to top of grout pad." Is revised to read; "Length of W8 x 35 and W6 x 8.5 or W6 x 9 shall be determined by measurement from top of ground to top of grout pad."

Sheet 1, Post Base Plate Detail, callout, was – "W6 x 9" is revised to read; "W6 x 8.5 or W6 x 9"

Sheet 1, Box Culvert Guardrail Steel Post Type 2 detail, callout, was – "W6 x 9 Steel Post" is revised to read;" "W6 x 8.5 or W6 x 9 Steel Post"

Sheet 1, Post Anchor Attachment Detail, callout, was – "W6 x 9 ~ See Note 1" is revised to read; "W6 x 8.5 or W6 x 9 ~ See Note 1"

Sheet 1, Detail A, callout, was – "W6 x 9 Steel Post ~ See Note 1" is revised to read; "W6 x 8.5 or W6 x 9 Steel Post ~ See Note 1"

Sheet 2, Box Culvert Guardrail Steel Post Type 1, callout, was – "W6 x 9 x 27.5" Steel Post" is revised to read; "W6 x 8.5 x 27.5" (IN) or W6 x 9 x 27.5" (IN) Steel Post"

Sheet 2, Detail B, callout, was – "W6 x 9 x 27.5" Steel Post" is revised to read; "W6 x 8.5 x 27.5" (IN) or W6 x 9 x 27.5" (IN) Steel Post"

C-16a

Note 1, reference C-28.40 is revised to C-20.10

C-16b

Note 3, reference C-28.40 is revised to C-20.10

C-20.10

Typical Section ~ without Curb & Typical Section ~ with Curb, callout, was – "6 x 8 Timber Post or W6 x 9 Steel Post (See Notes 1 & 5)" is revised to read; "6 x 8 Timber Post, or W6 x 8.5, or W6 x 9 Steel Post (See Notes 1 & 5)"

Wood Block, Plan View, callout, was – "6 x 8 Timber Post or W6 x 9 Steel Post (See Notes 1 & 5)" is revised to read; "6 x 8 Timber Post, or W6 x 8.5 or W6 x 9 Steel Post (See Notes 1 & 5)"

Isometric View, callout, was – "6 x 8 Timber Post or W6 x 9 Steel Post (Typ.)" is revised to read; "6 x 8 Timber Post, or W6 x 8.5 or W6 x 9 Steel Post (Typ)"

Isometric View, callout, was – "W6 x 9 x 6' Long Steel Post (See Notes 1 & 5)" is revised to read; "W6 x 8.5 x 6' (FT) or W6 x 9 x 6' (FT) Long Steel Post (See Notes 1 & 5)"

C-20.40

Plan View, Elevation View and Span with Headwall Detail, callout, was – "6 x 8 Timber Post or W6x9 Steel Post (Typ.) (See Note 3)" is revised to read; "6 x 8 Timber Post, or W6 x 8.5 or W6 x 9 Steel Post (Typ.) (See Note 3)"

C-20.41
Plan View, Box Culvert Post detail and Section A, callout, was – "W6 x 9 Steel Post" is revised to read; "W6 x 8.5 or W6 x 9 Steel Post"

C-20.42
Case 22A-31 (Plan View), callout, was – "6 x 8 Timber Post or W6 x 9 Steel Post (Typ.)" is revised to read; "6 x 8 Timber Post, or W6 x 8.5 or W6 x 9 Steel Post (Typ.)"

C-22.14
Plan, callout, was – "Location of Post (Without Block) ~ W6 x 9 Steel Post Only" is revised to read; "Location of Post (Without Block) ~ W6 x 8.5 or W6 x 9 Steel Post Only"
Elevation, callout, was – "Location of Post (Without Block) ~ W6 x 9 Steel Post Only" is revised to read; "Location of Post (Without Block) ~ W6 x 8.5 or W6 x 9 Steel Post Only"

C-22.16
Plan, 2x callout, was – "W6 x 9 Steel Post Only (without Block)" are revised to read; "W6 x 8.5 or W6 x 9 Steel Post Only (without Block)"
Elevation, callout, was – "Location of Posts without Blocks ~ W6 x 9 Steel Posts Only" is revised to read; "Location of Posts without Blocks ~ W6 x 8.5 or W6 x 9 Steel Posts Only"

C-22.41
Note 4, Third sentence, Was – "A maximum flare rate of 25 : 1 or flatter over the length of the terminal is allowed for the SKT-MGS (TL-3)." Is revised to read; "A maximum flare rate of 25 : 1 or flatter over the length of the terminal is allowed for the SKT-MGS (TL-3), with a maximum offset of 7.4" (in) over 50' (ft)."

Plan View, dimension callout, was – "(SEE NOTE 5)" is revised to read; "(SEE NOTE 4)"

C-25.18
General Note 6, was – "Posts 1 and 2 are 10 x 10 timber posts or W6 x 15 steel posts: 7' – 6" long. Posts 3 through 9 are 6 x 8 timber posts or W6 x 9 steel posts: 6' – 0" long." Is revised to read; "Posts 1 and 2 are 10 x 10 timber posts or W6 x 15 steel posts: 7' – 6" long. Posts 3 through 9 are 6 x 8 timber posts, or W6 x 8.5 or W6 x 9 steel posts: 6' – 0" long."

C-25.20
elevation view, dimension, was – ""W6 x 9 ~ 6' – 0" Long Steel Post with 6 x 12 Block" is revised to read;"W6 x 8.5 or W6 x 9 ~ 6' – 0" Long Steel Post with 6 x 12 Block"

C-25.22
elevation view, dimension, was – ""W6 x 9 ~ 6' – 0" Long Steel Post with 6 x 12 Block" is revised to read;"W6 x 8.5 or W6 x 9 ~ 6' – 0" Long Steel Post with 6 x 12 Block"

C-25.26
elevation view, dimension, was – ""W6 x 9 ~ 6' – 0" Long Steel Post with 6 x 12 Block" is revised to read;"W6 x 8.5 or W6 x 9 ~ 6' – 0" Long Steel Post with 6 x 12 Block"

F-10.12
Section Title, was – "Depressed Curb Section" is revised to read: "Depressed Curb and Gutter Section"

G-20.10

Multiple Sign Post Installation in Ditch Section, dimension “7’ MIN.” is revised to read; “3’ MIN.”, add dimension at third post on the right, add dimension from post and backslope junction vertically to under side of the sign, callout = “7’ MIN.”

G-50.10

Delete – Plan View (bottom center of sheet)

Delete – Mounting Bracket and Steel Strap Detail

Add Note 5, “5. For signs installed back to back on a single post no bracing is required.”

G-60.10

Sheet 4, Screen Detail, callout – “drill and Tap for ¼” diameter Cap Screw – Spacing approx. 9” o.c. ASTM F593, w/S.S. washer Liberally coat the threads with Anti-seize compound (TYP.)” is revised to read: “*Drill and Tap ¼” (IN) Diam. x 1” (IN) Cap Screw with washer ~ space approx.. 9” o.c. ~ Liberally coat threads with Anti-seize compound (TYP.)”

Add Boxed note: * Bolts, Nuts, and washers ~ ASTM F593 or A193 Type 304 or Type 316 Stainless Steel (S.S.)

G-60.20

Side View, callout, “Anchor Rod ~ 1-3/4” Diam. x 4’-4” Threaded 8” Min. Each End; W/ 2 Washers & 4 Heavy Hex Nuts ~ Galvanize Exposed Anchor Rod End for 1’-0” Min.” is revised to read; “Anchor Rod ~ 1-3/4” Diam. x 4’-4” Threaded 8” Min. Each End; W/ 2 Washers & 6 Heavy Hex Nuts ~ Galvanize Exposed Anchor Rod End for 1’-0” Min.”

G-60.30

End View, callout, “Anchor Rod ~ 1-3/4” Diam. x 4’-4” Threaded 8” Min. Each End; W/ 2 Washers & 4 Heavy Hex Nuts ~ Galvanize Exposed Anchor Rod End for 1’-0” Min.” is revised to read; “Anchor Rod ~ 1-3/4” Diam. x 4’-4” Threaded 8” Min. Each End; W/ 2 Washers & 6 Heavy Hex Nuts ~ Galvanize Exposed Anchor Rod End for 1’-0” Min.”

G-70.10

Sheet 4, Screen Detail, callout – “drill and Tap for ¼” diameter Cap Screw – Spacing approx. 9” o.c. ASTM F593, w/S.S. washer Liberally coat the threads with Anti-seize compound (TYP.)” is revised to read: “*Drill and Tap ¼” (IN) Diam. x 1” (IN) Cap Screw with washer ~ space approx.. 9” o.c. ~ Liberally coat threads with Anti-seize compound (TYP.)”

Add Boxed note: * Bolts, Nuts, and washers ~ ASTM F593 or A193 Type 304 or Type 316 Stainless Steel (S.S.)

H-70.20

Sheet 2, Spacing Detail, Mailbox Support Type 1, reference to Standard Plan I-70.10 is revised to H-70.10

J-3b

Sheet 2 of 2, Plan View of Service Cabinet, Boxed Note, “SEE STANDARD PLAN J-6C…” is revised to read: “SEE STANDARD PLAN J-10.10…”

Sheet 2 of 2, Plan View of Service Cabinet Notes, references to Std. Plan J-9a are revised to J-60.05 (3 instances).

Sheet 2 of 2, “Right Side of Service Cabinet” detail, callout, “1 5/8” x 2 7/16” 12 GA. SLOTTED STEEL CHANNEL BRACKETS (3 REQ’D), EMBED 12”MIN. IN FOUNDATION.”
Is revised to read: “1-5/8” x 3-1/4”, 12 GA. BACK TO BACK SLOTTED STEEL CHANNEL BRACKETS (3 REQ’D), EMBED 12” MIN. IN FOUNDATION”

J-10.22
Key Note 4, “Test with (SPDT Snap Action, Positive close 15 Amp – 120/277 volt “T” rated). Is revised to read: “Test Switch (SPDT snap action, positive close 15 amp – 120/277 volt “T” rated).”

J-20.11
Sheet 2, Foundation Detail, Elevation, callout – “Type 1 Signal Pole” is revised to read: “Type PS or Type 1 Signal Pole”
Sheet 2, Foundation Detail, Elevation, add note below Title, “(Type 1 Signal Pole Shown)”

J-22.15
Ramp Meter Signal Standard, elevation, dimension 4’ - 6” is revised to read; 6’-0”

J-28.50
Section D, callout, was – Backup Strip (ref. to key note 3) is revised to read; “Continuous Backup Strip (ref. to key note 3)”
Key Note 3, was – ¼” Thick, or No thinner than pole wall thickness. Tack weld or seal weld to Base plate. Is revised to read; “1/4” Thick, or No thinner than Pole wall thickness. Tack weld in root or continuous seal weld to Base plate or Pole wall.”

J-28.70
Detail C, dimension, 2” MAX. is revised to read: 1” MAX.
Detail D, dimension, 2” MAX. is revised to read: 1” MAX.

J-29.10
Galvanized Welded Wire Mesh detail, callout – “Drill and Tap for ¼” Diam. Cap Screw, 3 Places, @ 9” center, all 4 edges S.S. Screw, ASTM F593 and washer”
Is revised to read;
“*Drill and Tap ¼” (IN) Diam. x 1” (IN) Cap Screw with washer ~ space approx.. 9” o.c. ~ Liberally coat threads with Anti-seize compound (TYP.)”

Add Boxed note: * Bolts, Nuts, and washers ~ ASTM F593 or A193 Type 304 or Type 316 Stainless Steel (S.S.)

J-29.15
Title, “Camera Pole Standard” is revised to read; “Camera Pole Standard Details”

J-29-16
Title, “Camera Pole Standard Details” is revised to read; “Camera Pole Details”

J-60.14
All references to J-16b (6x) are revised to read; J-60.11

J-90.10

Section B, callout, “Hardware Mounting Rack ~ S. S. 1-5/8” Slotted Channel” is revised to read: “Hardware Mounting Rack (Typ.) ~ Type 304 S. S. 1-5/8” Slotted Channel”

J-90.20

Section B, callout, “Hardware Mounting Rack (Typ.) ~ S. S. 1-5/8” Slotted Channel” is revised to read: “Hardware Mounting Rack (Typ.) ~ Type 304 S. S. 1-5/8” Slotted Channel”

K-80.10

Sign Installation (Fill Section), dimension, 6’ TO 12’ MIN. is revised to read: 12’ MIN.
Sign Installation (Sidewalk and Curb Section), dimension, 6’ TO 12’ MIN. is revised to read: 12’ MIN.
Sign Installation (Behind Traffic Barrier Section), Delete dimensions - 6’ TO 12’ MIN. and 6’ MIN.
Sign with Supplemental Plaque Installation (Fill Section), dimension, 6’ TO 12’ MIN. is revised to read: 12’ MIN.
Sign Installation (Ditch Section), dimension, 6’ TO 12’ MIN. is revised to read: 12’ MIN. Delete dimension – 6’ MIN.

K-80.30

In the NARROW BASE, END view, the reference to Std. Plan C-8e is revised to Std. Plan K-80.35

L-20.10

Sheet 1, Type 3 elevation view, callout, was “Knuckled Selvage (Typ.)” located at the top of the fence elevation, is revised to read; “Twisted and Braided (Typ.)”
Sheet 2, Type 3, elevation view, callout, was “End or Corner (Brace) Post” is revised to read; “End or Corner Post”
Sheet 2, Type 4, elevation view, callout, was “End or Corner (Brace) Post” is revised to read; “End or Corner Post”

The following are the Standard Plan numbers applicable at the time this project was advertised. The date shown with each plan number is the publication approval date shown in the lower right-hand corner of that plan. Standard Plans showing different dates shall not be used in this contract.

A-10.10-00.....8/7/07	A-30.35-00.....10/12/07	A-50.20-01…....9/22/09
A-10.20-00.....10/5/07	A-40.00-00.....8/11/09	A-50.30-00…....11/17/08
A-10.30-00.....10/5/07	A-40.10-02.....6/2/11	A-50.40-00…....11/17/08
A-20.10-00.....8/31/07	A-40.15-00.....8/11/09	A-60.10-02.....6/17/14
A-30.10-00.....11/8/07	A-40.20-02.....5/29/13	A-60.20-02.....6/2/11
A-30.15-00.....11/8/07	A-40.50-01.....6/2/11	A-60.30-00.....11/8/07
A-30.30-01.....6/16/11	A-50.10-00…....11/17/08	A-60.40-00.....8/31/07

B-5.20-01.....6/16/11	B-30.50-01.....4/26/12	B-75.20-01.....6/10/08
B-5.40-01.....6/16/11	B-30.70-03.....4/26/12	B-75.50-01.....6/10/08
B-5.60-01.....6/16/11	B-30.80-00.....6/8/06	B-75.60-00.....6/8/06
B-10.20-01.....2/7/12	B-30.90-01.....9/20/07	B-80.20-00.....6/8/06
B-10.40-00.....6/1/06	B-35.20-00.....6/8/06	B-80.40-00.....6/1/06
B-10.60-00.....6/8/06	B-35.40-00.....6/8/06	B-82.20-00.....6/1/06
B-15.20-01.....2/7/12	B-40.20-00.....6/1/06	B-85.10-01.....6/10/08
B-15.40-01.....2/7/12	B-40.40-01.....6/16/10	B-85.20-00.....6/1/06

B-15.60-01.....2/7/12
B-20.20-02.....3/16/12
B-20.40-03.....3/16/12
B-20.60-03.....3/15/12
B-25.20-01.....3/15/12
B-25.60-00.....6/1/06
B-30.10-01.....4/26/12
B-30.20-02.....4/26/12
B-30.30-01.....4/26/12
B-30.40-01.....4/26/12
B-45.20-00.....6/1/06
B-45.40-00.....6/1/06
B-50.20-00.....6/1/06
B-55.20-00.....6/1/06
B-60.20-00.....6/8/06
B-60.40-00.....6/1/06
B-65.20-01.....4/26/12
B-65.40-00.....6/1/06
B-70.20-00.....6/1/06
B-70.60-00.....6/1/06
B-85.30-00.....6/1/06
B-85.40-00.....6/8/06
B-85.50-01.....6/10/08
B-90.10-00.....6/8/06
B-90.20-00.....6/8/06
B-90.30-00.....6/8/06
B-90.40-00.....6/8/06
B-90.50-00.....6/8/06
B-95.20-01.....2/3/09
B-95.40-00.....6/8/06

1

C-1.....6/16/11
C-1a.....10/14/09
C-1b.....6/16/11
C-1c.....5/30/97
C-1d.....10/31/03
C-2.....1/6/00
C-2a.....6/21/06
C-2b.....6/21/06
C-2c.....6/21/06
C-2d.....6/21/06
C-2e.....6/21/06
C-2f.....3/14/97
C-2g.....7/27/01
C-2h.....3/28/97
C-2i.....3/28/97
C-2j.....6/12/98
C-2k.....7/27/01
C-2n.....7/27/01
C-2o.....7/13/01
C-2p.....10/31/03
C-3.....7/2/12
C-3a.....10/4/05
C-3b.....6/27/11
C-3c.....6/27/11
C-4b.....6/8/06
C-4e.....10/23/14
C-4f.....7/2/12
C-6.....5/30/97
C-6a.....10/14/09
C-6c.....1/6/00
C-6d.....5/30/97
C-6f.....7/25/97
C-7.....6/16/11
C-7a.....6/16/11
C-8.....2/10/09
C-8a.....7/25/97
C-8b.....6/27/11
C-8e.....2/21/07
C-8f.....6/30/04
C-10.....6/3/10
C-16a.....6/3/10
C-16b.....6/3/10
C-20.10-02.....6/11/14
C-20.14-03.....6/11/14
C-20.15-02.....6/11/14
C-20.18-02.....6/11/14
C-20.19-02.....6/11/14
C-20.40-04.....6/11/14
C-20.41-00.....6/30/14
C-20.42-04.....6/11/14
C-20.45.01.....7/2/12
C-22.14-03.....6/11/14
C-22.16-04.....6/11/14
C-22.40-04.....10/23/14
C-22.41-01.....10/23/14
C-22.45-01.....10/23/14
C-23.60-03.....6/11/14
C.24.10-01.....6/11/14
C-25.18-04.....6/11/14
C-25.20-05.....7/2/12
C-25.22-04.....7/2/12
C-25.26-02.....7/2/12
C-25.80-03.....6/11/14
C-40.14-02.....7/2/12
C-40.16-02.....7/2/12
C-40.18-02.....7/2/12
C-70.10-01.....6/17/14
C-75.10-01.....6/11/14
C-75.20-01.....6/11/14
C-75.30-01.....6/11/14
C-80.10-01.....6/11/14
C-80.20-01.....6/11/14
C-80.30-01.....6/11/14
C-80.40-01.....6/11/14
C-80.50-00.....4/8/12
C-85.10-00.....4/8/12
C-85.11-00.....4/8/12
C-85.14-01.....6/11/14
C-85.15-01.....6/30/14
C-85.16-01.....6/17/14
C-85-18-01.....6/11/14
C-85.20-01.....6/11/14
C-90.10-00.....7/3/08

2

D-2.04-00.....11/10/05
D-2.06-01.....1/6/09
D-2.08-00.....11/10/05
D-2.14-00.....11/10/05
D-2.16-00.....11/10/05
D-2.18-00.....11/10/05
D-2.20-00.....11/10/05
D-2.32-00.....11/10/05
D-2.34-01.....1/6/09
D-2.48-00.....11/10/05
D-2.64-01.....1/6/09
D-2.66-00.....11/10/05
D-2.68-00.....11/10/05
D-2.80-00.....11/10/05
D-2.82-00.....11/10/05
D-2.84-00.....11/10/05
D-2.86-00.....11/10/05
D-2.88-00.....11/10/05
D-3.17-01……5/17/12
D-4.....12/11/98
D-6.....6/19/98
D-10.10-01.....12/2/08
D-10.15-01.....12/2/08
D-10.20-00.....7/8/08
D-10.25-00.....7/8/08
D-10.30-00.....7/8/08
D-10.35-00.....7/8/08

D-2.36-03.....6/11/14
D-2.42-00.....11/10/05
D-2.44-00.....11/10/05
D-2.60-00.....11/10/05
D-2.62-00.....11/10/05
D-2.46-01.....6/11/14
D-2.92-00.....11/10/05
D-3.09-00.....5/17/12
D-3.10-01……5/29/13
D-3.11-03……6/11/14
D-3.15-02……6/10/13
D-3.16-02……5/29/13
D-10.40-01.....12/2/08
D-10.45-01.....12/2/08
D-15.10-01.....12/2/08
D-15.20-02.....6/2/11
D-15.30-01.....12/02/08

1

E-1.....2/21/07
E-2.....5/29/98
E-4.....8/27/03
E-4a.....8/27/03

2

F-10.12-03.....6/11/14
F-10.16-00.....12/20/06
F-10.18-00.....6/27/11
F-10.40-02.....6/21/12
F-10.42-00.....1/23/07
F-10.62-02.....4/22/14
F-10.64-03.....4/22/14
F-30.10-03.....6/11/14
F-40.12-02.....6/20/13
F-40.14-02.....6/20/13
F-40.15-02.....6/20/13
F-40.16-02.....6/20/13
F-45.10-01.....6/21/12
F-80.10-03.....6/11/14

3

G-10.10-00.....9/20/07
G-20.10-01.....6/11/14
G-22.10-02.....6/17/14
G-24.10-00.....11/8/07
G-24.20-01.....2/7/12
G-24.30-01.....2/7/12
G-24.40-04.....6/17/14
G-24.50-03.....6/17/14
G-24.60-03.....6/17/14
G-25.10-04.....6/10/13
G-30.10-03.....6/17/14
G-50.10-01.....6/20/13
G-60.10-02.....6/10/13
G-60.20-01.....6/27/11
G-60.30-01.....6/27/11
G-70.10-02.....6/10/13
G-70.20-02....6/10/13
G-70.30-02.....6/10/13
G-90.10-01…..5/11/11
G-90.20-02…..3/22/13
G-90.30-02…..3/22/13
G-90.40-01…..10/14/09
G-95.10-01.....6/2/11
G-95.20-02.....6/2/11
G-95.30-02.....6/2/11

4

H-10.10-00.....7/3/08
H-10.15-00.....7/3/08
H-30.10-00.....10/12/07
H-32.10-00.....9/20/07
H-60.10-01.....7/3/08
H-60.20-01.....7/3/08
H-70.10-01.....2/7/12
H-70.20-01.....2/16/12
H-70.30-02.....2/7/12

5

I-10.10-01.....8/11/09
I-30.10-02.....3/22/13
I-30.15-02.....3/22/13
I-30.16-00.....3/22/13
I-30.17-00.....3/22/13
I-30.20-00.....9/20/07
I-30.30-01.....6/10/13
I-30.40-01.....6/10/13
I-30.60-00.....5/29/13
I-40.10-00.....9/20/07
I-40.20-00.....9/20/07
I-50.20-01.....6/10/13
I-60.10-01.....6/10/13
I-60.20-01.....6/10/13
I-80.10-01.....8/11/09

6

J-3.....8/1/97
J-3b.....3/4/05
J-3c.....6/24/02
J-10.....7/18/97
J-10.10-02……6/11/14
J-10.15-01.....6/11/14
J-10.22-00.....5/29/13
J-15.10-01.....6/11/14
J-15.15-01……6/11/14
J-20.10-03.....6/30/14
J-20.11-02.....6/30/14
J-20.15-03.....6/30/14
J-26.15-01…..5/17/12
J-26.20-00…..6/11/14
J-27.10-00…..3/15/12
J-27.15-00…..3/15/12
J-28.10-01.....5/11/11
J-28.22-00.....8/07/07
J-28.24-00.....8/07/07
J-28.26-01.....12/02/08
J-28.30-03.....6/11/14
J-28.40-02.....6/11/14
J-28.42-01.....6/11/14
J-28.43-00.....6/11/14
J-28.45-02.....6/11/14
J-28.50-02.....6/2/11
J-28.60-01.....6/2/11
J-28.70-01.....5/11/11
J-40.40-00……5/20/13
J-50.10-00…….6/3/11
J-50.11-00…….6/3/11
J-50.12-00…….6/3/11
J-50.15-00…….6/3/11
J-50.16-01…….3/22/13
J-50.20-00…….6/3/11
J-50.25-00…….6/3/11
J-50.30-00…….6/3/11
J-60.05-00…….6/16/11
J-60.11-00…....5/20/13
J-60.12-00…....5/20/13
J-60.13-00…....6/16/10
J-60.14-00……6/16/10

J-20.16-02.....6/30/14
J-20.20-02.....5/20/13
J-20.26-01.....7/12/12
J-21.10-04.....6/30/14

J-21.15-01.....6/10/13
J-21.16-01.....6/10/13
J-21.17-01.....6/10/13
J-21.20-01.....6/10/13
J-22.15-01.....6/10/13
J-22.16-02.....6/10/13
J-26.10-02…..3/15/12

J-29.10-00.....6/27/11
J-29.15-00.....6/27/11
J-29.16-01.....6/20/13
J-40.10-03……5/20/13

J-40.20-02……6/11/14
J-40.30-03……5/20/13
J-40.35-01……5/29/13
J-40.36-01……5/20/13
J-40.37-01……5/20/13
J-40.38-01.....5/20/13
J-40.39-00……5/20/13

J-75.10-01……5/11/11
J-75.20-00……2/10/09
J-75.30-01…….5/11/11
J-75.40-01……6/11/14
J-75.41-00……6/11/14
J-75.45-01……6/11/14
J-90.10-01…….6/27/11
J-90.20-01…….6/27/11
J-90.21-00……6/30/14

1

K-70.20-00.....2/15/07
K-80.10-00.....2/21/07
K-80.20-00.....12/20/06
K-80.30-00.....2/21/07
K-80.35-00.....2/21/07
K-80.37-00.....2/21/07

2

L-10.10-02.....6/21/12
L-20.10-02.....6/21/12
L-30.10-02.....6/11/14

L-40.10-02.....6/21/12
L-40.15-01.....6/16/11
L-40.20-02.....6/21/12

L-70.10-01.....5/21/08
L-70.20-01.....5/21/08

3

M-1.20-03.....6/24/14
M-1.40-02.....6/3/11
M-1.60-02.....6/3/11
M-1.80-03.....6/3/11
M-2.20-02.....6/3/11
M-3.10-03.....6/3/11
M-3.20-02.....6/3/11
M-3.30-03.....6/3/11
M-3.40-03.....6/3/11
M-3.50-02.....6/3/11
M-5.10-02.....6/3/11
M-7.50-01.....1/30/07
M-9.50-02.....6/24/14

M-9.60-00…….2/10/09
M-11.10-01.....1/30/07
M-15.10-01.....2/6/07
M-17.10-02.....7/3/08
M-20.10-02.....6/3/11
M-20.20-01.....1/30/07
M-20.30-02.....10/14/09
M-20.40-03.....6/24/14
M-20.50-02.....6/3/11
M-24.20-01.....5/31/06
M-24.40-01.....5/31/06
M-24.50-00.....6/16/11
M-24.60-04.....6/24/14

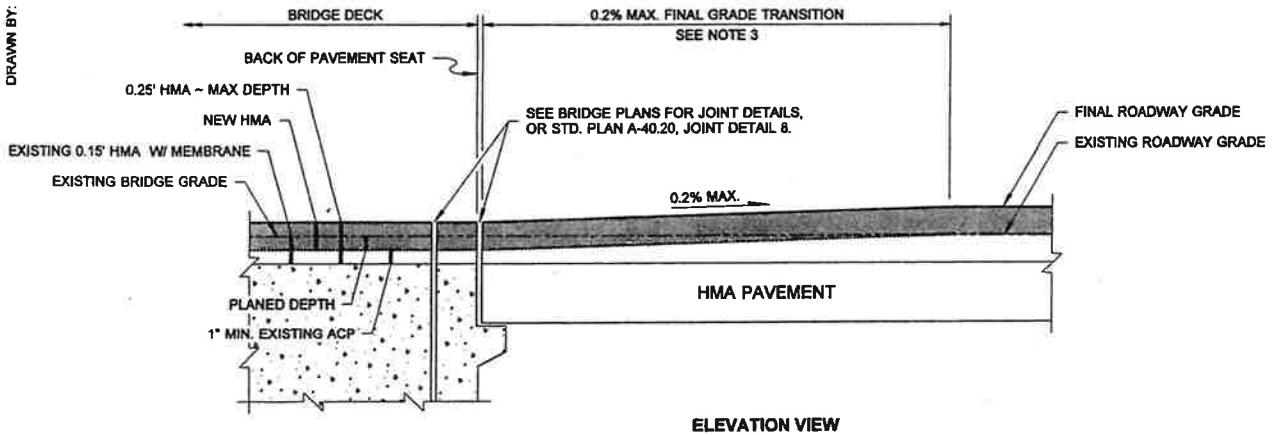
M-40.10-03.....6/24/14
M-40.20-00...10/12/07
M-40.30-00.....9/20/07
M-40.40-00.....9/20/07
M-40.50-00.....9/20/07
M-40.60-00.....9/20/07
M-60.10-01.....6/3/11
M-60.20-02.....6/27/11
M-65.10-02.....5/11/11
M-80.10-01.....6/3/11
M-80.20-00.....6/10/08
M-80.30-00.....6/10/08

4
5

APPENDIX A

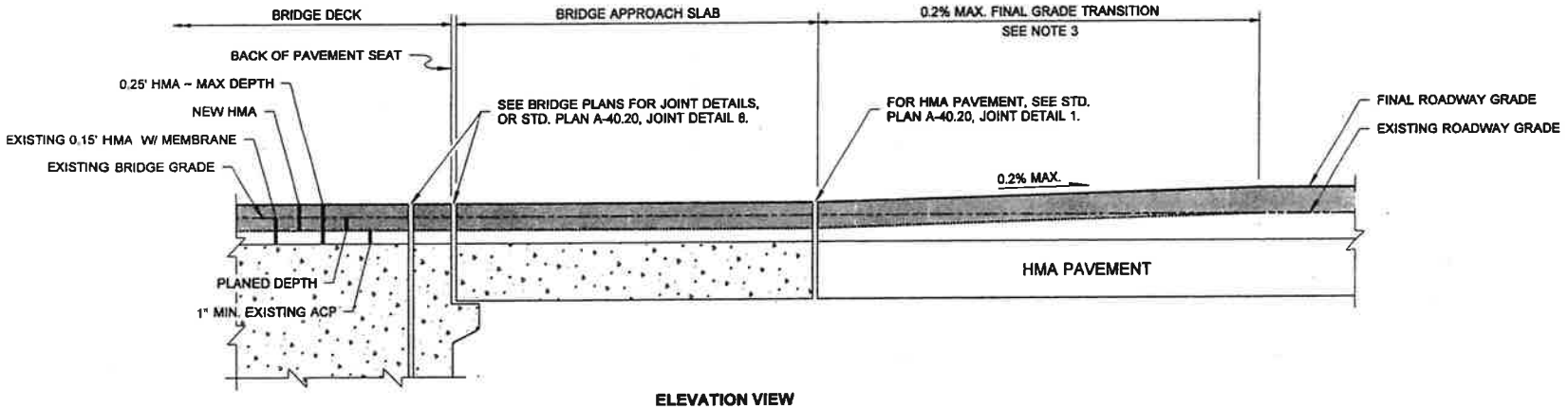
Standard Plans

DRAWN BY: MARK SUJKA



ELEVATION VIEW

HMA OVERLAY WITHOUT BRIDGE APPROACH SLAB



ELEVATION VIEW

HMA OVERLAY WITH BRIDGE APPROACH SLAB

NOTES

1. A typical bridge overlay will remove 0.07 feet of Asphaltic Concrete Pavement (ACP and place 0.15 feet of new Hot Mixed Asphalt (HMA). Depth of removal and placement will vary for each bridge. Total depth of HMA on the bridge shall not exceed 0.25 feet, unless shown otherwise in the plans.
2. If the existing depth of asphalt on the bridge deck is 0.25 feet, then the overlay will remove 0.15 feet of ACP and place 0.15 feet of new HMA. The existing bridge grad will remain unchanged.
3. FINAL GRADE TRANSITION: The maximum longitudinal taper slope to transition a increase in roadway grade to the new or existing bridge grade will be at most 1 inch rise to 40 feet run (1V:480H or flatter) (0.2% maximum). If several overlays are present, extended taper lengths shall be required to maintain the transition slope (1V:480H or flatter) (0.2% maximum).
4. If the ACP and membrane is to be removed from the bridge deck, see GSP 023106 for deck preparation before placing new membrane.
5. In accordance with Standard Specification 5-05.3(12), when pavement abuts bridges the finished pavement parallel to centerline shall be uniform to a degree that no variations greater than 1/8-inch are present when tested with a 10-foot straightedge.

BIJAN KHALEGHI
STATE OF WASHINGTON
27695
REGISTERED PROFESSIONAL ENGINEER

EXPIRES AUGUST 23, 2008

BRIDGE DECK TRANSITION FOR HMA OVERLAY

STANDARD PLAN A-60.30-0

SHEET 1 OF 1 SHEET

APPROVED FOR PUBLICATION

STATE DESIGN ENGINEER

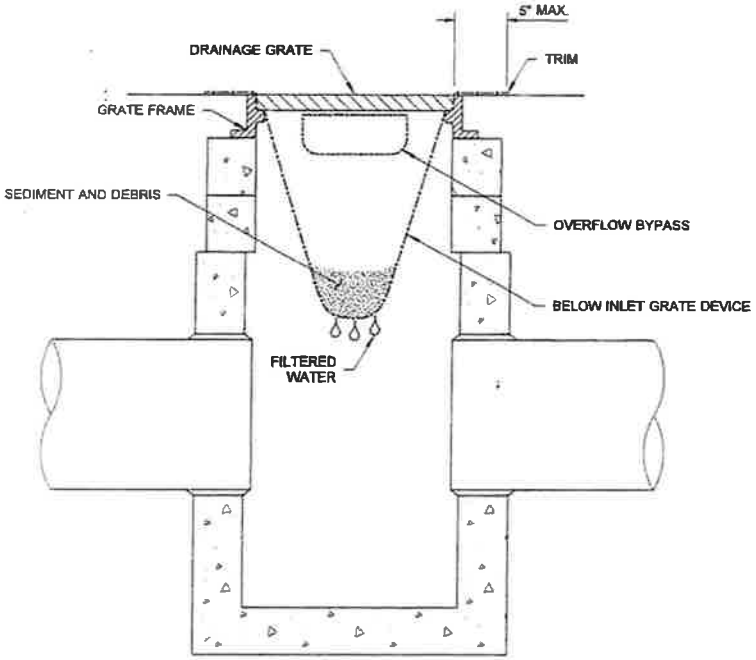
DATE

Washington State Department of Transportation

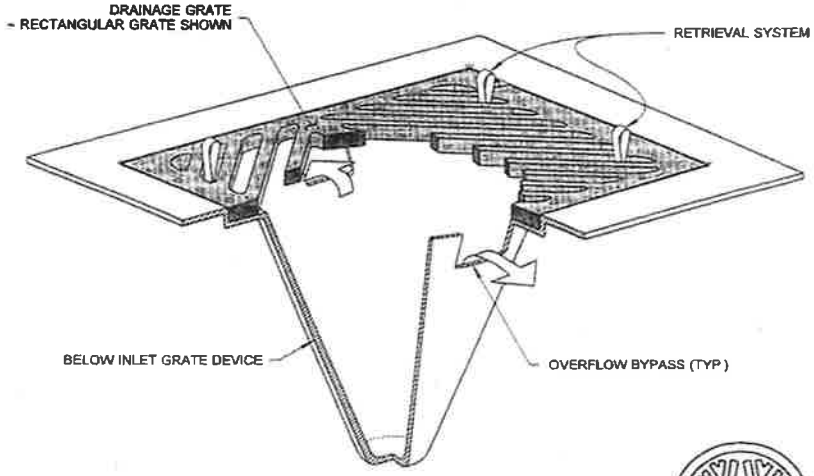
DRAWN BY: LISA CYFORD

NOTES

1. Size the Below Inlet Grate Device (BIGD) for the storm water structure it will service.
2. The BIGD shall have a built-in high-flow relief system (overflow bypass).
3. The retrieval system must allow removal of the BIGD without spilling the collected material.
4. Perform maintenance in accordance with Standard Specification 8-01.3(1



SECTION VIEW
NOT TO SCALE



ISOMETRIC VIEW

STATE OF WASHINGTON
REGISTERED
LANDSCAPE ARCHITECT
MARK W. MADIREE
CERTIFICATE NO. 0005
9/20/07

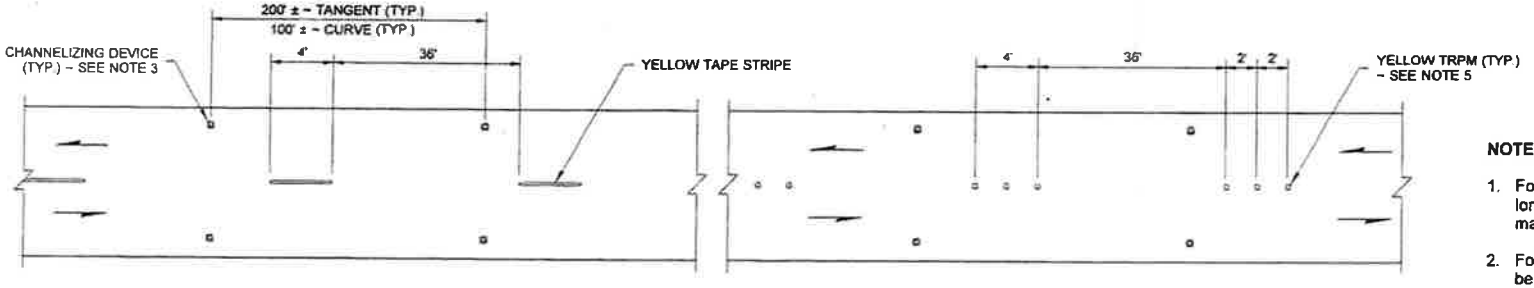
STORM DRAIN INLET PROTECTION

STANDARD PLAN I-40.20-

SHEET 1 OF 1 SHEET

APPROVED FOR PUBLICATION
STATE DESIGN ENGINEER
DATE
Washington State Department of Transportation

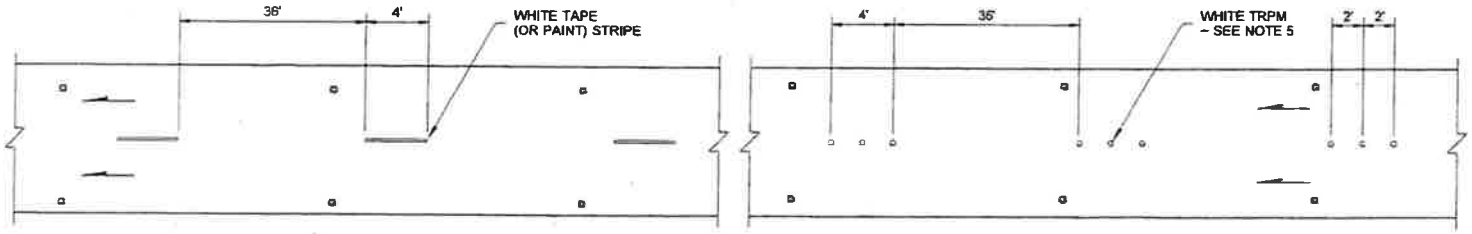
DRAWN BY: ELENA BRUNSTEIN



HOT MIXED ASPHALT PAVEMENT

BITUMINOUS SURFACE TREATMENT

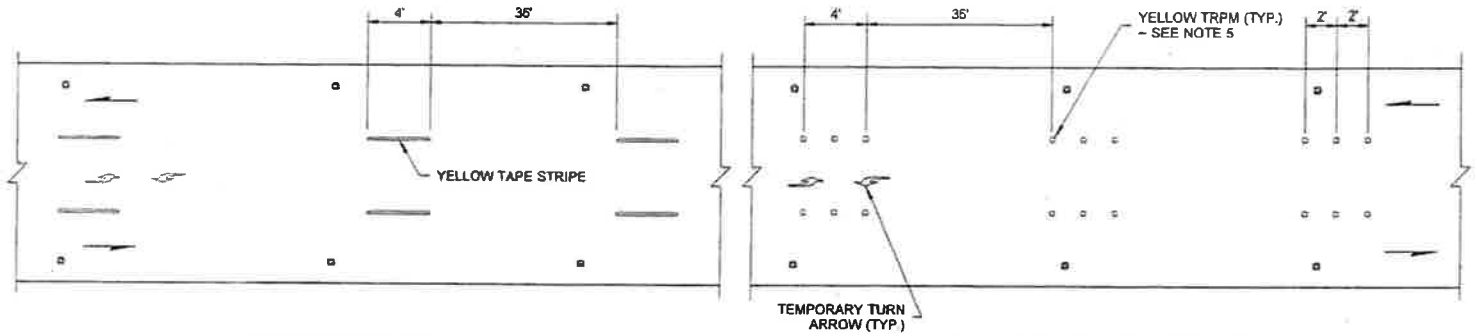
TWO-LANE ROADWAY



HOT MIXED ASPHALT PAVEMENT

BITUMINOUS SURFACE TREATMENT

ONE-WAY TWO-LANE ROADWAY



HOT MIXED ASPHALT PAVEMENT

BITUMINOUS SURFACE TREATMENT

TWO-WAY TWO-LANE LEFT TURN ROADWAY

NOTES

1. For long term projects conflicting pavement markings that are no longer applicable shall be removed or obliterated. Temporary markings shall be used as necessary.
2. For Hot Mixed Asphalt Pavement, a temporary striping tape shall be installed in conjunction with DO NOT PASS and "PASS WITH CARE" sign locations.
3. Temporary roadside delineation with Channelization Devices is optional. The appropriate taper length shall be L/2. See Standard Plan K-24.20 for minimum taper length (L).
4. For long term projects a channelization/pavement marking plan should be implemented.
5. Temporary Raised Pavement Marker (TRPM) may be used on a pattern spacing 5' O.C. to simulate a solid line.

EXPIRES AUGUST 9, 2007

TEMPORARY CHANNELIZATION

STANDARD PLAN K-70.20-00

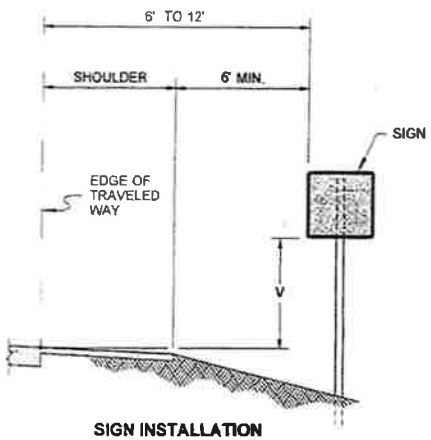
SHEET 1 OF 1 SHEET

APPROVED FOR PUBLICATION

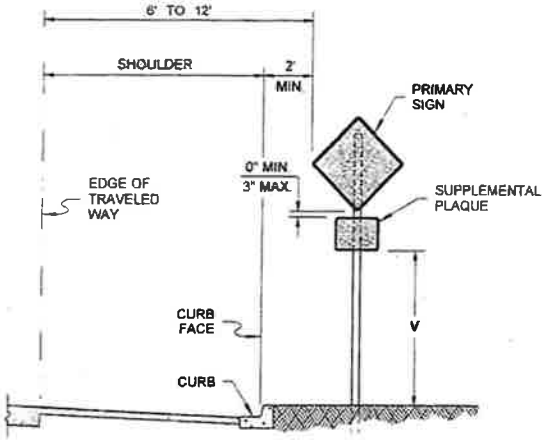
STATE DESIGN ENGINEER — DATE 04/15/0

Washington State Department of Transportation

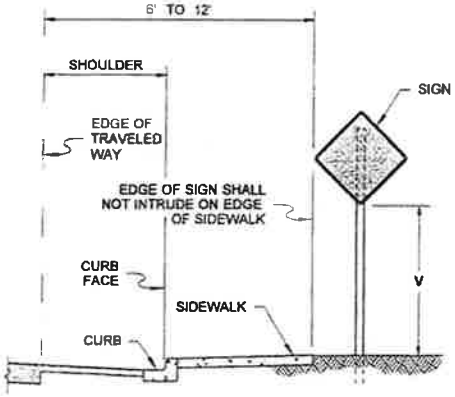
DRAWN BY: FERN LIDDELL



SIGN INSTALLATION (FILL SECTION)



SIGN INSTALLATION (CURB SECTION)

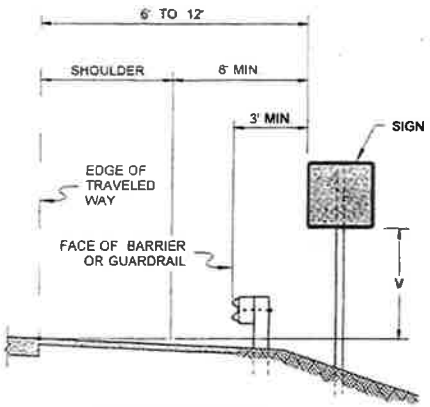


SIGN INSTALLATION (SIDEWALK AND CURB SECTION)

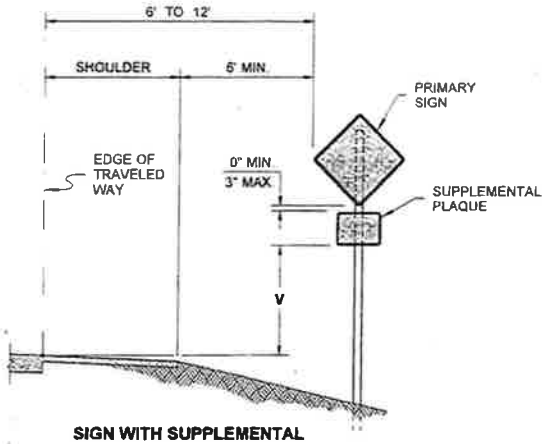
NOTES

1. For sign installation details, see Std. Plan G - seri
2. In rural areas, the "V" Height can be a minimum of 7 feet for primary signs and 6 feet for the supplemental plaques for greater visibility, as directed by the engineer.
3. The "V" height for signs, with an area of more tha 50 square feet and two or more sign supports, is 7 feet in both rural and urban areas.

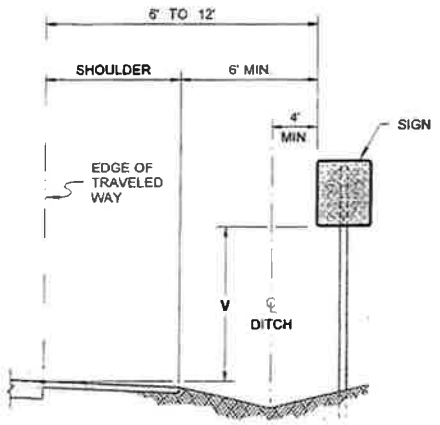
	HEIGHT V	
	TO BOTTOM OF SIGN (NO SUPPLEMENTAL PLAQUE)	TO BOTTOM OF SUPPLEMENTAL PLAQU (WHEN REQUIRED)
RURAL	5' MINIMUM	4' MINIMUM
URBAN	7' MINIMUM	6' MINIMUM



SIGN INSTALLATION (BEHIND TRAFFIC BARRIER)



SIGN WITH SUPPLEMENTAL PLAQUE INSTALLATION (FILL SECTION)



SIGN INSTALLATION (DITCH SECTION)

EXPIRES AUGUST 9, 2007

CLASS A CONSTRUCTION SIGNING INSTALLATION STANDARD PLAN K-80.10-(

SHEET 1 OF 1 SHEET

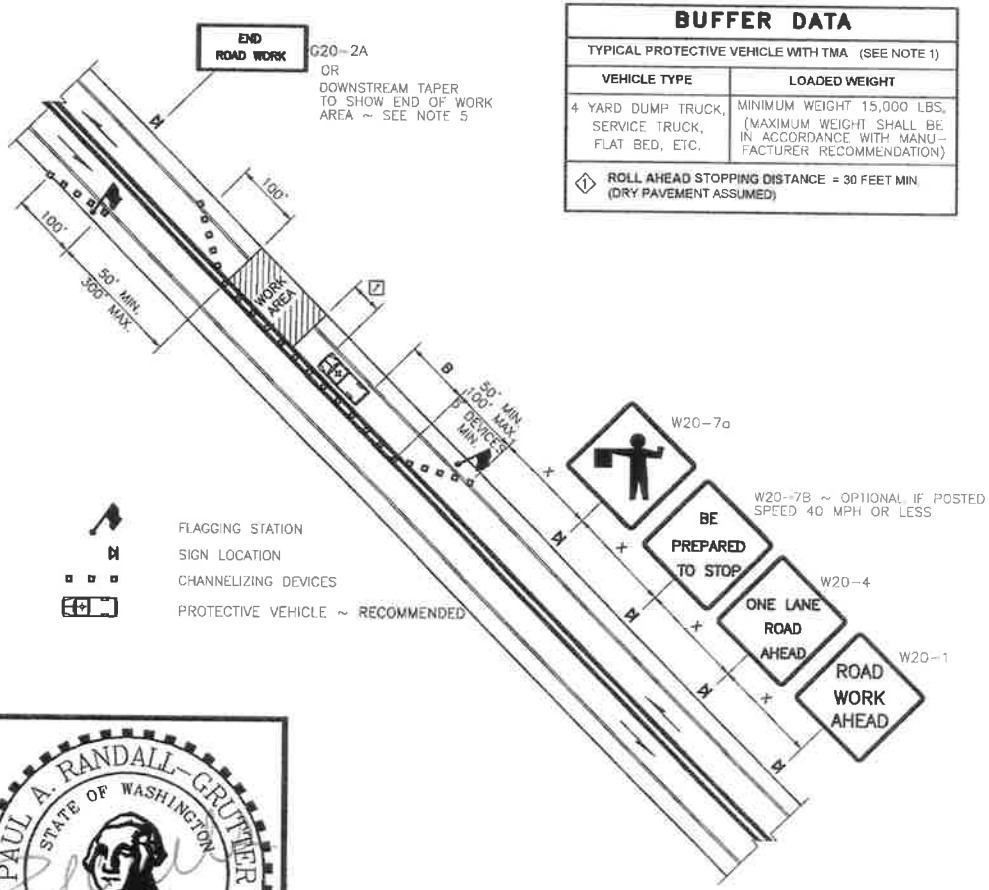
APPROVED FOR PUBLICATION

STATE DESIGN ENGINEER — DATE 07/24

Washington State Department of Transportation

LONGITUDINAL BUFFER SPACE=B									
POSTED SPEED (MPH)	25	30	35	40	45	50	55	60	65
LENGTH B (FEET)	155	200	250	305	360	425	495	570	645

BUFFER DATA	
TYPICAL PROTECTIVE VEHICLE WITH TMA (SEE NOTE 1)	
VEHICLE TYPE	LOADED WEIGHT
4 YARD DUMP TRUCK, SERVICE TRUCK, FLAT BED, ETC.	MINIMUM WEIGHT 15,000 LBS. (MAXIMUM WEIGHT SHALL BE IN ACCORDANCE WITH MANUFACTURER RECOMMENDATION)
① ROLL AHEAD STOPPING DISTANCE = 30 FEET MIN. (DRY PAVEMENT ASSUMED)	



NOTES:

1. A PROTECTIVE VEHICLE IS RECOMMENDED REGARDLESS IF A TRUCK MOUNTED ATTENUATOR (TMA) IS AVAILABLE; A WORK VEHICLE MAY BE USED. WHEN NO TMA IS USED, THE PROTECTIVE VEHICLE SHALL BE STRATEGICALLY LOCATED TO SHIELD WORKERS, WITH NO SPECIFIC ROLL-AHEAD DISTANCE.
2. NIGHT WORK REQUIRES ADDITIONAL ROADWAY LIGHTING AT FLAGGING STATIONS. SEE WSDOT STANDARD SPECIFICATIONS FOR ADDITIONAL DETAILS.
3. EXTEND CHANNELIZING DEVICE TAPER ACROSS SHOULDER ~ RECOMMENDED.
4. SIGN SEQUENCE IS THE SAME FOR BOTH DIRECTIONS OF TRAVEL ON THE ROADWAY.
5. CHANNELIZING DEVICE SPACING FOR THE DOWNSTREAM TAPER OPTION SHALL BE 20' O.C.
6. FOR SIGNS SIZE REFER TO MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) AND WSDOT SIGN FABRICATION MANUAL M55-05.

SIGN SPACING = X (1)		
RURAL HIGHWAYS	60/65 MPH	800'±
RURAL ROADS	45/55 MPH	500'±
RURAL ROADS & URBAN ARTERIALS	35/40 MPH	350'±
RURAL ROADS, URBAN ARTERIALS, RESIDENTIAL & BUSINESS DISTRICTS	25/30 MPH	200'± (2)
URBAN STREETS	25 MPH OR LESS	100'± (2)
ALL SIGNS ARE BLACK ON ORANGE UNLESS DESIGNATED OTHERWISE		

(1) ALL SIGN SPACING MAY BE ADJUSTED TO ACCOMMODATE INTERCHANGE RAMPS, AT-GRADE INTERSECTIONS, AND DRIVEWAYS.

(2) THIS SIGN SPACING MAY BE REDUCED IN URBAN AREAS TO FIT ROADWAY CONDITIONS.

PAUL A. RANDALL-GRUTTER
STATE OF WASHINGTON
40264
REGISTERED
PROFESSIONAL ENGINEER
9.19.11

LANE CLOSURE WITH FLAGGER CONTROL

STANDARD PLAN

SKAGIT COUNTY PUBLIC WORKS
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(360) 336-9400 FAX (360) 336 9478

DATE: 09/19/2011

DRAWN BY: GES

APPENDIX B

Washington State Prevailing Wage Rates

State of Washington
Department of Labor & Industries

Prevailing Wage Section - Telephone 360-902-5335
PO Box 44540, Olympia, WA 98504-4540

Washington State Prevailing Wage

The PREVAILING WAGES listed here include both the hourly wage rate and the hourly rate of fringe benefits. On public works projects, worker's wage and benefit rates must add to not less than this total. A brief description of overtime calculation requirements are provided on the Benefit Code Key.

Journey Level Prevailing Wage Rates for the Effective Date: 5/19/2015

County	Trade	Job Classification	Wage	Holiday	Overtime	Note
Skagit	Asbestos Abatement Workers	Journey Level	$42.67	5D	1H	
Skagit	Boilermakers	Journey Level	$64.29	5N	1C	
Skagit	Brick Mason	Brick And Block Finisher	$44.46	5A	1M	
Skagit	Brick Mason	Journey Level	$51.32	5A	1M	
Skagit	Brick Mason	Pointer-Caulker-Cleaner	$51.32	5A	1M	
Skagit	Building Service Employees	Janitor	$10.00		1	
Skagit	Building Service Employees	Shampooer	$9.47		1	
Skagit	Building Service Employees	Waxer	$9.47		1	
Skagit	Building Service Employees	Window Cleaner	$9.47		1	
Skagit	Cabinet Makers (In Shop)	Journey Level	$18.85		1	
Skagit	Carpenters	Acoustical Worker	$52.32	5D	4C	
Skagit	Carpenters	Bridge, Dock And Wharf Carpenters	$52.32	5D	4C	
Skagit	Carpenters	Carpenter	$52.32	5D	4C	
Skagit	Carpenters	Carpenters on Stationary Tools	$52.45	5D	4C	
Skagit	Carpenters	Creosoted Material	$52.42	5D	4C	
Skagit	Carpenters	Floor Finisher	$52.32	5D	4C	
Skagit	Carpenters	Floor Layer	$52.32	5D	4C	
Skagit	Carpenters	Scaffold Erector	$52.32	5D	4C	
Skagit	Cement Masons	Journey Level	$52.38	7A	1M	
Skagit	Divers & Tenders	Diver	$105.37	5D	4C	8A
Skagit	Divers & Tenders	Diver On Standby	$59.50	5D	4C	
Skagit	Divers & Tenders	Diver Tender	$54.82	5D	4C	
Skagit	Divers & Tenders	Surface Rcv & Rov Operator	$54.82	5D	4C	
Skagit	Divers & Tenders	Surface Rcv & Rov Operator Tender	$51.07	5A	4C	
Skagit	Dredge Workers	Assistant Engineer	$54.75	5D	3F	
Skagit	Dredge Workers	Assistant Mate (Deckhand)	$54.33	5D	3F	
Skagit	Dredge Workers	Boatmen	$54.75	5D	3F	
Skagit	Dredge Workers	Engineer Welder	$55.79	5D	3F	

Skagit	Dredge Workers	Leverman, Hydraulic	$56.92	5D	3F	
Skagit	Dredge Workers	Mates	$54.75	5D	3F	
Skagit	Dredge Workers	Oiler	$54.33	5D	3F	
Skagit	Drywall Applicator	Journey Level	$52.32	5D	1H	
Skagit	Drywall Tapers	Journey Level	$52.37	5P	1E	
Skagit	Electrical Fixture Maintenance Workers	Journey Level	$21.48		1	
Skagit	Electricians - Inside	Cable Splicer	$62.37	7H	1E	
Skagit	Electricians - Inside	Construction Stock Person	$30.95	7H	1D	
Skagit	Electricians - Inside	Journey Level	$58.23	7H	1E	
Skagit	Electricians - Motor Shop	Craftsman	$15.37		1	
Skagit	Electricians - Motor Shop	Journey Level	$14.69		1	
Skagit	Electricians - Powerline Construction	Cable Splicer	$69.95	5A	4D	
Skagit	Electricians - Powerline Construction	Certified Line Welder	$63.97	5A	4D	
Skagit	Electricians - Powerline Construction	Groundperson	$43.62	5A	4D	
Skagit	Electricians - Powerline Construction	Heavy Line Equipment Operator	$63.97	5A	4D	
Skagit	Electricians - Powerline Construction	Journey Level Lineperson	$63.97	5A	4D	
Skagit	Electricians - Powerline Construction	Line Equipment Operator	$53.81	5A	4D	
Skagit	Electricians - Powerline Construction	Pole Sprayer	$63.97	5A	4D	
Skagit	Electricians - Powerline Construction	Powderperson	$47.55	5A	4D	
Skagit	Electronic Technicians	Electronic Technicians Journey Level	$37.74	5B	1B	
Skagit	Elevator Constructors	Mechanic	$82.67	7D	4A	
Skagit	Elevator Constructors	Mechanic In Charge	$89.40	7D	4A	
Skagit	Fabricated Precast Concrete Products	Journey Level - In-Factory Work Only	$13.50		1	
Skagit	Fence Erectors	Fence Erector	$12.00		1	
Skagit	Flaggers	Journey Level	$36.17	7A	3I	
Skagit	Glaziers	Journey Level	$54.91	7L	1Y	
Skagit	Heat & Frost Insulators And Asbestos Workers	Journeyman	$61.18	5J	1S	
Skagit	Heating Equipment Mechanics	Mechanic	$57.51	7F	1E	
Skagit	Hod Carriers & Mason Tenders	Journey Level	$44.00	7A	3I	
Skagit	Industrial Power Vacuum Cleaner	Journey Level	$9.47		1	
Skagit	Inland Boatmen	Boat Operator	$54.57	5B	1K	
Skagit	Inland Boatmen	Cook	$50.95	5B	1K	
Skagit	Inland Boatmen	Deckhand	$51.19	5B	1K	
Skagit	Inland Boatmen	Deckhand Engineer	$52.18	5B	1K	
Skagit	Inland Boatmen	Launch Operator	$53.40	5B	1K	

Skagit	Inland Boatmen	Mate	$53.40	5B	1K	
Skagit	Inspection/Cleaning/Sealing Of Sewer & Water Systems By Remote Control	Cleaner Operator, Foamer Operator	$9.73		1	
Skagit	Inspection/Cleaning/Sealing Of Sewer & Water Systems By Remote Control	Grout Truck Operator	$11.48		1	
Skagit	Inspection/Cleaning/Sealing Of Sewer & Water Systems By Remote Control	Head Operator	$12.78		1	
Skagit	Inspection/Cleaning/Sealing Of Sewer & Water Systems By Remote Control	Technician	$9.47		1	
Skagit	Inspection/Cleaning/Sealing Of Sewer & Water Systems By Remote Control	Tv Truck Operator	$10.53		1	
Skagit	Insulation Applicators	Journey Level	$52.32	5D	4C	
Skagit	Ironworkers	Journeyman	$61.62	7N	1O	
Skagit	Laborers	Air, Gas Or Electric Vibrating Screed	$42.67	7A	3I	
Skagit	Laborers	Airtrac Drill Operator	$44.00	7A	3I	
Skagit	Laborers	Ballast Regular Machine	$42.67	7A	3I	
Skagit	Laborers	Batch Weighman	$36.17	7A	3I	
Skagit	Laborers	Brick Pavers	$42.67	7A	3I	
Skagit	Laborers	Brush Cutter	$42.67	7A	3I	
Skagit	Laborers	Brush Hog Feeder	$42.67	7A	3I	
Skagit	Laborers	Burner	$42.67	7A	3I	
Skagit	Laborers	Caisson Worker	$44.00	7A	3I	
Skagit	Laborers	Carpenter Tender	$42.67	7A	3I	
Skagit	Laborers	Caulker	$42.67	7A	3I	
Skagit	Laborers	Cement Dumper-paving	$43.46	7A	3I	
Skagit	Laborers	Cement Finisher Tender	$42.67	7A	3I	
Skagit	Laborers	Change House Or Dry Shack	$42.67	7A	3I	
Skagit	Laborers	Chipping Gun (under 30 Lbs.)	$42.67	7A	3I	
Skagit	Laborers	Chipping Gun(30 Lbs. And Over)	$43.46	7A	3I	
Skagit	Laborers	Choker Setter	$42.67	7A	3I	
Skagit	Laborers	Chuck Tender	$42.67	7A	3I	
Skagit	Laborers	Clary Power Spreader	$43.46	7A	3I	
Skagit	Laborers	Clean-up Laborer	$42.67	7A	3I	
Skagit	Laborers	Concrete Dumper/chute Operator	$43.46	7A	3I	
Skagit	Laborers	Concrete Form Stripper	$42.67	7A	3I	
Skagit	Laborers	Concrete Placement Crew	$43.46	7A	3I	
Skagit	Laborers	Concrete Saw Operator/core Driller	$43.46	7A	3I	
Skagit	Laborers	Crusher Feeder	$36.17	7A	3I	
Skagit	Laborers	Curing Laborer	$42.67	7A	3I	

Skagit	Laborers	Demolition: Wrecking & Moving (incl. Charred Material)	$42.67	7A	3I	
Skagit	Laborers	Ditch Digger	$42.67	7A	3I	
Skagit	Laborers	Diver	$44.00	7A	3I	
Skagit	Laborers	Drill Operator (hydraulic,diamond)	$43.46	7A	3I	
Skagit	Laborers	Dry Stack Walls	$42.67	7A	3I	
Skagit	Laborers	Dump Person	$42.67	7A	3I	
Skagit	Laborers	Epoxy Technician	$42.67	7A	3I	
Skagit	Laborers	Erosion Control Worker	$42.67	7A	3I	
Skagit	Laborers	Faller & Bucker Chain Saw	$43.46	7A	3I	
Skagit	Laborers	Fine Graders	$42.67	7A	3I	
Skagit	Laborers	Firewatch	$36.17	7A	3I	
Skagit	Laborers	Form Setter	$42.67	7A	3I	
Skagit	Laborers	Gabian Basket Builders	$42.67	7A	3I	
Skagit	Laborers	General Laborer	$42.67	7A	3I	
Skagit	Laborers	Grade Checker & Transit Person	$44.00	7A	3I	
Skagit	Laborers	Grinders	$42.67	7A	3I	
Skagit	Laborers	Grout Machine Tender	$42.67	7A	3I	
Skagit	Laborers	Groutmen (pressure)including Post Tension Beams	$43.46	7A	3I	
Skagit	Laborers	Guardrail Erector	$42.67	7A	3I	
Skagit	Laborers	Hazardous Waste Worker (level A)	$44.00	7A	3I	
Skagit	Laborers	Hazardous Waste Worker (level B)	$43.46	7A	3I	
Skagit	Laborers	Hazardous Waste Worker (level C)	$42.67	7A	3I	
Skagit	Laborers	High Scaler	$44.00	7A	3I	
Skagit	Laborers	Jackhammer	$43.46	7A	3I	
Skagit	Laborers	Laserbeam Operator	$43.46	7A	3I	
Skagit	Laborers	Maintenance Person	$42.67	7A	3I	
Skagit	Laborers	Manhole Builder-mudman	$43.46	7A	3I	
Skagit	Laborers	Material Yard Person	$42.67	7A	3I	
Skagit	Laborers	Motorman-dinky Locomotive	$43.46	7A	3I	
Skagit	Laborers	Nozzleman (concrete Pump, Green Cutter When Using Combination Of High Pressure Air & Water On Concrete & Rock, Sandblast, Gunite, Shotcrete, Water Bla	$43.46	7A	3I	
Skagit	Laborers	Pavement Breaker	$43.46	7A	3I	
Skagit	Laborers	Pilot Car	$36.17	7A	3I	
Skagit	Laborers	Pipe Layer Lead	$44.00	7A	3I	
Skagit	Laborers	Pipe Layer/tailor	$43.46	7A	3I	
Skagit	Laborers	Pipe Pot Tender	$43.46	7A	3I	

Skagit	Laborers	Pipe Reliner	$43.46	7A	3I	
Skagit	Laborers	Pipe Wrapper	$43.46	7A	3I	
Skagit	Laborers	Pot Tender	$42.67	7A	3I	
Skagit	Laborers	Powderman	$44.00	7A	3I	
Skagit	Laborers	Powderman's Helper	$42.67	7A	3I	
Skagit	Laborers	Power Jacks	$43.46	7A	3I	
Skagit	Laborers	Railroad Spike Puller - Power	$43.46	7A	3I	
Skagit	Laborers	Raker - Asphalt	$44.00	7A	3I	
Skagit	Laborers	Re-timberman	$44.00	7A	3I	
Skagit	Laborers	Remote Equipment Operator	$43.46	7A	3I	
Skagit	Laborers	Rigger/signal Person	$43.46	7A	3I	
Skagit	Laborers	Rip Rap Person	$42.67	7A	3I	
Skagit	Laborers	Rivet Buster	$43.46	7A	3I	
Skagit	Laborers	Rodder	$43.46	7A	3I	
Skagit	Laborers	Scaffold Erector	$42.67	7A	3I	
Skagit	Laborers	Scale Person	$42.67	7A	3I	
Skagit	Laborers	Sloper (over 20")	$43.46	7A	3I	
Skagit	Laborers	Sloper Sprayer	$42.67	7A	3I	
Skagit	Laborers	Spreader (concrete)	$43.46	7A	3I	
Skagit	Laborers	Stake Hopper	$42.67	7A	3I	
Skagit	Laborers	Stock Piler	$42.67	7A	3I	
Skagit	Laborers	Tamper & Similar Electric, Air & Gas Operated Tools	$43.46	7A	3I	
Skagit	Laborers	Tamper (multiple & Self-propelled)	$43.46	7A	3I	
Skagit	Laborers	Timber Person - Sewer (lagger, Shorer & Cribber)	$43.46	7A	3I	
Skagit	Laborers	Toolroom Person (at Jobsite)	$42.67	7A	3I	
Skagit	Laborers	Topper	$42.67	7A	3I	
Skagit	Laborers	Track Laborer	$42.67	7A	3I	
Skagit	Laborers	Track Liner (power)	$43.46	7A	3I	
Skagit	Laborers	Traffic Control Laborer	$38.68	7A	3I	8R
Skagit	Laborers	Traffic Control Supervisor	$38.68	7A	3I	8R
Skagit	Laborers	Truck Spotter	$42.67	7A	3I	
Skagit	Laborers	Tugger Operator	$43.46	7A	3I	
Skagit	Laborers	Tunnel Work-Compressed Air Worker 0-30 psi	$64.99	7A	3I	8Q
Skagit	Laborers	Tunnel Work-Compressed Air Worker 30.01-44.00 psi	$70.02	7A	3I	8Q
Skagit	Laborers	Tunnel Work-Compressed Air Worker 44.01-54.00 psi	$73.70	7A	3I	8Q
Skagit	Laborers	Tunnel Work-Compressed Air Worker 54.01-60.00 psi	$79.40	7A	3I	8Q
Skagit	Laborers	Tunnel Work-Compressed Air Worker 60.01-64.00 psi	$81.52	7A	3I	8Q
Skagit	Laborers	Tunnel Work-Compressed Air Worker 64.01-68.00 psi	$86.62	7A	3I	8Q

Skagit	Laborers	Tunnel Work-Compressed Air Worker 68.01-70.00 psi	$88.52	7A	3I	8Q
Skagit	Laborers	Tunnel Work-Compressed Air Worker 70.01-72.00 psi	$90.52	7A	3I	8Q
Skagit	Laborers	Tunnel Work-Compressed Air Worker 72.01-74.00 psi	$92.52	7A	3I	8Q
Skagit	Laborers	Tunnel Work-Guage and Lock Tender	$44.10	7A	3I	8Q
Skagit	Laborers	Tunnel Work-Miner	$44.10	7A	3I	8Q
Skagit	Laborers	Vibrator	$43.46	7A	3I	
Skagit	Laborers	Vinyl Seamer	$42.67	7A	3I	
Skagit	Laborers	Watchman	$32.87	7A	3I	
Skagit	Laborers	Welder	$43.46	7A	3I	
Skagit	Laborers	Well Point Laborer	$43.46	7A	3I	
Skagit	Laborers	Window Washer/cleaner	$32.87	7A	3I	
Skagit	Laborers - Underground Sewer & Water	General Laborer & Topman	$42.67	7A	3I	
Skagit	Laborers - Underground Sewer & Water	Pipe Layer	$43.46	7A	3I	
Skagit	Landscape Construction	Irrigation Or Lawn Sprinkler Installers	$14.15		1	
Skagit	Landscape Construction	Landscape Equipment Operators Or Truck Drivers	$14.15		1	
Skagit	Landscape Construction	Landscaping or Planting Laborers	$14.18		1	
Skagit	Lathers	Journey Level	$52.32	5D	1H	
Skagit	Marble Setters	Journey Level	$51.32	5A	1M	
Skagit	Metal Fabrication (In Shop)	Fitter	$15.16		1	
Skagit	Metal Fabrication (In Shop)	Laborer	$11.13		1	
Skagit	Metal Fabrication (In Shop)	Machine Operator	$10.66		1	
Skagit	Metal Fabrication (In Shop)	Painter	$11.41		1	
Skagit	Metal Fabrication (In Shop)	Welder	$15.16		1	
Skagit	Millwright	Journey Level	$38.36		1	
Skagit	Modular Buildings	Journey Level	$9.47		1	
Skagit	Painters	Journey Level	$37.80	6Z	2B	
Skagit	Pile Driver	Journey Level	$52.57	5D	4C	
Skagit	Plasterers	Journey Level	$50.42	7Q	1R	
Skagit	Playground & Park Equipment Installers	Journey Level	$9.47		1	
Skagit	Plumbers & Pipefitters	Journey Level	$63.57	5A	1G	
Skagit	Power Equipment Operators	Asphalt Plant Operators	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Assistant Engineer	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Barrier Machine (zipper)	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Batch Plant Operator, Concrete	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Bobcat	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Brokk - Remote Demolition Equipment	$51.97	7A	3C	8P

Skagit	Power Equipment Operators	Brooms	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Bump Cutter	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Cableways	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Chipper	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Compressor	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Concrete Pump: Truck Mount With Boom Attachment Over 42 M	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Concrete Finish Machine -laser Screed	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Concrete Pump - Mounted Or Trailer High Pressure Line Pump, Pump High Pressure.	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Concrete Pump: Truck Mount With Boom Attachment Up To 42m	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Conveyors	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Cranes: 20 Tons Through 44 Tons With Attachments	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Cranes: 100 Tons Through 199 Tons, Or 150' Of Boom (Including Jib With Attachments)	$55.79	7A	3C	8P
Skagit	Power Equipment Operators	Cranes: 200 Tons To 300 Tons, Or 250' Of Boom (including Jib With Attachments)	$56.36	7A	3C	8P
Skagit	Power Equipment Operators	Cranes: 45 Tons Through 99 Tons, Under 150' Of Boom (including Jib With Attachments)	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Cranes: A-frame - 10 Tons And Under	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Cranes: Friction 100 Tons Through 199 Tons	$56.36	7A	3C	8P
Skagit	Power Equipment Operators	Cranes: Friction Over 200 Tons	$56.92	7A	3C	8P
Skagit	Power Equipment Operators	Cranes: Over 300 Tons Or 300' Of Boom (including Jib With Attachments)	$56.92	7A	3C	8P
Skagit	Power Equipment Operators	Cranes: Through 19 Tons With Attachments A-frame Over 10 Tons	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Crusher	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Deck Engineer/deck Winches (power)	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Derricks, On Building Work	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Dozers D-9 & Under	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Drill Oilers: Auger Type, Truck Or Crane Mount	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Drilling Machine	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Elevator And Man-lift: Permanent And Shaft Type	$51.97	7A	3C	8P

Skagit	Power Equipment Operators	Finishing Machine, Bidwell And Gamaco & Similar Equipment	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Forklift: 3000 Lbs And Over With Attachments	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Forklifts: Under 3000 Lbs. With Attachments	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Grade Engineer: Using Blue Prints, Cut Sheets, Etc	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Gradechecker/stakeman	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Guardrail Punch	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Hard Tail End Dump Articulating Off- Road Equipment 45 Yards. & Over	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Hard Tail End Dump Articulating Off-road Equipment Under 45 Yards	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Horizontal/directional Drill Locator	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Horizontal/directional Drill Operator	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Hydralifts/boom Trucks Over 10 Tons	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Hydralifts/boom Trucks, 10 Tons And Under	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Loader, Overhead 8 Yards. & Over	$55.79	7A	3C	8P
Skagit	Power Equipment Operators	Loader, Overhead, 6 Yards. But Not Including 8 Yards	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Loaders, Overhead Under 6 Yards	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Loaders, Plant Feed	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Loaders: Elevating Type Belt	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Locomotives, All	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Material Transfer Device	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Mechanics, All (leadmen - $0.50 Per Hour Over Mechanic)	$55.79	7A	3C	8P
Skagit	Power Equipment Operators	Motor Patrol Grader - Non-finishing	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Motor Patrol Graders, Finishing	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Mucking Machine, Mole, Tunnel Drill, Boring, Road Header And/or Shield	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Oil Distributors, Blower Distribution & Mulch Seeding Operator	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Outside Hoists (elevators And Manlifts), Air Tuggers,strato	$54.33	7A	3C	8P
Skagit	Power Equipment Operators		$54.75	7A	3C	8P

		Overhead, Bridge Type Crane: 20 Tons Through 44 Tons				
Skagit	Power Equipment Operators	Overhead, Bridge Type: 100 Tons And Over	$55.79	7A	3C	8P
Skagit	Power Equipment Operators	Overhead, Bridge Type: 45 Tons Through 99 Tons	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Pavement Breaker	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Pile Driver (other Than Crane Mount)	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Plant Oiler - Asphalt, Crusher	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Posthole Digger, Mechanical	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Power Plant	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Pumps - Water	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Quad 9, Hd 41, D10 And Over	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Quick Tower - No Cab, Under 100 Feet In Height Based To Boom	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Remote Control Operator On Rubber Tired Earth Moving Equipment	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Rigger And Bellman	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Rollagon	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Roller, Other Than Plant Mix	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Roller, Plant Mix Or Multi-lift Materials	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Roto-mill, Roto-grinder	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Saws - Concrete	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Scraper, Self Propelled Under 45 Yards	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Scrapers - Concrete & Carry All	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Scrapers, Self-propelled: 45 Yards And Over	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Service Engineers - Equipment	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Shotcrete/gunite Equipment	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Shovel , Excavator, Backhoe, Tractors Under 15 Metric Tons.	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Shovel, Excavator, Backhoe: Over 30 Metric Tons To 50 Metric Tons	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Shovel, Excavator, Backhoes, Tractors: 15 To 30 Metric Tons	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Shovel, Excavator, Backhoes: Over 50 Metric Tons To 90 Metric Tons	$55.79	7A	3C	8P
Skagit	Power Equipment Operators	Shovel, Excavator, Backhoes: Over 90 Metric Tons	$56.36	7A	3C	8P
Skagit	Power Equipment Operators	Slipform Pavers	$55.24	7A	3C	8P
Skagit	Power Equipment Operators		$55.24	7A	3C	8P

		Spreader, Topsider & Screedman				
Skagit	Power Equipment Operators	Subgrader Trimmer	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Tower Bucket Elevators	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Tower Crane Over 175'in Height, Base To Boom	$56.36	7A	3C	8P
Skagit	Power Equipment Operators	Tower Crane Up To 175' In Height Base To Boom	$55.79	7A	3C	8P
Skagit	Power Equipment Operators	Transporters, All Track Or Truck Type	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Trenching Machines	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Truck Crane Oiler/driver - 100 Tons And Over	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Truck Crane Oiler/driver Under 100 Tons	$54.33	7A	3C	8P
Skagit	Power Equipment Operators	Truck Mount Portable Conveyor	$54.75	7A	3C	8P
Skagit	Power Equipment Operators	Welder	$55.24	7A	3C	8P
Skagit	Power Equipment Operators	Wheel Tractors, Farmall Type	$51.97	7A	3C	8P
Skagit	Power Equipment Operators	Yo Yo Pay Dozer	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Asphalt Plant Operators	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Assistant Engineer	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Barrier Machine (zipper)	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Batch Plant Operator, Concrete	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Bobcat	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Brokk - Remote Demolition Equipment	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Brooms	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Bump Cutter	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Cableways	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Chipper	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Compressor	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Concrete Pump: Truck Mount With Boom Attachment Over 42 M	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Concrete Finish Machine -laser Screed	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Concrete Pump - Mounted Or Trailer High Pressure Line Pump, Pump High Pressure.	$54.33	7A	3C	8P

Skagit	Power Equipment Operators-Underground Sewer & Water	Concrete Pump: Truck Mount With Boom Attachment Up To 42m	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Conveyors	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Cranes: 20 Tons Through 44 Tons With Attachments	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Cranes: 100 Tons Through 199 Tons, Or 150' Of Boom (Including Jib With Attachments)	$55.79	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Cranes: 200 Tons To 300 Tons, Or 250' Of Boom (including Jib With Attachments)	$56.36	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Cranes: 45 Tons Through 99 Tons, Under 150' Of Boom (including Jib With Attachments)	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Cranes: A-frame - 10 Tons And Under	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Cranes: Friction 100 Tons Through 199 Tons	$56.36	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Cranes: Friction Over 200 Tons	$56.92	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Cranes: Over 300 Tons Or 300' Of Boom (including Jib With Attachments)	$56.92	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Cranes: Through 19 Tons With Attachments A-frame Over 10 Tons	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Crusher	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Deck Engineer/deck Winches (power)	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Derricks, On Building Work	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Dozers D-9 & Under	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Drill Oilers: Auger Type, Truck Or Crane Mount	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Drilling Machine	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Elevator And Man-lift: Permanent And Shaft Type	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Finishing Machine, Bidwell And Gamaco & Similar Equipment	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Forklift: 3000 Lbs And Over With Attachments	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Forklifts: Under 3000 Lbs. With Attachments	$51.97	7A	3C	8P
Skagit			$54.75	7A	3C	8P

	Power Equipment Operators-Underground Sewer & Water	Grade Engineer: Using Blue Prints, Cut Sheets, Etc				
Skagit	Power Equipment Operators-Underground Sewer & Water	Gradechecker/stakeman	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Guardrail Punch	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Hard Tail End Dump Articulating Off- Road Equipment 45 Yards. & Over	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Hard Tail End Dump Articulating Off-road Equipment Under 45 Yards	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Horizontal/directional Drill Locator	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Horizontal/directional Drill Operator	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Hydralifts/boom Trucks Over 10 Tons	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Hydralifts/boom Trucks, 10 Tons And Under	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Loader, Overhead 8 Yards. & Over	$55.79	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Loader, Overhead, 6 Yards. But Not Including 8 Yards	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Loaders, Overhead Under 6 Yards	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Loaders, Plant Feed	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Loaders: Elevating Type Belt	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Locomotives, All	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Material Transfer Device	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Mechanics, All (leadmen - $0.50 Per Hour Over Mechanic)	$55.79	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Motor Patrol Grader - Non-finishing	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Motor Patrol Graders, Finishing	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Mucking Machine, Mole, Tunnel Drill, Boring, Road Header And/or Shield	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Oil Distributors, Blower Distribution & Mulch Seeding Operator	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Outside Hoists (elevators And Manlifts), Air Tuggers,strato	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Overhead, Bridge Type Crane: 20 Tons Through 44 Tons	$54.75	7A	3C	8P
Skagit			$55.79	7A	3C	8P

	Power Equipment Operators-Underground Sewer & Water	Overhead, Bridge Type: 100 Tons And Over				
Skagit	Power Equipment Operators-Underground Sewer & Water	Overhead, Bridge Type: 45 Tons Through 99 Tons	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Pavement Breaker	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Pile Driver (other Than Crane Mount)	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Plant Oiler - Asphalt, Crusher	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Posthole Digger, Mechanical	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Power Plant	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Pumps - Water	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Quad 9, Hd 41, D10 And Over	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Quick Tower - No Cab, Under 100 Feet In Height Based To Boom	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Remote Control Operator On Rubber Tired Earth Moving Equipment	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Rigger And Bellman	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Rollagon	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Roller, Other Than Plant Mix	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Roller, Plant Mix Or Multi-lift Materials	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Roto-mill, Roto-grinder	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Saws - Concrete	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Scraper, Self Propelled Under 45 Yards	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Scrapers - Concrete & Carry All	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Scrapers, Self-propelled: 45 Yards And Over	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Service Engineers - Equipment	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Shotcrete/gunite Equipment	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Shovel , Excavator, Backhoe, Tractors Under 15 Metric Tons.	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Shovel, Excavator, Backhoe: Over 30 Metric Tons To 50 Metric Tons	$55.24	7A	3C	8P

Skagit	Power Equipment Operators-Underground Sewer & Water	Shovel, Excavator, Backhoes, Tractors: 15 To 30 Metric Tons	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Shovel, Excavator, Backhoes: Over 50 Metric Tons To 90 Metric Tons	$55.79	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Shovel, Excavator, Backhoes: Over 90 Metric Tons	$56.36	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Slipform Pavers	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Spreader, Topsider & Screedman	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Subgrader Trimmer	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Tower Bucket Elevators	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Tower Crane Over 175'in Height, Base To Boom	$56.36	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Tower Crane Up To 175' In Height Base To Boom	$55.79	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Transporters, All Track Or Truck Type	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Trenching Machines	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Truck Crane Oiler/driver - 100 Tons And Over	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Truck Crane Oiler/driver Under 100 Tons	$54.33	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Truck Mount Portable Conveyor	$54.75	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Welder	$55.24	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Wheel Tractors, Farmall Type	$51.97	7A	3C	8P
Skagit	Power Equipment Operators-Underground Sewer & Water	Yo Yo Pay Dozer	$54.75	7A	3C	8P
Skagit	Power Line Clearance Tree Trimmers	Journey Level In Charge	$45.75	5A	4A	
Skagit	Power Line Clearance Tree Trimmers	Spray Person	$43.38	5A	4A	
Skagit	Power Line Clearance Tree Trimmers	Tree Equipment Operator	$45.75	5A	4A	
Skagit	Power Line Clearance Tree Trimmers	Tree Trimmer	$40.84	5A	4A	
Skagit	Power Line Clearance Tree Trimmers	Tree Trimmer Groundperson	$30.74	5A	4A	
Skagit	Refrigeration & Air Conditioning Mechanics	Journey Level	$23.95		1	
Skagit	Residential Brick Mason	Journey Level	$25.00		1	
Skagit	Residential Carpenters	Journey Level	$20.53		1	
Skagit	Residential Cement Masons	Journey Level	$16.00		1	
Skagit		Journey Level	$40.14	5D	4C	

	Residential Drywall Applicators					
Skagit	Residential Drywall Tapers	Journey Level	$30.00		1	
Skagit	Residential Electricians	JOURNEY LEVEL	$28.93		1	
Skagit	Residential Glaziers	Journey Level	$37.30	7L	1H	
Skagit	Residential Insulation Applicators	Journey Level	$13.96		1	
Skagit	Residential Laborers	Journey Level	$18.46		1	
Skagit	Residential Marble Setters	Journey Level	$25.00		1	
Skagit	Residential Painters	Journey Level	$15.00		1	
Skagit	Residential Plumbers & Pipefitters	Journey Level	$39.44	5A	1G	
Skagit	Residential Refrigeration & Air Conditioning Mechanics	Journey Level	$37.72	5A	1G	
Skagit	Residential Sheet Metal Workers	Journey Level (Field or Shop)	$20.91		1	
Skagit	Residential Soft Floor Layers	Journey Level	$23.46		1	
Skagit	Residential Sprinkler Fitters (Fire Protection)	Journey Level	$29.76		1	
Skagit	Residential Stone Masons	Journey Level	$25.00		1	
Skagit	Residential Terrazzo Workers	Journey Level	$25.00		1	
Skagit	Residential Terrazzo/Tile Finishers	Journey Level	$27.75		1	
Skagit	Residential Tile Setters	Journey Level	$25.00		1	
Skagit	Roofers	Journey Level	$31.84		1	
Skagit	Sheet Metal Workers	Journey Level (Field or Shop)	$57.51	7F	1E	
Skagit	Shipbuilding & Ship Repair	Carpenter	$21.69		1	
Skagit	Shipbuilding & Ship Repair	Electrician	$18.72		1	
Skagit	Shipbuilding & Ship Repair	Heat & Frost Insulator	$61.18	5J	1S	
Skagit	Shipbuilding & Ship Repair	Laborer	$11.71		1	
Skagit	Shipbuilding & Ship Repair	Machinist	$18.72		1	
Skagit	Shipbuilding & Ship Repair	Operator	$18.72		1	
Skagit	Shipbuilding & Ship Repair	Painter	$18.72		1	
Skagit	Shipbuilding & Ship Repair	Pipefitter	$18.72		1	
Skagit	Shipbuilding & Ship Repair	Welder/burner	$18.72		1	
Skagit	Sign Makers & Installers (Electrical)	Journey Level	$16.03		1	
Skagit	Sign Makers & Installers (Non-Electrical)	Journey Level	$13.28		1	
Skagit	Soft Floor Layers	Journey Level	$42.41	5A	3D	
Skagit	Solar Controls For Windows	Journey Level	$9.47		1	
Skagit	Sprinkler Fitters (Fire Protection)	Journey Level	$69.74	5C	1X	
Skagit	Stage Rigging Mechanics (Non Structural)	Journey Level	$13.23		1	
Skagit	Stone Masons	Journey Level	$51.32	5A	1M	
Skagit	Street And Parking Lot Sweeper Workers	Journey Level	$15.00		1	

Skagit	Surveyors	Assistant Construction Site Surveyor	$54.33	7A	3C	8P
Skagit	Surveyors	Chainman	$53.81	7A	3C	8P
Skagit	Surveyors	Construction Site Surveyor	$55.24	7A	3C	8P
Skagit	Telecommunication Technicians	Journey Level	$27.65		1	
Skagit	Telephone Line Construction - Outside	Cable Splicer	$36.96	5A	2B	
Skagit	Telephone Line Construction - Outside	Hole Digger/Ground Person	$20.49	5A	2B	
Skagit	Telephone Line Construction - Outside	Installer (Repairer)	$35.40	5A	2B	
Skagit	Telephone Line Construction - Outside	Special Aparatus Installer I	$36.96	5A	2B	
Skagit	Telephone Line Construction - Outside	Special Apparatus Installer II	$36.19	5A	2B	
Skagit	Telephone Line Construction - Outside	Telephone Equipment Operator (Heavy)	$36.96	5A	2B	
Skagit	Telephone Line Construction - Outside	Telephone Equipment Operator (Light)	$34.34	5A	2B	
Skagit	Telephone Line Construction - Outside	Telephone Lineperson	$34.34	5A	2B	
Skagit	Telephone Line Construction - Outside	Television Groundperson	$19.45	5A	2B	
Skagit	Telephone Line Construction - Outside	Television Lineperson/Installer	$25.89	5A	2B	
Skagit	Telephone Line Construction - Outside	Television System Technician	$30.97	5A	2B	
Skagit	Telephone Line Construction - Outside	Television Technician	$27.77	5A	2B	
Skagit	Telephone Line Construction - Outside	Tree Trimmer	$34.34	5A	2B	
Skagit	Terrazzo Workers	Journey Level	$46.96	5A	1M	
Skagit	Tile Setters	Journey Level	$46.96	5A	1M	
Skagit	Tile, Marble & Terrazzo Finishers	Journey Level	$25.00		1	
Skagit	Traffic Control Stripers	Journey Level	$43.11	7A	1K	
Skagit	Truck Drivers	Asphalt Mix Over 16 Yards (W. WA-Joint Council 28)	$49.85	5D	3A	8L
Skagit	Truck Drivers	Asphalt Mix To 16 Yards (W. WA-Joint Council 28)	$49.01	5D	3A	8L
Skagit	Truck Drivers	Dump Truck	$16.98		1	
Skagit	Truck Drivers	Dump Truck And Trailer	$16.98		1	
Skagit	Truck Drivers	Other Trucks (W. WA-Joint Council 28)	$49.85	5D	3A	8L
Skagit	Truck Drivers	Transit Mixer	$32.12		1	
Skagit	Well Drillers & Irrigation Pump Installers	Irrigation Pump Installer	$11.60		1	
Skagit	Well Drillers & Irrigation Pump Installers	Oiler	$9.47		1	

Skagit	Well Drillers & Irrigation Pump Installers	Well Driller	$11.60		1	

****

Overtime Codes

Overtime calculations are based on the hourly rate actually paid to the worker. On public works projects, the hourly rate must be not less than the prevailing rate of wage minus the hourly rate of the cost of fringe benefits actually provided for the worker.

1. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY OR FORTY (40) HOURS PER WEEK SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

B. All hours worked on Saturdays shall be paid at one and one-half times the hourly rate of wage. All hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

C. The first two (2) hours after eight (8) regular hours Monday through Friday and the first ten (10) hours on Saturday shall be paid at one and one-half times the hourly rate of wage. All other overtime hours and all hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

D. The first two (2) hours before or after a five-eight (8) hour workweek day or a four-ten (10) hour workweek day and the first eight (8) hours worked the next day after either workweek shall be paid at one and one-half times the hourly rate of wage. All additional hours worked and all worked on Sundays and holidays shall be paid at double the hourly rate of wage.

E. The first two (2) hours after eight (8) regular hours Monday through Friday and the first eight (8) hours on Saturday shall be paid at one and one-half times the hourly rate of wage. All other hours worked Monday through Saturday, and all hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

F. The first two (2) hours after eight (8) regular hours Monday through Friday and the first ten (10) hours on Saturday shall be paid at one and one-half times the hourly rate of wage. All other overtime hours worked, except Labor Day, shall be paid at double the hourly rate of wage. All hours worked on Labor Day shall be paid at three times the hourly rate of wage.

G. The first ten (10) hours worked on Saturdays and the first ten (10) hours worked on a fifth calendar weekday in a four-ten hour schedule, shall be paid at one and one-half times the hourly rate of wage. All hours worked in excess of ten (10) hours per day Monday through Saturday and all hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

H. All hours worked on Saturdays (except makeup days if work is lost due to inclement weather conditions or equipment breakdown) shall be paid at one and one-half times the hourly rate of wage. All hours worked Monday through Saturday over twelve (12) hours and all hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

I. All hours worked on Sundays and holidays shall also be paid at double the hourly rate of wage.

J. The first two (2) hours after eight (8) regular hours Monday through Friday and the first ten (10) hours on Saturday shall be paid at one and one-half times the hourly rate of wage. All hours worked over ten (10) hours Monday through Saturday, Sundays and holidays shall be paid at double the hourly rate of wage.

K. All hours worked on Saturdays and Sundays shall be paid at one and one-half times the hourly rate of wage. All hours worked on holidays shall be paid at double the hourly rate of wage.

M. All hours worked on Saturdays (except makeup days if work is lost due to inclement weather conditions) shall be paid at one and one-half times the hourly rate of wage. All hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

1. N. All hours worked on Saturdays (except makeup days) shall be paid at one and one-half times the hourly rate of wage. All hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

   O. The first ten (10) hours worked on Saturday shall be paid at one and one-half times the hourly rate of wage. All hours worked on Sundays, holidays and after twelve (12) hours, Monday through Friday and after ten (10) hours on Saturday shall be paid at double the hourly rate of wage.

   P. All hours worked on Saturdays (except makeup days if circumstances warrant) and Sundays shall be paid at one and one-half times the hourly rate of wage. All hours worked on holidays shall be paid at double the hourly rate of wage.

   Q. The first two (2) hours after eight (8) regular hours Monday through Friday and up to ten (10) hours worked on Saturdays shall be paid at one and one-half times the hourly rate of wage. All hours worked in excess of ten (10) hours per day Monday through Saturday and all hours worked on Sundays and holidays (except Christmas day) shall be paid at double the hourly rate of wage. All hours worked on Christmas day shall be paid at two and one-half times the hourly rate of wage.

   R. All hours worked on Sundays and holidays shall be paid at two times the hourly rate of wage.

   S. The first two (2) hours after eight (8) regular hours Monday through Friday and the first eight (8) hours on Saturday shall be paid at one and one-half times the hourly rate of wage. All hours worked on holidays and all other overtime hours worked, except Labor Day, shall be paid at double the hourly rate of wage. All hours worked on Labor Day shall be paid at three times the hourly rate of wage.

   U. All hours worked on Saturdays shall be paid at one and one-half times the hourly rate of wage. All hours worked on Sundays and holidays (except Labor Day) shall be paid at two times the hourly rate of wage. All hours worked on Labor Day shall be paid at three times the hourly rate of wage.

   V. All hours worked on Sundays and holidays (except Thanksgiving Day and Christmas day) shall be paid at one and one-half times the hourly rate of wage. All hours worked on Thanksgiving Day and Christmas day shall be paid at double the hourly rate of wage.

   W. All hours worked on Saturdays and Sundays (except make-up days due to conditions beyond the control of the employer)) shall be paid at one and one-half times the hourly rate of wage. All hours worked on holidays shall be paid at double the hourly rate of wage.

   X. The first four (4) hours after eight (8) regular hours Monday through Friday and the first twelve (12) hours on Saturday shall be paid at one and one-half times the hourly rate of wage. All hours worked over twelve (12) hours Monday through Saturday, Sundays and holidays shall be paid at double the hourly rate of wage. When holiday falls on Saturday or Sunday, the day before Saturday, Friday, and the day after Sunday, Monday, shall be considered the holiday and all work performed shall be paid at double the hourly rate of wage.

   Y. All hours worked outside the hours of 5:00 am and 5:00 pm (or such other hours as may be agreed upon by any employer and the employee) and all hours worked in excess of eight (8) hours per day (10 hours per day for a 4 x 10 workweek) and on Saturdays and holidays (except labor day) shall be paid at one and one-half times the hourly rate of wage. (except for employees who are absent from work without prior approval on a scheduled workday during the workweek shall be paid at the straight-time rate until they have worked 8 hours in a day (10 in a 4 x 10 workweek) or 40 hours during that workweek.) All hours worked Monday through Saturday over twelve (12) hours and all hours worked on Sundays and Labor Day shall be paid at double the hourly rate of wage.

   Z. All hours worked on Saturdays and Sundays shall be paid at one and one-half times the hourly rate of wage. All hours worked on holidays shall be paid the straight time rate of pay in addition to holiday pay.

2. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY OR FORTY (40) HOURS PER WEEK SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

B. All hours worked on holidays shall be paid at one and one-half times the hourly rate of wage.

C. All hours worked on Sundays shall be paid at one and one-half times the hourly rate of wage. All hours worked on holidays shall be paid at two times the hourly rate of wage.

F. The first eight (8) hours worked on holidays shall be paid at the straight hourly rate of wage in addition to the holiday pay. All hours worked in excess of eight (8) hours on holidays shall be paid at double the hourly rate of wage.

G. All hours worked on Sunday shall be paid at two times the hourly rate of wage. All hours worked on paid holidays shall be paid at two and one-half times the hourly rate of wage including holiday pay.

H. All hours worked on Sunday shall be paid at two times the hourly rate of wage. All hours worked on holidays shall be paid at one and one-half times the hourly rate of wage.

O. All hours worked on Sundays and holidays shall be paid at one and one-half times the hourly rate of wage.

R. All hours worked on Sundays and holidays and all hours worked over sixty (60) in one week shall be paid at double the hourly rate of wage.

U. All hours worked on Saturdays shall be paid at one and one-half times the hourly rate of wage. All hours worked over 12 hours in a day or on Sundays and holidays shall be paid at double the hourly rate of wage.

W. The first two (2) hours after eight (8) regular hours Monday through Friday and the first eight (8) hours on Saturday shall be paid at one and one-half times the hourly rate of wage. All other hours worked Monday through Saturday, and all hours worked on Sundays and holidays shall be paid at double the hourly rate of wage. On a four-day, ten-hour weekly schedule, either Monday thru Thursday or Tuesday thru Friday schedule, all hours worked after ten shall be paid at double the hourly rate of wage. The first eight (8) hours worked on the fifth day shall be paid at one and one-half times the hourly rate of wage. All other hours worked on the fifth, sixth, and seventh days and on holidays shall be paid at double the hourly rate of wage.

3. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY OR FORTY (40) HOURS PER WEEK SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

A. Work performed in excess of eight (8) hours of straight time per day, or ten (10) hours of straight time per day when four ten (10) hour shifts are established, or forty (40) hours of straight time per week, Monday through Friday, or outside the normal shift, and all work on Saturdays shall be paid at time and one-half the straight time rate. Hours worked over twelve hours (12) in a single shift and all work performed after 6:00 pm Saturday to 6:00 am Monday and holidays shall be paid at double the straight time rate of pay. Any shift starting between the hours of 6:00 pm and midnight shall receive an additional one dollar ($1.00) per hour for all hours worked that shift. The employer shall have the sole discretion to assign overtime work to employees. Primary consideration for overtime work shall be given to employees regularly assigned to the work to be performed on overtime situations. After an employee has worked eight (8) hours at an applicable overtime rate, all additional hours shall be at the applicable overtime rate until such time as the employee has had a break of eight (8) hours or more.

3. C. Work performed in excess of eight (8) hours of straight time per day, or ten (10) hours of straight time per day when four ten (10) hour shifts are established, or forty (40) hours of straight time per week, Monday through Friday, or outside the normal shift, and all work on Saturdays shall be paid at one and one-half times the hourly rate of wage. All work performed after 6:00 pm Saturday to 5:00 am Monday and Holidays shall be paid at double the hourly rate of wage. After an employee has worked eight (8) hours at an applicable overtime rate, all additional hours shall be at the applicable overtime rate until such time as the employee has had a break of eight (8) hours or more.

D. All hours worked between the hours of 6:00 pm and 6:00 am, Monday through Saturday, shall be paid at a premium rate of 15% over the hourly rate of wage. All other hours worked after 6:00 am on Saturdays, shall be paid at one and one-half times the hourly rate of wage. All hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

E. All hours worked Sundays and holidays shall be paid at double the hourly rate of wage. Each week, once 40 hours of straight time work is achieved, then any hours worked over 10 hours per day Monday through Saturday shall be paid at double the hourly wage rate.

F. All hours worked on Saturday shall be paid at one and one-half times the hourly rate of wage. All hours worked on Sunday shall be paid at two times the hourly rate of wage. All hours worked on paid holidays shall be paid at two and one-half times the hourly rate of wage including holiday pay.

H. All work performed on Sundays between March 16th and October 14th and all Holidays shall be compensated for at two (2) times the regular rate of pay. Work performed on Sundays between October 15th and March 15th shall be compensated at one and one half (1-1/2) times the regular rate of pay.

I. All hours worked on Saturdays shall be paid at one and one-half times the hourly rate of wage. In the event the job is down due to weather conditions during a five day work week (Monday through Friday,) or a four day-ten hour work week (Tuesday through Friday,) then Saturday may be worked as a voluntary make-up day at the straight time rate. However, Saturday shall not be utilized as a make-up day when a holiday falls on Friday. All hours worked Monday through Saturday over twelve (12) hours and all hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

4. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY OR FORTY (40) HOURS PER WEEK SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

A. All hours worked in excess of eight (8) hours per day or forty (40) hours per week shall be paid at double the hourly rate of wage. All hours worked on Saturdays, Sundays and holidays shall be paid at double the hourly rate of wage.

B. All hours worked over twelve (12) hours per day and all hours worked on holidays shall be paid at double the hourly rate of wage.

C. On Monday through Friday, the first four (4) hours of overtime after eight (8) hours of straight time work shall be paid at one and one half (1-1/2) times the straight time rate of pay, unless a four (4) day ten (10) hour workweek has been established. On a four (4) day ten (10) hour workweek scheduled Monday through Thursday, or Tuesday through Friday, the first two (2) hours of overtime after ten (10) hours of straight time work shall be paid at one and one half (1-1/2) times the straight time rate of pay. On Saturday, the first twelve (12) hours of work shall be paid at one and one half (1-1/2) times the straight time rate of pay, except that if the job is down on Monday through Friday due to weather conditions or other conditions outside the control of the employer, the first ten (10) hours on Saturday may be worked at the straight time rate of pay. All hours worked over twelve (12) hours in a day and all hours worked on Sunday and Holidays shall be paid at two (2) times the straight time rate of pay.

4. D. All hours worked in excess of eight (8) hours per day or forty (40) hours per week shall be paid at double the hourly rate of wage. All hours worked on Saturday, Sundays and holidays shall be paid at double the hourly rate of pay. Rates include all members of the assigned crew.

EXCEPTION:
On all multipole structures and steel transmission lines, switching stations, regulating, capacitor stations, generating plants, industrial plants, associated installations and substations, except those substations whose primary function is to feed a distribution system, will be paid overtime under the following rates:

The first two (2) hours after eight (8) regular hours Monday through Friday of overtime on a regular workday, shall be paid at one and one-half times the hourly rate of wage. All hours in excess of ten (10) hours will be at two (2) times the hourly rate of wage. The first eight (8) hours worked on Saturday will be paid at one and one-half (1-1/2) times the hourly rate of wage. All hours worked in excess of eight (8) hours on Saturday, and all hours worked on Sundays and holidays will be at the double the hourly rate of wage.

All overtime eligible hours performed on the above described work that is energized, shall be paid at the double the hourly rate of wage.

E. The first two (2) hours after eight (8) regular hours Monday through Friday and the first eight (8) hours on Saturday shall be paid at one and one-half times the hourly rate of wage. All other hours worked Monday through Saturday, and all hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

On a four-day, ten-hour weekly schedule, either Monday thru Thursday or Tuesday thru Friday schedule, all hours worked after ten shall be paid at double the hourly rate of wage. The Monday or Friday not utilized in the normal four-day, ten hour work week, and Saturday shall be paid at one and one half (1½) times the regular shift rate for the first eight (8) hours. All other hours worked Monday through Saturday, and all hours worked on Sundays and holidays shall be paid at double the hourly rate of wage.

F. All hours worked between the hours of 6:00 pm and 6:00 am, Monday through Saturday, shall be paid at a premium rate of 20% over the hourly rate of wage. All hours worked on Sundays shall be paid at one and one-half times the hourly rate of wage. All hours worked on holidays shall be paid at double the hourly rate of wage.

Holiday Codes

5. A. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Friday after Thanksgiving Day, and Christmas Day (7).

B. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Friday after Thanksgiving Day, the day before Christmas, and Christmas Day (8).

C. Holidays: New Year's Day, Presidents' Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, the Friday after Thanksgiving Day, And Christmas Day (8).

D. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, the Friday and Saturday after Thanksgiving Day, And Christmas Day (8).

H. Holidays: New Year's Day, Memorial Day, Independence Day, Thanksgiving Day, the Day after Thanksgiving Day, And Christmas (6).

I. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, and Christmas Day (6).

J. Holidays: New Year's Day, Memorial Day, Independence Day, Thanksgiving Day, Friday after Thanksgiving Day, Christmas Eve Day, And Christmas Day (7).

5. K. Holidays: New Year's Day, Presidents' Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Friday After Thanksgiving Day, The Day Before Christmas, And Christmas Day (9).

L. Holidays: New Year's Day, Martin Luther King Jr. Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Friday After Thanksgiving Day, And Christmas Day (8).

N. Holidays: New Year's Day, Presidents' Day, Memorial Day, Independence Day, Labor Day, Veterans' Day, Thanksgiving Day, The Friday After Thanksgiving Day, And Christmas Day (9).

P. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Friday And Saturday After Thanksgiving Day, The Day Before Christmas, And Christmas Day (9). If A Holiday Falls On Sunday, The Following Monday Shall Be Considered As A Holiday.

Q. Paid Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, and Christmas Day (6).

R. Paid Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Day After Thanksgiving Day, One-Half Day Before Christmas Day, And Christmas Day. (7 1/2).

S. Paid Holidays: New Year's Day, Presidents' Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, And Christmas Day (7).

T. Paid Holidays: New Year's Day, Washington's Birthday, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, The Friday After Thanksgiving Day, Christmas Day, And The Day Before Or After Christmas (9).

Z. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Veterans Day, Thanksgiving Day, the Friday after Thanksgiving Day, And Christmas Day (8).

Holiday Codes Continued

6. A. Paid Holidays: New Year's Day, Presidents' Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, The Friday After Thanksgiving Day, And Christmas Day (8).

E. Paid Holidays: New Year's Day, Day Before Or After New Year's Day, Presidents Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Day After Thanksgiving Day, Christmas Day, And A Half-Day On Christmas Eve Day. (9 1/2).

G. Paid Holidays: New Year's Day, Martin Luther King Jr. Day, Presidents' Day, Memorial Day, Independence Day, Labor Day, Veterans' Day, Thanksgiving Day, The Friday After Thanksgiving Day, Christmas Day, And Christmas Eve Day (11).

H. Paid Holidays: New Year's Day, New Year's Eve Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Friday After Thanksgiving Day, Christmas Day, The Day After Christmas, And A Floating Holiday (10).

I. Paid Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Friday After Thanksgiving Day, And Christmas Day (7).

T. Paid Holidays: New Year's Day, Presidents' Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, The Friday After Thanksgiving Day, The Last Working Day Before Christmas Day, And Christmas Day (9).

6. Z. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Friday after Thanksgiving Day, And Christmas Day (7). If a holiday falls on Saturday, the preceding Friday shall be considered as the holiday. If a holiday falls on Sunday, the following Monday shall be considered as the holiday.

Holiday Codes Continued

7. A. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, The Friday And Saturday After Thanksgiving Day, And Christmas Day (8). Any Holiday Which Falls On A Sunday Shall Be Observed As A Holiday On The Following Monday. If any of the listed holidays falls on a Saturday, the preceding Friday shall be a regular work day.

B. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, the Friday and Saturday after Thanksgiving Day, And Christmas Day (8). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

C. Holidays: New Year's Day, Martin Luther King Jr. Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, The Friday After Thanksgiving Day, And Christmas Day (8). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

D. Paid Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Veteran's Day, Thanksgiving Day, the Friday after Thanksgiving Day, And Christmas Day (8). Unpaid Holidays: President's Day. Any paid holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any paid holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

E. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, the Friday after Thanksgiving Day, And Christmas Day (7). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

F. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, the Friday after Thanksgiving Day, the last working day before Christmas day and Christmas day (8). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

G. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, and Christmas Day (6). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday.

H. Holidays: New Year's Day, Martin Luther King Jr. Day, Independence Day, Memorial Day, Labor Day, Thanksgiving Day, the Friday after Thanksgiving Day, the Last Working Day before Christmas Day and Christmas Day (9). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

I. Holidays: New Year's Day, President's Day, Independence Day, Memorial Day, Labor Day, Thanksgiving Day, The Friday After Thanksgiving Day, The Day Before Christmas Day And Christmas Day (9). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

J. Holidays: New Year's Day, Independence Day, Memorial Day, Labor Day, Thanksgiving Day and Christmas Day (6). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

7. K. Holidays: New Year's Day, Memorial Day, Independence Day, Thanksgiving Day, the Friday and Saturday after Thanksgiving Day, And Christmas Day (8). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

L. Holidays: New Year's Day, Memorial Day, Labor Day, Independence Day, Thanksgiving Day, the Last Work Day before Christmas Day, And Christmas Day (7). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

M. Paid Holidays: New Year's Day, The Day after or before New Year's Day, President's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, the Friday after Thanksgiving Day, Christmas Day, And the Day after or before Christmas Day 10). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

N. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, the Friday after Thanksgiving Day, And Christmas Day (7). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. When Christmas falls on a Saturday, the preceding Friday shall be observed as a holiday.

P. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Friday after Thanksgiving Day, And Christmas Day (7). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday.

Q. Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, the Friday after Thanksgiving Day, the Last Working Day before Christmas Day and Christmas Day (8). Any holiday which falls on a Sunday shall be observed as a holiday on the following Monday. If any of the listed holidays falls on a Saturday, the preceding Friday shall be a regular work day.

R. Paid Holidays: New Year's Day, the day after or before New Year's Day, President's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, the Friday after Thanksgiving Day, Christmas Day, and the day after or before Christmas Day (10). If any of the listed holidays fall on Saturday, the preceding Friday shall be observed as the holiday. If any of the listed holidays falls on a Sunday, the day observed by the Nation shall be considered a holiday and compensated accordingly.

S. Paid Holidays: New Year's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, Friday After Thanksgiving Day, Christmas Day, The Day After Christmas, And A Floating Holiday (9). If any of the listed holidays falls on a Sunday, the day observed by the Nation shall be considered a holiday and compensated accordingly.

T. Paid Holidays: New Year's Day, The Day After Or Before New Year's Day, President's Day, Memorial Day, Independence Day, Labor Day, Thanksgiving Day, The Friday After Thanksgiving Day, Christmas Day, and The Day After Or Before Christmas Day. (10). If any of the listed holidays falls on a Sunday, the day observed by the Nation shall be considered a holiday and compensated accordingly. Any holiday which falls on a Saturday shall be observed as a holiday on the preceding Friday.

Note Codes

8. A. In addition to the hourly wage and fringe benefits, the following depth premiums apply to depths of fifty feet or more:
Over 50' To 100' -$2.00 per Foot for Each Foot Over 50 Feet
Over 100' To 150' -$3.00 per Foot for Each Foot Over 100 Feet
Over 150' To 220' -$4.00 per Foot for Each Foot Over 150 Feet
Over 220' -$5.00 per Foot for Each Foot Over 220 Feet

8. C. In addition to the hourly wage and fringe benefits, the following depth premiums apply to depths of fifty feet or more:
Over 50’ To 100' -$1.00 per Foot for Each Foot Over 50 Feet
Over 100' To 150' -$1.50 per Foot for Each Foot Over 100 Feet
Over 150' To 200' -$2.00 per Foot for Each Foot Over 150 Feet
Over 200' -Divers May Name Their Own Price

D. Workers working with supplied air on hazmat projects receive an additional $1.00 per hour.

L. Workers on hazmat projects receive additional hourly premiums as follows -Level A: $0.75, Level B: $0.50, And Level C: $0.25.

M. Workers on hazmat projects receive additional hourly premiums as follows: Levels A & B: $1.00, Levels C & D: $0.50.

N. Workers on hazmat projects receive additional hourly premiums as follows -Level A: $1.00, Level B: $0.75, Level C: $0.50, And Level D: $0.25.

P. Workers on hazmat projects receive additional hourly premiums as follows -Class A Suit: $2.00, Class B Suit: $1.50, Class C Suit: $1.00, And Class D Suit $0.50.

Q. The highest pressure registered on the gauge for an accumulated time of more than fifteen (15) minutes during the shift shall be used in determining the scale paid.

R. Effective August 31, 2012 – A Traffic Control Supervisor shall be present on the project whenever flagging or spotting or other traffic control labor is being utilized. A Traffic Control Laborer performs the setup, maintenance and removal of all temporary traffic control devices and construction signs necessary to control vehicular, bicycle, and pedestrian traffic during construction operations. Flaggers and Spotters shall be posted where shown on approved Traffic Control Plans or where directed by the Engineer. All flaggers and spotters shall possess a current flagging card issued by the State of Washington, Oregon, Montana, or Idaho. These classifications are only effective on or after August 31, 2012.

S. Effective August 31, 2012 – A Traffic Control Supervisor shall be present on the project whenever flagging or spotting or other traffic control labor is being utilized. Flaggers and Spotters shall be posted where shown on approved Traffic Control Plans or where directed by the Engineer. All flaggers and spotters shall possess a current flagging card issued by the State of Washington, Oregon, Montana, or Idaho. This classification is only effective on or after August 31, 2012.

T. Effective August 31, 2012 – A Traffic Control Laborer performs the setup, maintenance and removal of all temporary traffic control devices and construction signs necessary to control vehicular, bicycle, and pedestrian traffic during construction operations. Flaggers and Spotters shall be posted where shown on approved Traffic Control Plans or where directed by the Engineer. All flaggers and spotters shall possess a current flagging card issued by the State of Washington, Oregon, Montana, or Idaho. This classification is only effective on or after August 31, 2012.

APPENDIX C

Construction Contract and Contract Bond-Informational Only

CONSTRUCTION CONTRACT AGREEMENT

THIS AGREEMENT, effective upon the date of mutual execution, is made and entered into between Skagit County, Washington, and _____, hereinafter called the Contractor.

WITNESSETH:
That in consideration of the terms and conditions contained herein and attached and made a part of this agreement, the parties hereto covenant and agree as follows:

I. The Contractor shall do all work and furnish all tools, materials, equipment, and transportation required for the construction of 2015 Resurfacing of ACP Roadways Project #ESAO15-1 in accordance with and as described in the attached plans and specifications and the Washington State Department of Transportation *Standard Specifications for Road, Bridge, and Municipal Construction M 41-10 2014 edition*, which are by this reference incorporated herein and made a part hereof, and shall perform any changes to the work in accord with the Contract Documents.

II. The Contractor shall provide and bear the expense of all equipment, work, and labor of any sort whatsoever that may be required for the transfer of materials and for constructing and completing the work provided for in this contract and every part thereof and shall guarantee said materials and work for a period of one year after substantial completion of this contract, except as may be modified by the plans, specifications and/or contract documents.

III. Skagit County, Washington, hereby promises and agrees with the Contractor to retain and does retain the Contractor to provide the materials and to do and cause to be done the above-described work and to complete and finish the same according to the attached plans and specifications and the terms and conditions herein contained, and hereby contracts to pay for the same according to the attached specifications and the schedule of prices bid and hereto attached, at the time and in the manner and upon the conditions provided for in this contract.

IV. The Contractor for himself/herself, and for his/her heirs, executors, administrators, successors, and assigns, does hereby agree to full performance of all covenants required of the Contractor in the contract.

V. It is further provided that no liability shall attach to Skagit County by reason of entering into this contract, except as provided herein.

IN WITNESS WHEREOF the Contractor has executed this instrument on the day and year first below written, and the Authorized Official has caused this instrument to be executed by and in the name of Skagit County the day and year first above written.

CONTRACTOR

Signature _____ Mailing Address:

Printed _____

Title _____

Date _____ Telephone No. (___) ____-____

DATED this _____ day of _____, 2015.

BOARD OF COUNTY COMMISSIONERS
SKAGIT COUNTY, WASHINGTON

_____
Kenneth A. Dahlstedt, Chair

_____
Lisa Janicki, Commissioner

Attest: _____
Ron Wesen, Commissioner

_____
Clerk of the Board

For contracts under $5,000:
Authorization per Resolution R20030146

Recommended: _____
County Administrator

_____
Department Head

Approved as to form:

_____
Civil Deputy Prosecuting Attorney

Approved as to indemnification:

_____
Risk Manager

Approved as to budget:

_____
Budget & Finance Director

CONTRACT BOND

KNOW ALL MEN BY THESE PRESENTS, that Skagit County, a Municipal Corporation of Washington, has awarded _____ of _____, as Principal, and _____as Surety, are jointly and severally held and bound unto the County of Skagit in the penal sum of _____ ($_____), dollars, for the payment of which we jointly and severely bind ourselves, our heirs, executors, administrators, and assigns, and successors and assigns, firmly by these presents.

THE CONDITION of this bond is such that whereas, on the _____ day of _____ A.D., 2015, the said Principal, herein, executed a certain contract with the County of Skagit by the items, conditions and provisions of which contract the said _____, Principal, herein agree to furnish all material and do certain work, to wit: That _____ will undertake and complete the construction of

2015 Resurfacing of ACP Roadways Project #ESAO15-1

according to the maps, plans and specifications made a part of said contract, which contract as so executed, is hereunto attached, is now referred to and by reference is incorporated herein and made a part hereof as fully for all purposes as if here set forth at length. The bond shall cover all approved change orders as if they were in the original contract.

NOW, THEREFORE, if the Principal herein shall faithfully and truly observe and comply with the terms, conditions and provisions of said contract in all respects and shall well and truly and fully do and perform all matters and things by _____ (principal) undertaken to be performed under said contract, upon the terms proposed therein, and within the time prescribed therein, and until the same is accepted, and shall pay all laborers, mechanics, subcontractors and material men, and all persons who shall supply such contractor or subcontractor with provisions and supplies for the carrying on of such work, and shall in all respects faithfully perform said contract according to law, then this obligation to be void, otherwise to remain in full force and effect.

WITNESS our hands this _____ day of _____, 2015.

_____

_____

_____
(Principal)

_____

_____

_____
Attorney-in-Fact, Surety

_____
Name and Address
Local Office of Agent

APPROVED AS TO FORM
RICH WEYRICH
Skagit County Prosecuting Attorney

APPROVED AS TO FORM
JESSICA NEIL HOYSON
Skagit County Risk Manager

BY: _____
Approving Authority

DATE: _____, 2015

_____
SURETY BOND NUMBER

_____
CONTRACT NUMBER

_____

_____

APPENDIX D

Proposal Forms-Informational Only

Proposal for Bidding Purposes

For Construction of:

2015 RESURFACING OF ACP ROADWAYS PROJECT #ESAO15-1

SKAGIT COUNTY PUBLIC WORKS

SKAGIT COUNTY
Public Works Department
1800 Continental Place
Mount Vernon, WA 98273

PROPOSAL

2015 RESURFACING OF ACP ROADWAYS
PROJECT #ESAO15-1

All bid envelopes must be plainly marked on the outside, **"Sealed Bid, 2015 Resurfacing of ACP Roadways Project #ESAO15-1**

Sealed Bids will be received at the following location before the specified time:

Bids may be hand delivered to: The Reception Desk of Skagit County Commissioners Office, located at 1800 Continental Place, Mount Vernon, WA.

Bids may be mailed to: Skagit County Commissioners
1800 Continental Place, Suite 100
Mount Vernon, Washington, 98273

The bid opening date for this project will be **Monday, June 8, 2015.** The bids will be publicly opened and read after **2:30 p.m.** on this date.

Bid Advertisement: Skagit Valley Herald – May 21, May 28, 2015

ENTIRE PROPOSAL TO BE RETURNED AS YOUR BID PACKAGE

FAILURE TO SIGN OR COMPLETE ALL INFORMATION ON THE FORMS PROVIDED CAN RESULT IN REJECTION OF THE PROPOSAL AS NON-RESPONSIVE

PROPOSAL

BOARD OF SKAGIT COUNTY COMMISSIONERS
MOUNT VERNON, WASHINGTON 98273

Attention:

This certifies that the undersigned has examined the locations of:

2015 RESURFACING OF ACP ROADWAYS PROJECT #ESAO15-1

and that the plans, specifications and contract governing the work embraced in this improvement, and the method by which payment will be made for said work is understood. The undersigned hereby proposes to undertake and complete the work embraced in this improvement, or as much thereof as can be completed with the money available in accordance with the said plans, specifications, and contract, and the following schedule of rates and prices:

Note: for work performed on this project the contractor should refer to Section 1-07.2(1) of the contract provisions and Department of Revenue Rule #171.

(Note: Unit prices for all items, all extensions, and total amount of bid shall be shown. All entries must be typed or entered in ink.)

2015 RESURFACING OF ACP ROADWAYS PROJECT #ESAO15-1

Item No.	Description	Spec	QTY	Unit of Measure	Unit Price	Total Price
1	MOBILIZATION	1-09.7	1.00	LS	$_____ .___	$_____ .___
2	SPCC PLAN	1-07.15(1)	1.00	LS	$_____ .___	$_____ .___
3	UNANTICIPATED SITE WORK	1-09.6 SP	EST	DOL	$ **1.00**	$ **5,000.00**
4	TRAFFIC CONTROL LABOR	1-10.5(2)	500.00	HR	$_____ .___	$_____ .___
5	TRAFFIC CONTROL SUPERVISOR	1-10.5(2)	1.00	LS	$_____ .___	$_____ .___
6	OTHER TEMPORARY TRAFFIC CONTROL	1-10.5(2)	1.00	LS	$_____ .___	$_____ .___

7	CONSTRUCTION SIGNS CLASS A	1-10.5(2)	312.00	SF	$_____ .___	$_____ .___
8	REMOVING MISC. TRAFFIC ITEMS	2-02.5 SP	1.00	LS	$_____ .___	$_____ .___
9	TRIMMING AND CLEANUP	2-11.5	1.00	LS	$_____ .___	$_____ .___
10	PLANING BITUMINOUS PAVEMENT	5-04.5 SP	2600.00	SY	$_____ .___	$_____ .___
11	PAVEMENT REPAIR EXCAVATION INCL. HAUL	5-04.5 SP	74.00	SY	$_____ .___	$_____ .___
12	2" DEPTH PLANING REPAIR	5-04.5 SP	673.00	SY	$_____ .___	$_____ .___
13	HMA CL 1/2" PG 64-22	5-04.5 SP	6300.00	TON	$_____ .___	$_____ .___
14	MEMBRANE WATER PROOFING (DECK SEAL)	6-08.5 SP	202.00	SY	$_____ .___	$_____ .___
15	ESC LEAD	8-01.5	2.00	DAY	$_____ .___	$_____ .___
16	EROSION CONTROL	8-01.5	1.00	LS	$_____ .___	$_____ .___
					TOTAL BID	**$**_____ **.**___

FOR WORK PERFORMED ON THIS PROJECT THE CONTRACTOR SHOULD REFER TO SECTION 1-07.2(1) OF THE CONTRACT PROVISIONS AND DEPARTMENT OF REVENUE RULE #171.

PROPOSAL – Signature Page

The bidder is hereby advised that by signature of this proposal he/she is deemed to have acknowledged all requirements and signed all certificates contained herein.

The undersigned hereby agrees to pay labor not less than the prevailing rates of wages in accordance with the requirements of the special provisions for this project.

A proposal guaranty in an amount of five percent (5%) of the total bid based upon the approximate estimate of quantities at the above prices and in the form as indicated below is attached hereto:

☐	CASHIER'S CHECK	In the amount of $_____ Dollars
☐	CERTIFIED CHECK (Payable to Skagit County)	In the amount of $_____ Dollars
☐	PROPOSAL BOND	In the amount five percent (5%) of the total bid.

Receipt is hereby acknowledged of Addendum(s) No. (s) _____, _____, & _____

Signature of Authorized Officials(s):

Proposal Must Be Signed →

_____

_____

PRINT NAME

Firm Name: _____

Address: _____

_____

_____

Telephone No.: _____

State of Washington Contractor's License No._____

UBI No._____

Employment Security Department No._____

Note:

(1) This proposal form is not transferable and any alteration of the firm's name entered hereon without prior permission from the Skagit County will be cause for considering the proposal irregular and subsequent rejection of the bid.

(2) Please refer to Section 1-02.6 of the Standard Specifications, "Preparation of Proposal", or "Article 4" of the Instruction to Bidders for building construction jobs.

BID PROPOSAL MUST BE SIGNED.

FAILURE TO SIGN OR COMPLETE ALL INFORMATION CAN RESULT IN REJECTION OF THE PROPOSAL AS NON-RESPONSIVE.

SUBMIT THE ENCLOSED PROPOSAL BOND FORM WITH YOUR PROPOSAL

USE OF OTHER FORMS MAY SUBJECT YOUR BID TO REJECTION

PROPOSAL BOND

KNOW ALL MEN BY THESE PRESENTS, That we, _____

_____

of _____ as principal, and the

_____ a corporation duly

organized under the laws of the State of _____,
and authorized to do business in the State of Washington, as surety, are held and firmly bound unto Skagit County in the full and penal sum of five (5) percent of the total amount of the bid proposal of said principal for the work hereinafter described for the payment of which, well and truly to be made, we bind our heirs, executors, administrators and assigns, and successors and assigns, firmly by these presents.

The condition of this bond is such, that whereas the principal herein is herewith submitting his or its sealed proposal for the following highway construction, to wit:

2015 Resurfacing of ACP Roadways Project #ESAO15-1

said bid and proposal, by reference thereto, being made a part hereof.

NOW THEREFORE, If the said proposal bid by said principal be accepted, and the contract be awarded to said principal, and if said principal shall duly make and enter into and execute said contract and shall furnish bond as required by Skagit County within a period of twenty (20) days from and after said award, exclusive of the day of such award, then this obligation shall be null and void, otherwise it shall remain and be in full force and effect.

IN TESTIMONY WHEREOF, The principal and surety have caused these presents to be signed and sealed this _____ day of _____, 2015.

_____

_____
(Principal)

_____ _____
(Surety) (Attorney-in-fact)

Failure to return this Declaration as part of the bid proposal package will make the bid nonresponsive and ineligible for award.

NON-COLLUSION DECLARATION

I, by signing the proposal, hereby declare, under penalty of perjury under the laws of the United States that the following statements are true and correct:

1. That the undersigned person(s), firm, association or corporation has (have) not, either directly or indirectly, entered into any agreement, participated in any collusion, or otherwise taken any action in restraint of free competitive bidding in connection with the project for which this proposal is submitted.
2. **That by signing the signature page of this proposal, I am deemed to have signed and to have agreed to the provisions of this declaration.**

NOTICE TO ALL BIDDERS

To report rigging activities call:

1-800-424-9071

The U.S. Department of Transportation (USDOT) operates the above toll-free "hotline" Monday through Friday, 8:00 a.m. to 5:00 p.m., eastern time. Anyone with knowledge of possible bid rigging, bidder collusion, or other fraudulent activities should use the "hotline" to report such activities.

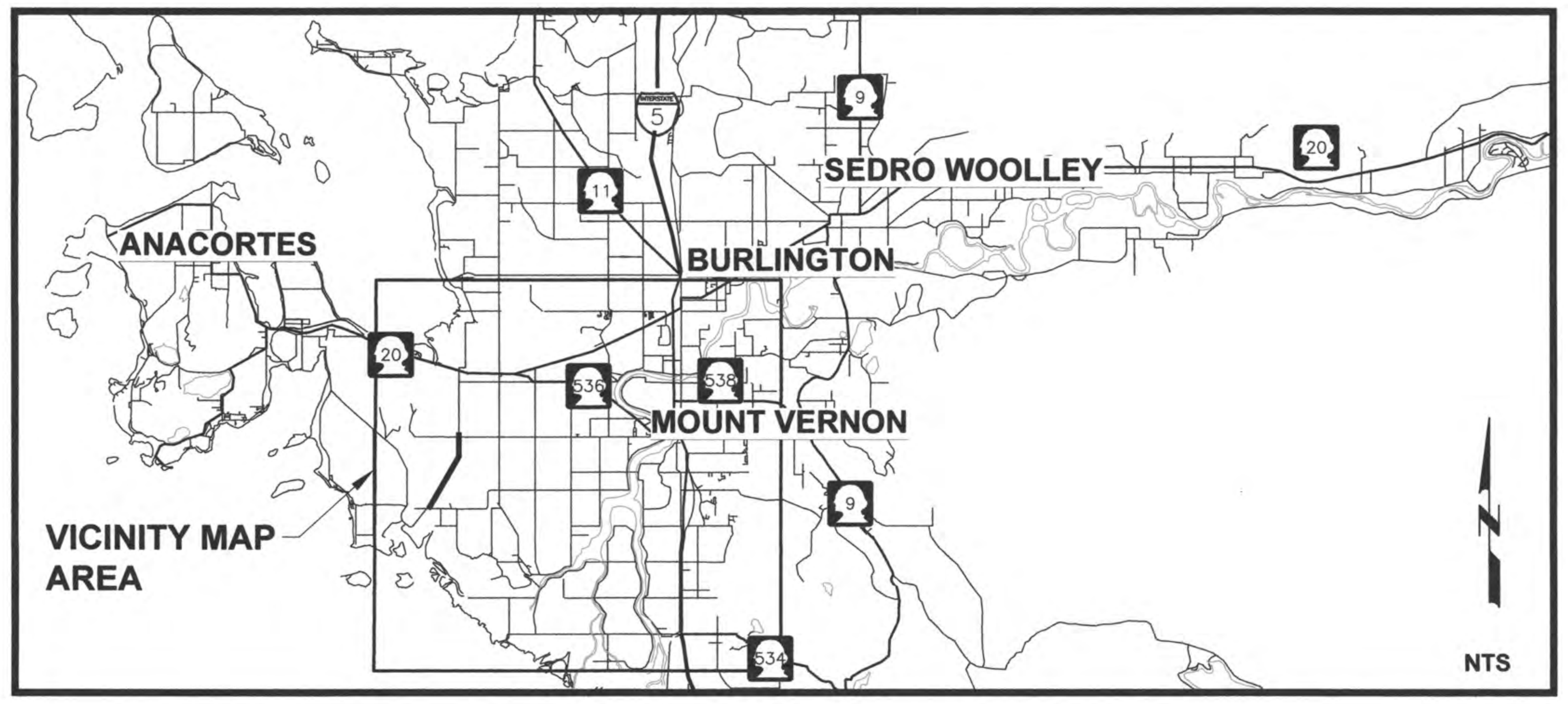
The "hotline" is part of USDOT's continuing effort to identify and investigate highway construction contract fraud and abuse and is operated under the direction of the USDOT Inspector General. All information will be treated confidentially and caller anonymity will be respected.

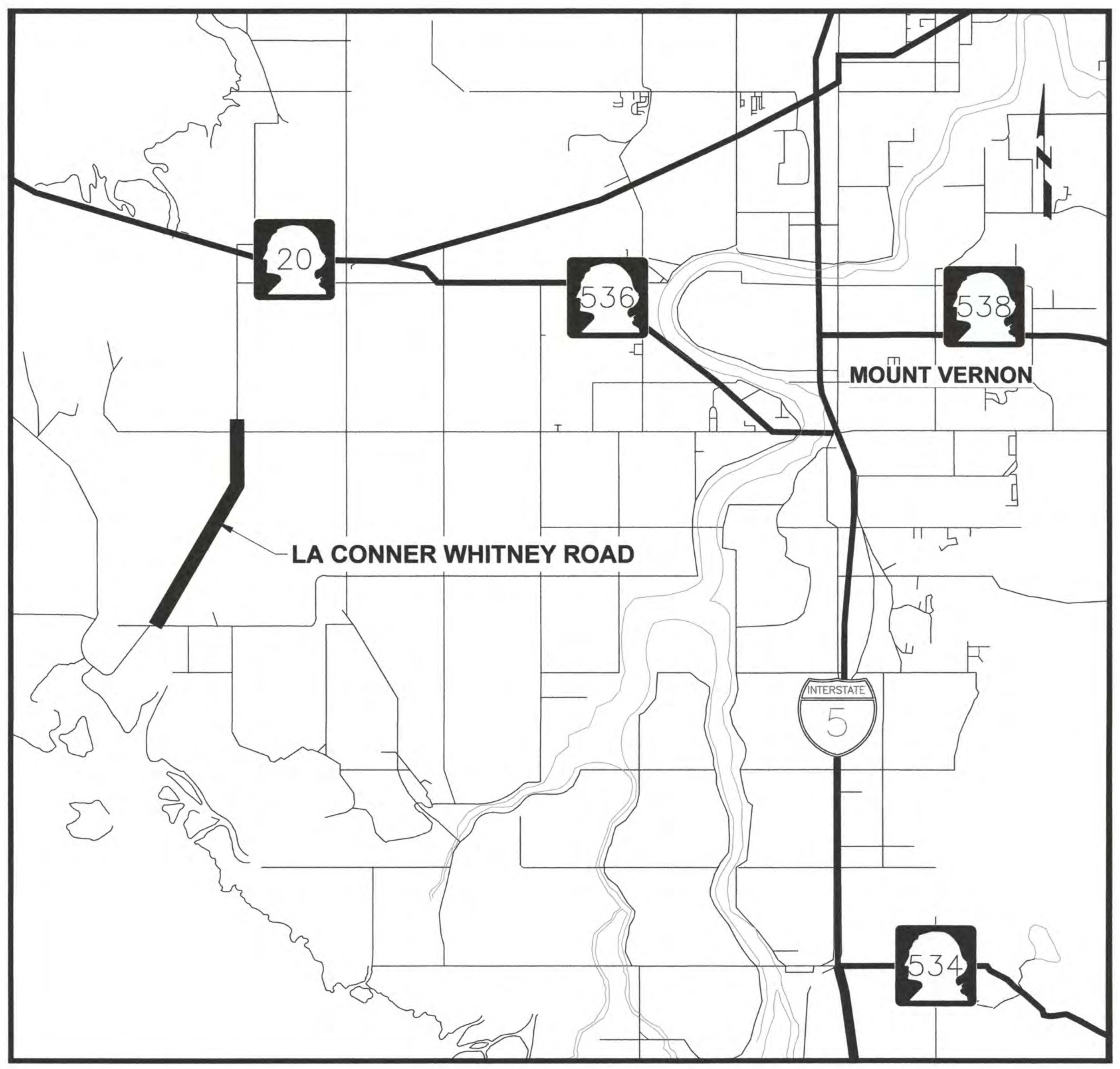
APPENDIX E

Vicinity Map and Plans

2015 RESURFACING OF ACP ROADWAYS

ESAO15-1





VICINITY MAP 1" = 1 MILE

SKAGIT COUNTY OFFICIALS

BOARD OF COMMISSIONERS
- KENNETH A. DAHLSTEDT, CHAIR
- LISA JANICKI, COMMISSIONER
- RON WESEN, COMMISSIONER

PUBLIC WORKS
- DAN BERENTSON, DIRECTOR

PAUL A. RANDALL–GRUTTER, P.E., COUNTY ENGINEER — DATE 5.4.15

SHEET LIST TABLE

SHEET	TITLE
1	COVER SHEET
2	SUMMARY OF QUANTITIES
3	DETAILS
4	DETAILS
5	LA CONNER WHITNEY ROAD PLAN STA 200+00 TO STA 280+50
6	LA CONNER WHITNEY ROAD PLAN STA 280+50 TO STA 318+50
7	CLASS A SIGNING PLAN

LINETYPES:

ROAD CENTERLINE
ASPHALT EDGE
ALIGNMENT LINE
RIGHT–OF–WAY LINE

CALL TWO (2) BUSINESS DAYS BEFORE YOU DIG
1–800–424–5555

SKAGIT COUNTY PUBLIC WORKS
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(360) 336-9400 FAX (360) 336 9478

NO. | REVISIONS | DATE

DESIGN ENGINEER: SHANE THOMAS ODEN, STATE OF WASHINGTON, 47001, REGISTERED PROFESSIONAL ENGINEER

ENGINEER OF RECORD: PAUL A. RANDALL-GRUTTER, STATE OF WASHINGTON, 40264, REGISTERED PROFESSIONAL ENGINEER

PROJECT NO.: ESAO15-1
FED. AID NO.:
DESIGNED BY: TH — DRAWN BY: TH
CHECKED BY: — APPROVED BY:

PROJECT LOCATED NEAR:
MOUNT VERNON, WA
T. 33 & 34 N., R. 3 E., W.M.

2015 RESURFACING OF ACP ROADWAYS

COVER SHEET

1 INCH SCALE BAR
ADJUST SCALE ACCORDINGLY

SHEET 1 OF 7

JEFFL - April 29, 2015 - 3:43 PM - J:\EDS\2015 ASPHALT OVERLAY ESAO15-1\DESIGN_CAD\CONSTR\CV01ESAO15-1.DWG

LA CONNER WHITNEY ROAD SUMMARY OF QUANTITIES				
Item No	**Spec.**	**Qty.**	**Unit**	**Item Description**
1	1-09.7	1	LS	MOBILIZATION
2	1-07.15(1)	1	LS	SPCC PLAN
3	1-09.6 SP	EST	DOL	UNANTICIPATED SITE WORK
4	1-10.5(2)	500	HR	TRAFFIC CONTROL LABOR
5	1-10.5(2)	1	LS	TRAFFIC CONTROL SUPERVISOR
6	1-10.5(2)	1	LS	OTHER TEMPORARY TRAFFIC CONTROL
7	1-10.5(2)	312	SF	CONSTRUCTION SIGNS CLASS A
8	2-02.5 SP	1	LS	REMOVING MISC. TRAFFIC ITEMS
9	2-11.5	1	LS	TRIMMING AND CLEANUP
10	5-04.5 SP	2600	SY	PLANING BITUMINOUS PAVEMENT
11	5-04.5 SP	74	SY	PAVEMENT REPAIR EXCAVATION INCL. HAUL
12	5-04.5 SP	673	SY	2" DEPTH PLANING REPAIR
13	5-04.5 SP	6300	TON	HMA CL 1/2" PG 64-22
14	6-08.5 SP	202	SY	MEMBRANE WATER PROOFING (DECK SEAL)
15	8-01.5	2	DAY	ESC LEAD
16	8-01.5	1	LS	EROSION CONTROL
THE CONTRACTOR SHALL INCLUDE FOR COMPENSATION THE AMOUNT OF ANY TAXES PAID IN THE VARIOUS UNIT BID PRICES IN ACCORDANCE WITH SECTION 1-07.2(1)				

GENERAL CONSTRUCTION NOTES:

1. ALL WORK SHALL CONFORM TO THE 2014 EDITION OF THE WASHINGTON STATE DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND MUNICIPAL CONSTRUCTION AND AMENDMENTS.
2. UNDERGROUND AND OVERHEAD UTILITIES ARE KNOWN TO EXIST IN THE AREA OF CONSTRUCTION. THE LOCATION OF EXISTING UTILITIES SHOWN IS APPROXIMATE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CONTACT ALL UTILITY OWNERS FOR LOCATIONS AND TO FIELD VERIFY ALL UTILITY LOCATIONS PRIOR TO CONSTRUCTION. THE ONE-CALL NUMBER FOR UNDERGROUND UTILITIES IS 1-800-424-5555. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE INTEGRITY OF ALL EXISTING UTILITIES THROUGHOUT CONSTRUCTION. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROMPTLY NOTIFY THE ENGINEER OF ANY CONFLICT WITH EXISTING UTILITIES.
3. A COPY OF THE APPROVED PLANS AND SPECIFICATIONS MUST BE ON THE JOB SITE WHENEVER CONSTRUCTION IS IN PROGRESS.
4. THE INFORMATION SHOWN ON THESE PLANS IS APPROXIMATE AND IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ACTUAL CONDITIONS IN THE FIELD PRIOR TO BIDDING AND NOTICE TO PROCEED.
5. OVERLAY ENTIRE PROJECT LENGTH WITH 2" HMA CL ½" PG 64-22.
6. ALL EXISTING RAISED PAVEMENT MARKERS (RPM) SHALL BE REMOVED PRIOR TO OVERLAYING ASPHALT.

ABBREVIATIONS:

AC	ACRE	EVC	END VERTICAL CURVE	PT	POINT OF TANGENCY
AP	ANGLE POINT	FT & '	FEET	R	RADIUS
ASPH	ASPHALT	GALV	GALVANIZED	RD	ROAD
AVE	AVENUE	GND	GROUND	RGE	RANGE
BLDG	BUILDING	HMA	HOT MIX ASPHALT	RNP	RIGHT NO PASS
BLVD	BOULEVARD	HR	HOUR	RPM	RAISED PAVEMENT MARKER
BVC	BEGIN VERTICAL CURVE	IN & "	INCHES	R/W & ROW	RIGHT OF WAY
CALC	CALCULATED	INV	INVERT	RT	RIGHT
CAT	CATEGORY	K	CURVE COEFFICIENT	S	SOUTH
CB	CATCH BASIN	L	LENGTH OF CURVE	SEC	SECTION
CIP	CAST IRON PIPE	LF	LINEAR FOOT	SF	SQUARE FOOT
CFS	CUBIC FEET PER SECOND	LNP	LEFT NO PASS	SHLD	SHOULDER
CL & ℄	CENTERLINE	LS	LUMP SUM	SPEC	SPECIFICATIONS
CP	CONTROL POINT	LT	LEFT	ST	STREET
CPCP	CORRUGATED POLYETHYLENE CULVERT PIPE	MAX	MAXIMUM	STA	STATION
		MIN	MINIMUM	SY	SQUARE YARD
CPSSP	CORRUGATED POLYETHYLENE STORM SEWER PIPE	MIC	MONUMENT IN CASE	SYMM	SYMMETRICAL
		MON	MONUMENT	TWN	TOWNSHIP
CONT'D	CONTINUED	MUTCD	MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES	TYP	TYPICAL
CULV	CULVERT			UG	UNDERGROUND
CY	CUBIC YARD	N	NORTH/NORTHING	VCL	VERTICAL CURVE LENGTH
DEG	DEGREE	NTS	NOT TO SCALE	VPI	VERTICAL POINT INTERSECTION
DIA	DIAMETER	OD	OUTSIDE DIAMETER	W	WEST
DWG	DRAWING	OHW	ORDINARY HIGH WATER MARK	W.M.	WILLAMETTE MERIDIAN
E	EAST/EASTING	PERF	PERFORATED PIPE	WSDOT	WASHINGTON STATE DEPARTMENT OF TRANSPORTATION
EL & ELEV	ELEVATION	PC	POINT OF CURVATURE		
EST	ESTIMATED	PI	POINT OF INTERSECTION		

SKAGIT COUNTY
PUBLIC WORKS
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(360) 336-9400 FAX (360) 336 9478

NO.	REVISIONS	DATE

DESIGN ENGINEER
THOMAS ODEN
STATE OF WASHINGTON
21001
REGISTERED PROFESSIONAL ENGINEER

ENGINEER OF RECORD
RANDALL GRUTTER
STATE OF WASHINGTON
20228
REGISTERED PROFESSIONAL ENGINEER

PROJECT NO.: ESAO15-1
FED. AID NO.:
DESIGNED BY: TH
DRAWN BY: TH
CHECKED BY: RJ
APPROVED BY: TJO

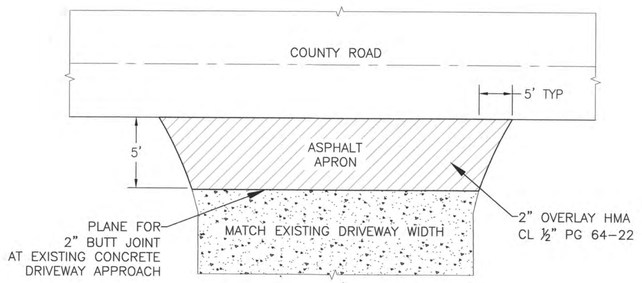
PROJECT LOCATED NEAR:
MOUNT VERNON, WA
T. 33 & 34 N., R. 3 E., W.M.

2015 RESURFACING OF ACP ROADWAYS

SUMMARY OF QUANTITIES

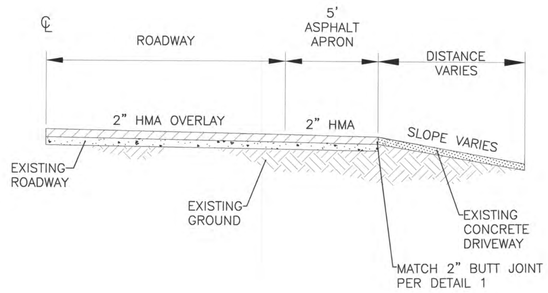
1 INCH SCALE BAR
ADJUST SCALE ACCORDINGLY

SHEET
2 OF 7



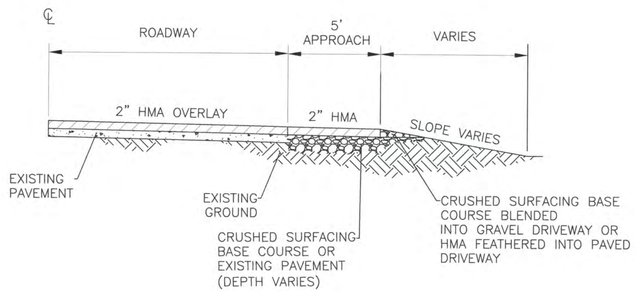
1 TYPICAL APPROACH APRON PLAN VIEW

NTS



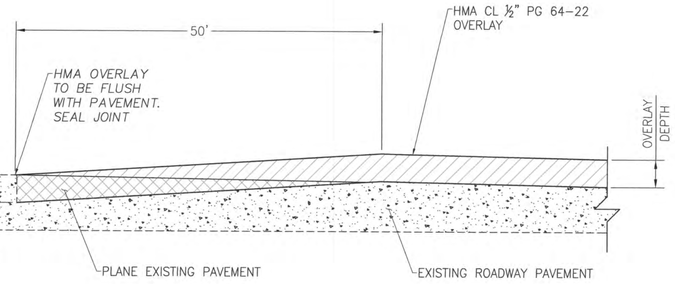
2 TYPICAL APPROACH APRON - CONCRETE DRIVEWAY

NTS



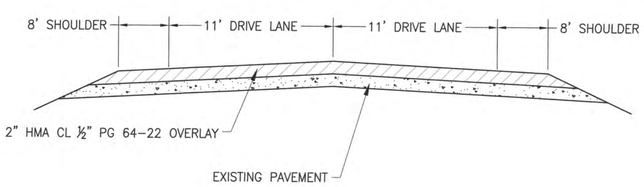
3 TYPICAL APPROACH APRON - ASPHALT OR GRAVEL DRIVEWAY OR FIELD ACCESS

NTS



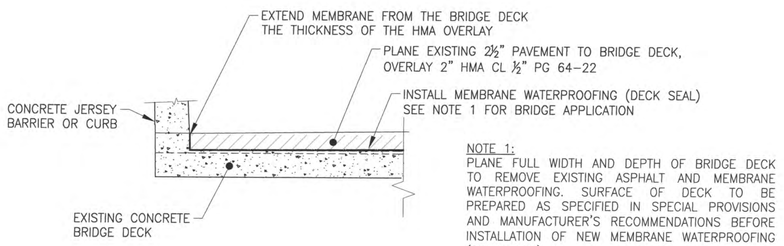
4 BUTT JOINT DETAIL

NTS



5 TYPICAL OVERLAY SECTION

NTS



6 TYPICAL BRIDGE PLANING DETAIL

NTS

CALL TWO (2) BUSINESS DAYS BEFORE YOU DIG

1-800-424-5555

SKAGIT COUNTY PUBLIC WORKS
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(360) 336-9400 FAX (360) 336 9478

NO.	REVISIONS	DATE

DESIGN ENGINEER
THOMAS ODEN
STATE OF WASHINGTON
21700
REGISTERED PROFESSIONAL ENGINEER

ENGINEER OF RECORD
RANDALL GRUTTER
STATE OF WASHINGTON
20284
REGISTERED PROFESSIONAL ENGINEER

PROJECT NO.: ESAO15-1
FED. AID NO.:
DESIGNED BY: TH
DRAWN BY: TH
CHECKED BY:
APPROVED BY:

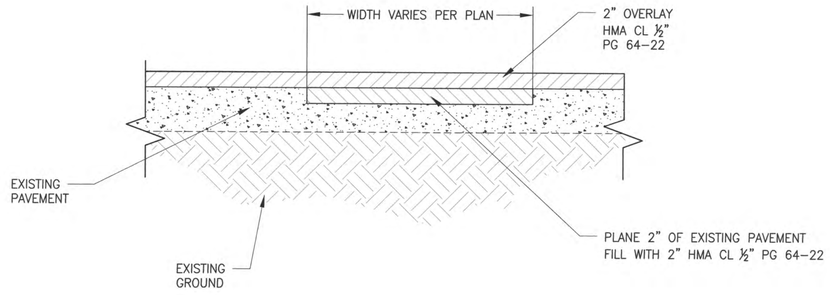
PROJECT LOCATED NEAR:
MOUNT VERNON, WA
T. 33 & 34 N., R. 3 E., W.M.

2015 RESURFACING OF ACP ROADWAYS

DETAILS

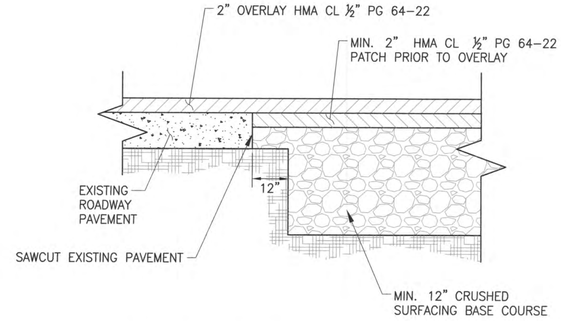
1 INCH SCALE BAR
ADJUST SCALE ACCORDINGLY

SHEET
3 OF 7



9 2" DEPTH PLANING REPAIR DETAIL

NTS



8 PAVEMENT REPAIR DETAIL

NTS

CALL TWO (2) BUSINESS DAYS BEFORE YOU DIG

1-800-424-5555

SKAGIT COUNTY
PUBLIC WORKS
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(360) 336-9400 FAX (360) 336 9478

NO.	REVISIONS	DATE

DESIGN ENGINEER
THOMAS ODEN
STATE OF WASHINGTON
17001
REGISTERED PROFESSIONAL ENGINEER

ENGINEER OF RECORD
PAUL A. RANDALL-GRUTTER
STATE OF WASHINGTON
40884
REGISTERED PROFESSIONAL ENGINEER

PROJECT NO.: ESAO15-1
FED. AID NO.:
DESIGNED BY: TH
DRAWN BY: TH
CHECKED BY: PAV
APPROVED BY: DJV

PROJECT LOCATED NEAR:
MOUNT VERNON, WA
T. 33 & 34 N., R. 3 E., W.M.

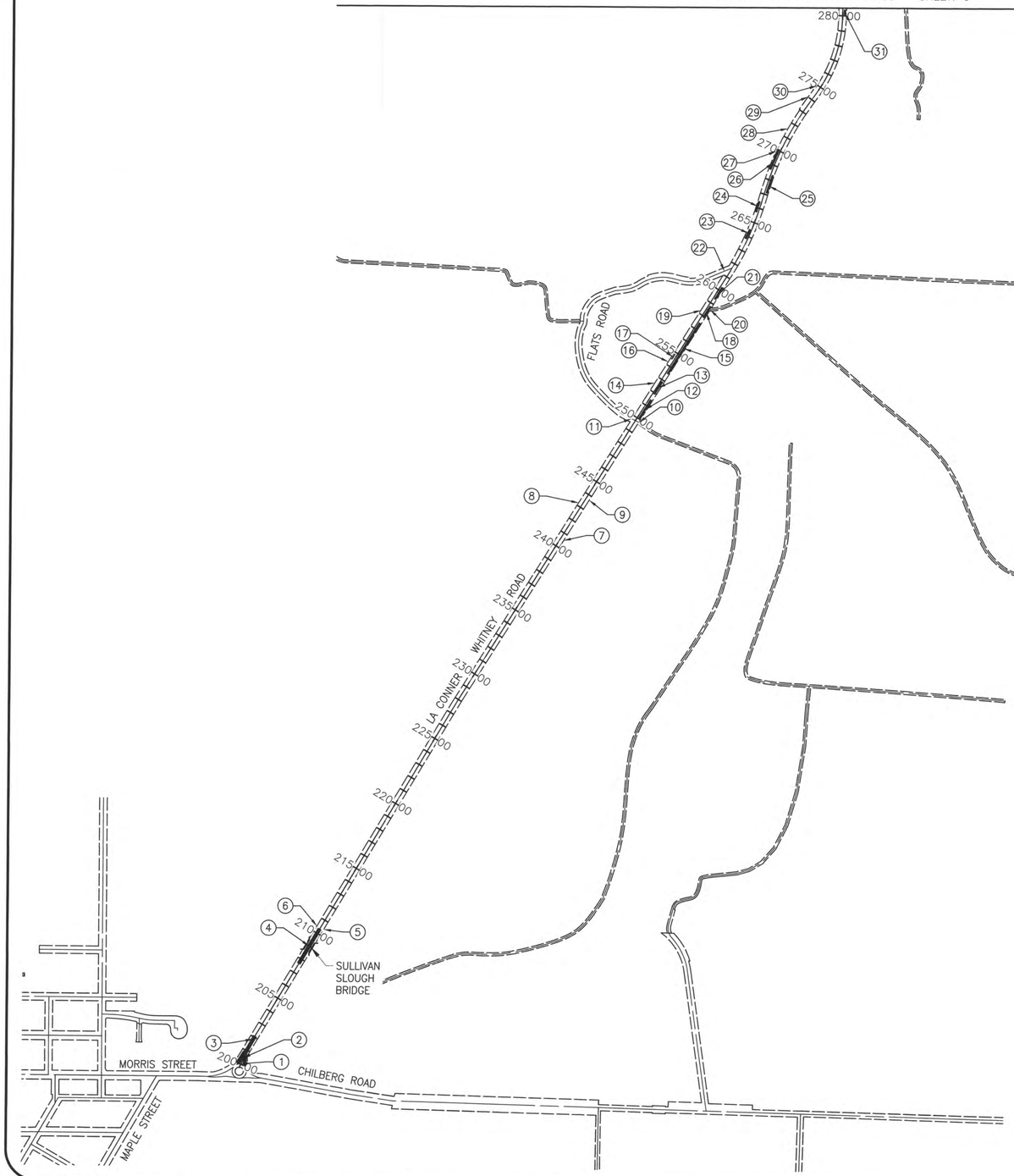
2015 RESURFACING OF ACP ROADWAYS

DETAILS

1 INCH SCALE BAR
ADJUST SCALE ACCORDINGLY

SHEET
4 OF **7**

JEFFL - April 29, 2015 - 3:44 PM - J:\EDS\2015 ASPHALT OVERLAY ESAO15-1\DESIGN_CAD\CONSTR\DT01ESAO15-1.DWG



CONSTRUCTION NOTES:

1. BEGIN PROJECT STA 200+00 AT ROUNDABOUT CONCRETE. PLANE 2" DEEP STA 200+00 TO STA 201+60 INCLUDING BOTH ROUNDABOUT APPROACH LANES NORTH OF ROUNDABOUT. TAPER PLANING FROM 2" AT STA 201+60 TO 0" DEEP AT STA 202+10. SEE NOTE 1 BELOW.
2. PAVEMENT REPAIR EXCAVATION NORTHBOUND LANE ENTIRE 11' LANE STA 200+65 TO 201+00.
3. PAVEMENT REPAIR EXCAVATION SOUTHBOUND LANE OUTER 7' STA 201+00 TO STA 201+40.
4. TAPER PLANING FROM 0" AT STA 207+65 TO 2 ½" AT STA 208+65. STA 208+65 TO 209+35 PLANE ENTIRE AREA OVER BRIDGE DECK, APPROX. 202 SY. INSTALL MEMBRANE WATERPROOFING (DECK SEAL) ON BRIDGE AS SPECIFIED IN DETAIL 6, SHEET 3. OVERLAY BRIDGE DECK WITH 2" HMA CL. ½ PG 64-22. TAPER PLANING FROM 2 ½" AT 209+35 TO 0" AT 210+35.
5. 20' WIDE FIELD ACCESS APPROACH ON RIGHT.
6. 30' WIDE FIELD ACCESS APPROACH ON LEFT.
7. 20' WIDE FIELD ACCESS APPROACH ON RIGHT.
8. 30' WIDE FIELD ACCESS APPROACH ON LEFT.
9. 25' WIDE DRIVEWAY APPROACH ON RIGHT.
10. 40' WIDE DRIVEWAY APPROACH ON RIGHT.
11. STA 249+80 LEFT PLANE FOR 35' LONG 2" DEEP BUTT JOINT AT SOUTH END OF FLATS ROAD.
12. STA 250+00 TO STA 251+50 2" DEPTH PLANING REPAIR OUTER 4' OF NORTHBOUND LANE.
13. STA 251+90 TO STA 252+90 2" DEPTH PLANING REPAIR OUTER 4' OF NORTHBOUND LANE.
14. 30' WIDE DRIVEWAY APPROACH ON LEFT.
15. STA 253+50 TO STA 257+50 2" DEPTH PLANING REPAIR OUTER 4' OF NORTHBOUND LANE.
16. 20' WIDE DRIVEWAY APPROACH ON LEFT.
17. STA 254+50 TO STA 254+85 2" DEPTH PLANING REPAIR OUTER 4' OF SOUTHBOUND LANE.
18. STA 257+90 TO STA 258+80 2" DEPTH PLANING REPAIR ENTIRE 11' WIDE NORTHBOUND LANE.
19. 25' WIDE DRIVEWAY APPROACH ON LEFT.
20. 30' WIDE DRIVEWAY APPROACH ON RIGHT AT RING LANE (PRIVATE ROAD).
21. STA 259+30 TO STA 260+25 2" DEPTH PLANING REPAIR OUTER 4' OF NORTHBOUND LANE.
22. STA 261+70 LEFT PLANE FOR 35' LONG 2" DEEP BUTT JOINT AT NORTH END OF FLATS ROAD.
23. STA 263+80 TO STA 264+50 2" DEPTH PLANING REPAIR OUTER 4' OF SOUTHBOUND LANE.
24. STA 265+70 TO STA 266+45 2" DEPTH PLANING REPAIR OUTER 4' OF SOUTHBOUND LANE.
25. STA 267+10 TO STA 268+35 2" DEPTH PLANING REPAIR OUTER 4' OF NORTHBOUND LANE.
26. STA 268+65 TO STA 270+10 2" DEPTH PLANING REPAIR OUTER 4' OF SOUTHBOUND LANE.
27. 25' WIDE DRIVEWAY APPROACH ON LEFT.
28. 25' WIDE DRIVEWAY APPROACH ON LEFT.
29. 75' WIDE DRIVEWAY APPROACH ON LEFT.
30. 35' WIDE FIELD ACCESS APPROACH ON LEFT.
31. STA 280+05 TO STA 280+40 2" DEPTH PLANING REPAIR OUTER 4' OF NORTHBOUND LANE.

NOTES:

1. CONTRACTOR SHALL BE AWARE THAT THE OVERLAY STARTS AT THE CONCRETE ROUNDABOUT SURFACE. CARE SHALL BE TAKEN TO NOT DAMAGE THE CONCRETE ROUNDABOUT SURFACE.
2. 2" PLANING REPAIR AND PAVEMENT REPAIR LIMITS WILL BE MARKED IN THE FIELD BY SKAGIT COUNTY PRIOR TO CONSTRUCTION.
3. SEE SHEETS 3 & 4 FOR APPROACH, BUTT JOINT, PAVEMENT REPAIR AND 2" PLANING REPAIR DETAILS.

CALL TWO (2) BUSINESS DAYS BEFORE YOU DIG
1-800-424-5555

0 400 800
SCALE IN FEET

SKAGIT COUNTY PUBLIC WORKS
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(360) 336-9400 FAX (360) 336 9478

NO.	REVISIONS	DATE

DESIGN ENGINEER: THOMAS ODEN, STATE OF WASHINGTON, 24001, REGISTERED PROFESSIONAL ENGINEER
ENGINEER OF RECORD: PAUL A. RANDALL-GRUTTER, STATE OF WASHINGTON, 40084, REGISTERED PROFESSIONAL ENGINEER

PROJECT NO.: ESAO15-1
FED. AID NO.:
DESIGNED BY: TH
CHECKED BY: JV
DRAWN BY: TH
APPROVED BY: PAG

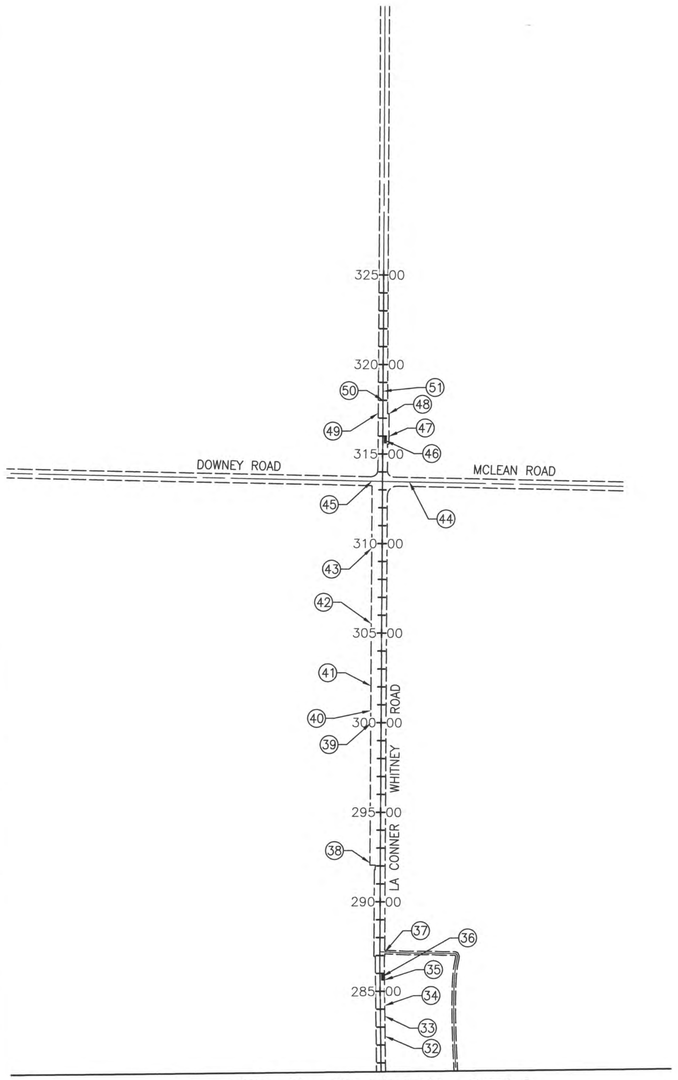
PROJECT LOCATED NEAR:
MOUNT VERNON, WA
T. 33 & 34 N., R. 3 E., W.M.

2015 RESURFACING OF ACP ROADWAYS

LA CONNER WHITNEY ROAD PLAN
STA 200+00 TO STA 280+50

1 INCH SCALE BAR
ADJUST SCALE ACCORDINGLY

SHEET
5 OF 7



MATCH LINE – STA. 280+50.85 – SHEET: 5

NOTES:

1. CONTRACTOR SHALL BE AWARE THAT THE OVERLAY STARTS AT THE CONCRETE ROUNDABOUT SURFACE. CARE SHALL BE TAKEN TO NOT DAMAGE THE CONCRETE ROUNDABOUT SURFACE.
2. PLANING REPAIR LIMITS WILL BE MARKED IN THE FIELD BY SKAGIT COUNTY PRIOR TO CONSTRUCTION.
3. SEE SHEETS 3 & 4 FOR APPROACH, BUTT JOINT, PAVEMENT REPAIR AND 2" PLANING REPAIR DETAILS.
4. WATER VALVE RISERS ON LA CONNER WHITNEY ROAD WILL BE PLACED BY OTHERS PRIOR TO CONSTRUCTION. ALL COORDINATION SHALL BE THROUGH THE ENGINEER.
5. GAS VALVES SHALL BE LOCATED AND MARKED BY THE CONTRACTOR FOR RAISING BY OTHERS AFTER CONSTRUCTION.
6. WATER AND GAS VALVE LOCATIONS HAVE BEEN SHOWN FOR INFORMATIONAL PURPOSES ONLY.

◯ CONSTRUCTION NOTES:

32. 20' WIDE DRIVEWAY APPROACH ON RIGHT.
33. 20' WIDE DRIVEWAY APPROACH ON RIGHT.
34. 75' WIDE DRIVEWAY APPROACH ON RIGHT.
35. 20' WIDE DRIVEWAY APPROACH ON RIGHT.
36. STA 285+60 TO 285+95 2" DEPTH PLANING REPAIR OUTER 4' OF NORTHBOUND LANE.
37. 30' WIDE DRIVEWAY APPROACH ON RIGHT.
38. 30' WIDE FIELD ACCESS APPROACH ON LEFT.
39. 20' FIELD ACCESS APPROACH ON LEFT.
40. 20' FIELD ACCESS APPROACH ON LEFT.
41. 35' WIDE CONCRETE DRIVEWAY APPROACH ON LEFT.
42. 25' WIDE DRIVEWAY APPROACH ON LEFT.
43. 20' WIDE FIELD ACCESS APPROACH ON LEFT.
44. STA 313+50 150' RIGHT PLANE FOR 50' LONG 2" DEEP BUTT JOINT AT MCLEAN ROAD.
45. STA 313+50 PLANE FOR 35' LONG 2" DEEP BUTT JOINT AT DOWNEY ROAD.
46. STA 315+60 TO STA 315+95 2" DEPTH PLANING REPAIR OUTER 4' OF NORTHBOUND LANE.
47. 25' WIDE DRIVEWAY APPROACH ON RIGHT.
48. 70' WIDE DRIVEWAY APPROACH ON RIGHT.
49. 40' WIDE DRIVEWAY APPROACH ON LEFT.
50. STA 318+00 END 1" PRE-LEVEL OF DRIVE LANES.
51. END OF PROJECT STA 318+50 PLANE FOR 50' LONG 2" BUTT JOINT STA 318+00 TO STA 318+50.

UTILITY VAULT SCHEDULE

STATION	OFFSET	DESCRIPTION
STA 252+05	19' LT	1' X 2' CONC. METER BOX

WATER VALVE BOX SCHEDULE

STATION	OFFSET	NOTES
STA 249+85	20' LT	1 WATER VALVE
STA 250+20	11' TO 20' LT	4 WATER VALVES
STA 250+65	15' LT	1 WATER VALVE
STA 251+20	17' LT	1 WATER VALVE
STA 269+60	17' LT	1 WATER VALVE
STA 269+80	17' LT	2 WATER VALVES
STA 272+75	18' LT	1 WATER VALVE
STA 282+05	17' RT	1 GAS VALVE
STA 302+00	17' RT	1 GAS VALVE
STA 313+03	23' LT	1 WATER VALVE
STA 313+15	23' LT	3 WATER VALVES
STA 313+25	16' LT	1 WATER VALVE
STA 313+82	19' LT	1 WATER VALVE

CALL TWO (2) BUSINESS DAYS BEFORE YOU DIG
1-800-424-5555

N

0 400 800
SCALE IN FEET

SKAGIT COUNTY PUBLIC WORKS
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(360) 336-9400 FAX (360) 336 9478

NO.	REVISIONS	DATE

DESIGN ENGINEER
THOMAS J. OREN
STATE OF WASHINGTON
17201
REGISTERED PROFESSIONAL ENGINEER

ENGINEER OF RECORD
PAUL A. RANDALL-GRUTTER
STATE OF WASHINGTON
40084
REGISTERED PROFESSIONAL ENGINEER

PROJECT NO.: ESAO15-1
FED. AID NO.:
DESIGNED BY: TH
DRAWN BY: TH
CHECKED BY: PV
APPROVED BY: PV

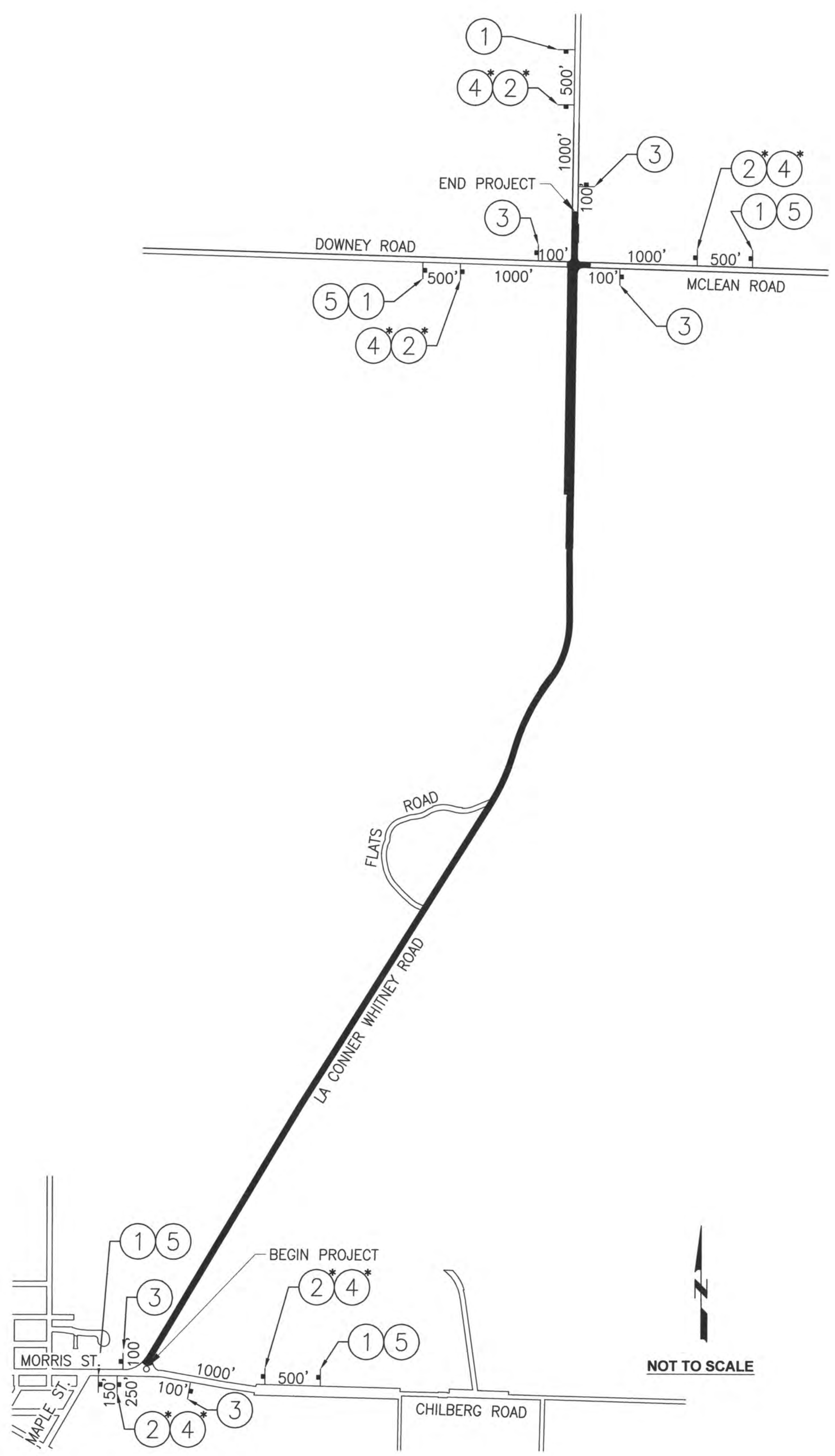
PROJECT LOCATED NEAR:
MOUNT VERNON, WA
T. 33 & 34 N., R. 3 E., W.M.

2015 RESURFACING OF ACP ROADWAYS

LA CONNER WHITNEY ROAD PLAN
STA 280+50 TO STA 318+50

1 INCH SCALE BAR
ADJUST SCALE ACCORDINGLY

SHEET
6 OF 7



LA CONNER WHITNEY ROAD

LACONNER WHITNEY ROAD

LEADER NO.	CONSTRUCTION SIGN	QUANTITY	SIZE	S.F.
1	ROAD WORK AHEAD (W20-1)	5	48"x48"	80
2*	MOTORCYCLES USE EXTREME CAUTION (W21-1701)	5	48"x48"	80
3	END OF ROAD WORK (G20-2)	5	48"x24"	40
4*	GROOVED PAVEMENT (W20-1)	5	48"x48"	80
5	LA CONNER WHITNEY ROAD (G20-2 (MOD))	4	48"x24"	32
				TOTAL S.F. = 312

* COVER SIGNS UNLESS CONDITION EXISTS

CONSTRUCTION SIGN CLASS A NOTES:

1. SIGNS SHALL BE PLACED IN ACCORDANCE WITH THE MOST CURRENT M.U.T.C.D. ADOPTED BY WAC 468-95 AND ITS MODIFICATIONS.
2. SIGNS SHALL CONFORM TO THE 2014 WSDOT STANDARD SPECIFICATION FOR ROAD, BRIDGE, AND MUNICIPAL CONSTRUCTION MANUAL.
3. SIGNS SHALL NOT BE LOCATED WHERE THEY MAY CAUSE SIGHT DISTANCE PROBLEMS.
4. IF WORK INCLUDES GROOVED PAVEMENT, ABRUPT LANE EDGES, STEEL PLATES, OR GRAVEL OR EARTH SURFACES, SIGNS SHALL BE PLACED STATING THE CONDITION, AS REQUIRED BY THE CURRENT M.U.T.C.D. AND WAC 468-95.
5. CONTRACTOR SHALL FURNISH ALL SIGNS.
6. SIGNS SHALL BE MOUNTED ON 4X4 POSTS, UNLESS OTHERWISE NOTED.
7. CLASS B SIGNS ARE NOT SHOWN ON DRAWING.
8. ALL SIGN SPACING MAY BE ADJUSTED TO ACCOMMODATE AT-GRADE INTERSECTIONS AND DRIVEWAYS.

SKAGIT COUNTY PUBLIC WORKS
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(360) 336-9400 FAX (360) 336 9478

NO. | REVISIONS | DATE

DESIGN ENGINEER
ENGINEER OF RECORD

PROJECT NO.: ESAO15-1
FED. AID NO.:
DESIGNED BY: TH
DRAWN BY: TH
CHECKED BY:
APPROVED BY:
PROJECT LOCATED NEAR:
MOUNT VERNON, WA
T. 33 & 34 N., R. 3E., W.M.

2015 RESURFACING OF ACP ROADWAYS

CLASS A SIGNING PLAN

1 INCH SCALE BAR
ADJUST SCALE ACCORDINGLY

SHEET 7 OF 7